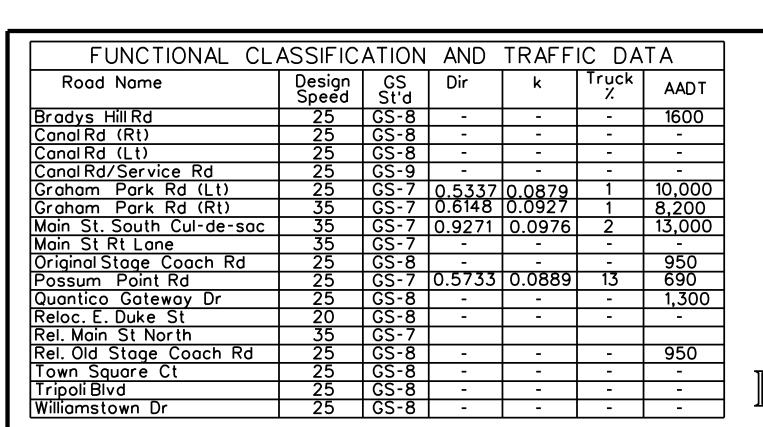
PROJECT MANAGER Hoginam_Nguyen, P.E. (703).792-8161.PWC_Dept. of Transportation SURVEYED BY, DATE Leon_E.Treutle, L.S. (703).259-3224_7/17/13
DESIGN BY _Jeff_Cronin_P.E., Johnson, Mirmiran_&_Thompson (804) 323-9900
SUBSURFACE UTILITY BY, DATE _Leon_E.Treutle, L.S. (703).259-3224_7/17/13



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FOR INDEX OF SHEETS SEE SHEET 1B
THIS PROJECT WAS DEVELOPED UTILIZING THE DEPARTMENT'S
ENGINEERING DESIGN PACKAGE (GEOPAK).
GEOPAK Computer Identification No. UPC 119481

CONVENTIONAL SIGNS

STATE LINE ... COUNTY LINE

WATER LINE

GUARD RAIL

RAILROADS

BRIDGES

HEDGE TREES

CULVERTS ... DROP INLET

POWER POLES

HEAVY WOODS

GROUND ELEVATION GRADE ELEVATION

TRAVELED WAY.

RETAINING WALL

GAS LINE

CITY, TOWN OR VILLAGE RIGHT OF WAY LINE ...

UNFENCED PROPERTY LINE

ELECTRIC UNDERGROUND CABLE

TELEPHONE OR TELEGRAPH POLES

TELEPHONE OR TELEGRAPH LINES

FENCED PROPERTY LINE

SANITARY SEWER LINE

BASE OR SURVEY LINE

LEVEE OR EMBANKMENT

RG I W

FHWA 534 DATA 43

COMMONWEALTH OF VIRGINIA

Prince William County Dept. of Transportation

PLAN AND PROFILE OF PROPOSED STATE HIGHWAY

TIER 2 PROJECT

LOCALLY ADMINISTERED PROJECTS

PRINCE WILLIAM COUNTY DEPARTMENT OF TRANSPORTATION

RECOMMENDED FOR APPROVAL FOR RIGHT OF WAY ACQUISITION

DIRECTOR OF TRANSPORTATION

PRINCE WILLIAM COUNTY DEPARTMENT OF TRANSPORTATION

RECOMMENDED FOR APPROVAL FOR RIGHT OF WAY ACQUISITION

INFRASTRUCTURE INVESTMENT DIRECTOR

STATE LOCATION AND DESIGN ENGINEER

CHIEF FINANCIAL OFFICER

APPROVED FOR RIGHT OF WAY ACQUISITION

RECOMMENDED FOR APPROVAL FOR CONSTRUCTION

INFRASTRUCTURE INVESTMENT DIRECTOR

STATE LOCATION AND DESIGN ENGINEER

STATE STRUCTURE AND BRIDGE ENGINEER

CHIEF FINANCIAL OFFICER

CHIEF ENGINEER

CHIEF ENGINEER

CHIEF OF POLICY

ASSISTANT DIRECTOR OF TRANSPORTATION

RECOMMENDED FOR APPROVAL FOR CONSTRUCTION

Prince William County

FUNCTIONAL CLASSIFICATION AND TRAFFIC DATA

NHS - URBAN PRINCIPAL ARTERIAL (GS-5) - ROLLING - 45 MPH MIN. DESIGN SPEED

Fr: 0.016 MI. S. OF BRADYS HILL ROAD
To: 0.022 MI. N. OF ROUTE 234 (DUMFRIES ROAD)

ADT 2021 24,000

ADT 2042 69,000

DHV 4,620

D (%) (design hour) 55/45

T (%) (design hour) 3%

V (MPH) 45

DESIGN VEHICLE WB-62 & S-BUS-40

SEE TYPICAL SECTION SHEETS FOR CONNECTION FUNCTIONAL CLASSIFICATION

TOWN OF DUMFRIES

FRALEY BLVD (US RTE.1) - WIDEN TO SIX LANES

FROM: 0.016 MI. S. OF BRADYS HILL ROAD

TO: 0.022 MI. N. OF ROUTE 234 (DUMFRIES ROAD)

To Nexardrio

Aggroy, Town
Municipal Units

Aggroy, Town
Municipal Units

Real of Particles Blvd.

Real of Particles Blvd.

Description Reference
Poil: 5ta 254-75 (Constr. B/L Rin.)
PROJ. 0001-212-249, RW-201

THE COMPLETE ELECTRONIC PDF VERSION OF THE PLAN
ASSEMBLY AS AWARDED, INCLUDING ALL SUBSEQUENT REVISIONS,
WILL BE THE OFFICIAL CONSTRUCTION PLANS. FOR INFORMATION
RELATIVE TO ELECTRONIC FILES AND LAYERED PLANS, SEE GENERAL NOTES.

DESIGN FEATURES RELATING TO CONSTRUCTION OR TO REGULATION AND CONTROL OF TRAFFIC MAY BE SUBJECT TO CHANGE AS DEEMED NECESSARY BY THE DEPARTMENT.

THIS PROJECT IS TO BE CONSTRUCTED IN ACCORDANCE WITH THE DEPARTMENT'S 2020 ROAD AND BRIDGE SPECIFICATIONS, 2016 ROAD AND BRIDGE STANDARDS (REV SEPT. 2022), 2009 MUTCD, 2011 VIRGINIA SUPPLEMENT TO THE MUTCD, 2011 VIRGINIA WORK AREA PROTECTION MANUAL AND AS AMENDED BY CONTRACT PROVISIONS AND THE COMPLETE ELECTRONIC PDF VERSION OF THE PLAN ASSEMBLY.

ALL CURVES ARE TO BE SUPERELEVATED, TRANSITIONED AND WIDENED IN ACCORDANCE WITH STANDARD $\underline{\text{TC-5.11ULS}}$, EXCEPT WHERE OTHERWISE NOTED.

THE <u>ORIGINAL</u> APPROVED TITLE SHEET(S), INCLUDING ORIGINAL SIGNATURES, IS FILED IN THE VDOT CENTRAL OFFICE PLAN LIBRARY. ANY MISUSE OF ELECTRONIC FILES, INCLUDING SCANNED SIGNATURES, IS ILLEGAL AND ENFORCED TO THE FULL EXTENT OF THE LAW.

Town of Dumfries Population 5,823 (2022 Census)

| STATE PROJECT | SECTION | FEDERAL AID PROJECT NO. | TYPE CODE | UPC NO. | LENGTH II BRIDG | | LENGTH E BRIDG | XCLUDING (E(S) | BRIDGE PLAN | TYPE PROJECT | DESCRIPTION | | | |
|------------------|---------|----------------------------|--------------|------------|--------------------|-------|-------------------|-------------------|----------------|-----------------|--|--|--|--|
| NO. | | TROOLET NO. | | 140. | FEET | MILES | FEET | MILES | NO. | I KOOLC I | | | | |
| | PE-101 | NHPP-5B0I(305) | PENG | 119481 | 10,200.54 | 1.932 | 9,920.54 | 1 <i>8</i> 79 | | Prel. Engr. | From: 0.016 Mi. S. of Brody's Hill Rd | | | |
| | | | | | | | | | | | To: 0.022 Mi. N. of Dumfries Rd (Route 234) | | | |
| -249 | RW-201 | NHPP-5B0((306) | ROWA | 119481 | 10,062.63 | 1.906 | 9,782.63 | 1.852 | | Right of Way | From: 0.004 Mi. S. of Brady's Hill Rd | | | |
| -512- | | | | | | | | | | | To: O.DI5 Mi. S. of Dumfries Rd (Route 234) | | | |
| 1000 | C-50I | NHPP-5B0((307) | 1000 | 119481 | 10,200.54 | 1.932 | 9,920,54 | 1 <i>,</i> 879 | | Construction | From: 0.016 Mi.S.of Brady's Hill Rd | | | |
| | | | | | | | | | | | To: 0.012 Mi.S.of Dumfries Rd (Route 234) | | | |
| | B-602 | NHPP-5B0I(307) | | 119481 | 280.00 | 0.053 | | | 302-14 | Bridge | Bridge carrying Rte. I (Fraley Blvd.) over Quantico Creek (State Str. No. xxx) | | | |

Project Lengths are based on Rte.I Construction Baseline.

DATE DIVISION ADMINISTRATOR
FEDERAL HIGHWAY ADMINISTRATION
U.S. DEPARTMENT OF TRANSPORTATION

Copyright 2022, Commonwealth of Virginia

APPROVED FOR CONSTRUCTION

PROJECT SHEET NO. 1

SCALE

500′

REVISED

PROJECT MANAGER Hoainam Nguyen, P.E. (703) 792-8161 PWC Dept. of Transportation SURVEYED BY, DATE Leon E. Treutle LS (703)259-3224 7/17/13

DESIGN BY JMT Engineering (804) 323-9900

SUBSURFACE UTILITY BY, DATE Leon E. Treutle LS (703)259-3224 7/17/13

LOCATION MAP

dII948I0Ia.dgn

Plotted By: ong

DESIGN FEATURES RELATING TO CONSTRUCTION OR TO REGULATION AND CONTROL OF TRAFFIC MAY BE SUBJECT TO CHANGE AS DEEMED NECESSARY BY THE DEPARTMENT

REVISED STATE ROUTE PROJECT 0001-212-249, RW-201, C-501, B-6XX

Creek Town Õ

Luantico

613

95

/ 788

234

627

TO FARFA

629

619

95

975

730

Triangle

721/

U.S. MARINE CORPS

RESERVATION

U.S.M.C.

619

TRUCK

SCALE

PROJECT SHEET NO. 0001-212-249

R/W PLANS

633

12A

PROJECT MANAGER <u>Hoainam Nguyen, P.E.(703) 792-8161 PWC Dept. of Transportation</u> SURVEYED BY, DATE <u>Leon E.Treutle LS (703)259-3224 7/17/13</u> DESIGN BY <u>JMT Engineering (804) 323-9900</u> SUBSURFACE UTILITY BY, DATE <u>Leon E.Treutle LS (703)259-3224 7/17/13</u> INDEX OF SHEETS

| DESIGN FEATURES RELATING TO CONSTRUCTION | REVISED | STATE | STATE | | | | |
|---|---------|-------|-------|--------------------------------|-------|--|--|
| OR TO REGULATION AND CONTROL OF TRAFFIC | | SIAIE | ROUTE | PROJECT | SHEET | | |
| MAY BE SUBJECT TO CHANGE AS DEEMED NECESSARY BY THE DEPARTMENT | | VA. | , | 0001-212-249, RW-201, C-501 | IB | | |

| SI | HEET NO. | <u>DESCRIPTION</u> | | |
|-----------|---------------------------|---|-------------------|---|
| <u>51</u> | <u> </u> | | SHEET NO. | <u>DESCRIPTION</u> |
| / | | TITLE SHEET | SHEET HO. | <u>BESCHII TION</u> |
| IA | | LOCATION MAP | 13 | PLAN SHEET STA. 320.00 to 326.00 |
| IB | | INDEX OF SHEETS | /3A | PROFILE SHEET STA. 320.00 to 326.00 |
| | C(I) thru IC(3) | RIGHT OF WAY DATA SHEETS | <i>13B</i> | PROFILE SHEET CANAL RD RT, CANAL RD LT. |
| | O(1) | REVISION DATA SHEET | | CANAL RD SERVICE RD, GRAHAM ST |
| | E(I) thru IE(4) | EXISTING DRAINAGE DESCRIPTIONS | 14 | PLAN SHEET STA. 326+00 to 333+00 |
| | F(I) & IF(2) | SURVEY HORIZONTAL AND VERTICAL CONTROL SHEETS | <i>14</i> A | PROFILE SHEET STA. 326.00 to 333.00 |
| | G(I) thru IG(4) | CONSTRUCTION ALIGNMENT DATA SHEETS | I4B | NOT USED |
| | G(5) thru IG(II) | ENTRANCE CONSTRUCTION ALIGNMENT DATA SHEETS | 14C | PLAN SHEET CANAL RD SERVICE RD & POSSUM POINT RD |
| * IH | | RADIAL OFFSET SHEET | 14D | PROFILE SHEET RELOC. MAIN ST NORTH, MAIN ST RIGHT TURN LANE |
| // | | TRANSPORTATION MANAGEMENT PLAN GENERAL NOTES | - 14E | PROFILE SHEET POSSUM POINT RD & TRIPOLI BLVD |
| * IJ | | CONSTRUCTION SIGN SCHEDULE | <i>1</i> 5 | PLAN SHEET STA. 333.00 to 340.00 |
| | (I) thru IK(22) | MAINTENANCE OF TRAFFIC / SEQUENCE OF CONSTRUCTION SHEETS | 15A | PROFILE SHEET STA. 333.00 to 340.00 |
| | (I) thru IL(IO) | TEMPORARY SIGNAL SHEETS | 15B | NOT USED |
| | M(I) thru IM(2) | EROSION CONTROL GENERAL NOTES & LEGENDS | 15C | PLAN SHEET RELOC. OLD STAGE COACH RD & ORIGINAL STAGE COACH RD |
| | (()) thru IN(23) | PHASE I EROSION CONTROL PLAN SHEETS | 15D | PROFILE SHEET RELOC. OLD STAGE COACH RD & ORIGINAL STAGE COACH RD |
| | (24) | SEDIMENT TRAP DETAIL SHEET | 16 | PLAN SHEET STA. 340.00 to 347.00 |
| 10 | | NOT USED | 16A | PROFILE SHEET STA. 340.00 to 347.00 |
| IP | P(I) thru IP(23) | PHASE 2 EROSION CONTROL PLAN SHEETS | 17 | PLAN SHEET STA. 347.00 to 353.00 |
| 10 | | HYDROLOGIC DATA SHEET | // /7A | PROFILE SHEET STA. 347.00 to 353.00 |
| | (1) thru 2(2) | GENERAL NOTES | 18 | PLAN SHEET STA. 353.00 to 360.00 |
| | A(I) thru 2D(3) | TYPICAL SECTIONS | 18A | PROFILE SHEET STA. 353.00 to 360.00 |
| | E(I) thru 2E(6) | DRAINAGE DESCRIPTION SHEETS & DITCH TYPICALS | IUA | THOUSE STEET STA SSS OU TO SOUTO |
| | F(O) | NOT USED | 19(1) thru 19(6) | ENTRANCE PROFILE SHEETS |
| | G(O) | NOT USED | | |
| | H(I) thru 2H(4) | STORMWATER POLLUTION PREVENTION PLAN (SWPPP) | 20(1) thru 20(22) | STORM SEWER PROFILES |
| | [(1)-2 <u>[</u> (2) | BIO-RETENTION LANSCAPING PLAN & CONSTRUCTION SEQUENCE | 21(1) thru 21(18) | SIGNING AND PAVEMENT MARKING PLANS |
| 210 | ((3,3A)-2 <u>[</u> (7,7A) | SWM PLAN DETAIL SHEETS & SWM PROFILE DETAIL SHEETS | 22(1) thru 22(6) | SIGNAL PLANS |
| 25 | J(I) thru 2J(2) | DEMOLITION OF BUILDINGS/CLEARING OF PARCELS | | |
| 2K | K(I) - 2K(8) | RETAINING WALLS (I-8) | | |
| * 2L | <u>'</u> | GRADING DIAGRAM AND SUMMARY | BRIDGE PLANS, B- | 602. PLAN NO. 302-14 (4 SHEETS) |
| * 2M | M | PAVEMENT SUMMARY | [RTE.I(FRALEY | BLVD. OVER QUANTICO CREEK] |
| * 21 | V | INCIDENTAL SUMMARY | TOTAL CROSS SEC | CTION SHEETS XI- X203 |
| * 20 | 0 | ROADSIDE DEVELOPMENT SHEET | | TION SHEET NUMBER XI FOR INDEX OF SHEETS) |
| 3 | | PLAN SHEET STA. 251.00 to 258.00 | | |
| <i>3A</i> | 4 | PROFILE SHEET STA. 251.00 to 258.00 | | |
| <i>3E</i> | 8 | PLAN SHEET QUANTICO GATEWAY DR & MAIN ST S. CUL-DE-SAC | | |
| 30 | C | PROFILE SHEET QUANTICO GATEWAY DR., MAIN ST CUL-DE-SAC, MAIN ST CUL-DE-SAC EOP & TOWNSQUARE COURT | | |
| 4 | | PLAN SHEET STA. 258.00 to 265.00 | | |
| 4A | 4 | PROFILE SHEET STA. 258.00 to 265.00 | | |
| 5 | | PLAN SHEET STA. 265.00 to 272.00 | | |
| 5A | 4 | PROFILE SHEET STA. 265.00 to 272.00 | | |
| 6 | | PLAN SHEET STA. 272.00 to 279.00 | | |
| 6A | 4 | PROFILE SHEET STA. 272.00 to 279.00 | | |
| 6B | 3 | PLAN SHEET PARCEL 024 ENTRANCE | | |
| 7 | | PLAN SHEET STA. 279.00 to 286.00 | | |
| 7A | 4 | PROFILE SHEET STA. 279.00 to 286.00 | | |
| 7B | 3 | PLAN SHEET GRAHAM PARK RD LT. | | |
| <i>7C</i> | 2 | PLAN SHEET GRAHAM PARK RD RT. | | |
| 70 | | PROFILE SHEET GRAHAM PARK RD LT.& GRAHAM PARK RD RT. | | |
| 8 | | PLAN SHEET STA. 286.00 to 293.00 | | |
| 8A | 4 | PROFILE SHEET STA. 286+00 to 293+00 | | |
| 9 | | PLAN SHEET STA. 293.00 to 300.00 | | |
| 9A | 4 | PROFILE SHEET STA. 293.00 to 300.00 | | |
| 10 | | PLAN SHEET STA. 300.00 to 306.00 | | |
| 101 | | PROFILE SHEET STA. 300.00 to 306.00 | | |
| 11 | | PLAN SHEET STA. 306.00 to 313.00 | | |
| IIA | 4 | PROFILE SHEET STA. 306+00 to 313+00 | | |
| IIB | | PROFILE SHEET RELOC.EAST DUKE ST & WILLIAMSTOWN DR | | |
| טוי פו | - | PLAN SHEET STA. 313.00 to 320.00 | | |
| 12 | · A | PROFILE SHEET STA 313.00 to 320.00 | | |

STA. 313.00 to 320.00

PROFILE SHEET

* Sheets not included with this submission.

SHEET NO. PROJECT 0001-212-249

dll948l0lcl.dgn Plotted By:ong

PROJECT MANAGER <u>Hodinam Nguyen</u>, P.E.(703) 792-8161 PWC <u>Dept. of Transportation</u>
SURVEYED BY, DATE <u>Leon E.Treutle LS</u> (703)259-3224 7/17/13

DESIGN BY <u>JMT Engineering</u> (804) 323-9900

SUBSURFACE UTILITY BY, DATE <u>Leon E.Treutle LS</u> (703)259-3224 7/17/13

PRELIMINARY RIGHT OF WAY DATA SHEET

| REVISED | STATE | | STATE | SHEET NO. |
|---------|-------|-------|---------------------------------|-----------|
| | SIAIE | ROUTE | PROJECT | SHEET NO. |
| | VA. | 1 | 0001-212-249 , RW-201 | IC(I) |

DESIGN FEATURES RELATING TO CONSTRUCTION OR TO REGULATION AND CONTROL OF TRAFFIC MAY BE SUBJECT TO CHANGE AS DEEMED NECESSARY BY THE DEPARTMENT

| PARCEL NO. LANDOWNER LANDOWNER LANDOWNER LANDOWNER TOTAL FEE TAKING PRESCRIPTIVE R/W FEE REMAINDER PRESCRIPTIVE R/W FEE REMAINDER DRAINAGE & SLOPE & DRAINAGE & RET. WALL DRAINAGE SLOPE & JOINT COLUMBIA DOMINION VERIZON MISC. TEMPORARY ENTRANCE WALL PROFFEE WALL PROFFEE WALL PROFFEE WALL PROFFEE TOTAL A CRES OR SOLUARE | | | | | | | | AREA PERMANENT EASEMENTS UTILITY TEMPORARY CON | | | | | OR A RY CONSTRI | ISTRUCTION | | | | | | |
|--|--------|---|--|------------|-------------------|------------------|------------|--|------|----------------|--------------|-------------------|-----------------|------------|---------|---------------------------|-------------------|----------------------------|----------|--|
| Part | PARCEI | | | тотлі | EEE TA VINC | DDECCDIDTIVE D/W | | DD A DIA CE | | | | | | 0111 | | | 1 LAVII V | | | |
| March 1975 Mar | | LANDOWNER | SHEET NO. | | | | | | | | | | | | VERIZON | MISC. ENTRANCE WALL PROFF | | | PROFFERS | |
| Manual Properties (1987) Manual Properties (| | | | | ACRES OR SQ. FEET | SQ. FEET | | | AG | CRES OR SQ. FE | ET | ACRES OR SQ. FEET | | | | | ACRES OR SQ. FEET | | | YES/NO |
| MARKET DEVELOPE FOR PROPERTY CONTROL FOR THE PARTY CONTROL FOR T | 001 | NOT USED | | | | | | 0 SF | 0 SF | 0 SF | 0 SF | 0 SF | | | | | 0 SF | 0 SF | 0 SF | |
| Dec 1.5 | 002 | NOT USED | | | | | | | | | | | | | | | | | | |
| Post Continue Co | 003 | MARINE TOYS FOR TOTS FOUNDATION | 3,3B | 2.27 AC | 216 SF | | 2.265 AC | | | | 321 SF | | | | | 2240 SF | 4518 SF | | | NO |
| Total Tota | 004 | NOT USED | | | | | | | | | | | | | | | | | | |
| DEFINITION FOR CONTINUENCE CONTINUENCE 14 20 20 14 20 20 14 20 20 20 20 20 20 20 2 | 005 | EVELYN L. BARNES, TRUSTEE | 3,3B,4 | 0.846 AC | 36852 SF | | 0 AC | | | 176 SF | | 3412 SF | | | | | | | | NO |
| Company Comp | 006 | ANDREW COVAN | 3 | 0.586 AC | 937 SF | | 0.564 AC | | | | | | 3155 SF | | | | 2329 SF | | | NO |
| SANDARA PARE MINISTRA MARKET 14 174 of 96 of 179 of 17 | 007 | DUMFRIES - TRIANGLE VOLUNTEER FIRE DEPARTMENT | 3 | 0.501 AC | 407 SF | | 0.492 AC | | | | | | 2617 SF | | 410 SF | | 683 SF | 2675 SF | | NO |
| THANGE POWER PURPOSE DURNAGE COMPANY I C 10 10 10 10 10 10 10 | 008 | DUMFRIES - TRIANGLE VOLUNTEER FIRE DEPARTMENT | 3,4 | 2.184 AC | | | 2.184 AC | | | | | | | | 839 SF | | | 1199 SF | | NO |
| OFF | 009 | ASAD NAJAFE & SHEKIBA NAJAFE | 3,4 | 1.151 AC | 954 SF | | 1.129 AC | | | | | 109 SF | 2174 SF | | | | | 2251 SF | | NO |
| Page | 010 | TRIANGLE OFFICE BUILDING COMPANY, LLC | 3B | 4.067 AC | | | 4.067 AC | | | | | | | | | | 1681 SF | | | NO |
| MILESPAY | 011 | NOT USED | | | | | | | | | | | | | | | | | | |
| MAKE | 012 | TRIANGLE SELF STORAGE, LC | 4,5 | 2.861 AC | 20403 SF | | 2.393 AC | | | 2850 SF | 865 SF | | | | 250 SF | | 4683 SF | 3015 SF | | NO |
| 0.000 0.00 | 013 | NOT USED | | | | | | | | | | | | | | | | | | |
| 0.00 ARCOLD PROPREDIAL 4 0.758 AC 0.00 AC 0. | 014 | QUANTICO PROPERTY, LLC | 5 | 2.502 AC | 22216 SF | | 1.992 AC | 18057 SF | | | | | | | | | | | | NO |
| 0.000 0.00000 0.00000 0.00000 0.00000 0.0000 | 015 | ARNOLD PROPERTIES, LLC | 4 | 1.771 AC | 122 SF | | 1.768 AC | | | | | | 2114 SF | | | | | 2085 SF | | NO |
| 10 10 10 10 10 10 10 10 | 016 | ARNOLD PROPERTIES, LLC | 4 | 0.785 AC | | | 0.785 AC | 14016 SF | | | | | 97 SF | | | | | 913 SF | | NO |
| 99 CUMMINIMENT ALLI OF VIRGINIA 0 A REPORT STATEMENT OF PROPERTISS LLC 5 A9F AC 5 A9F AC 6 A9F AC 7 TRANSCE PLACE, LLC 6 BEST AND ACC 7 TRANSCE PLACE, LLC 6 BEST AND ACC 7 TRANSCE PLACE, LLC 7 TRANSCE PLACE, LLC 7 TRANSCE PLACE, LLC 8 APF ACC 8 APF ACC 8 APF ACC 9 ACC 1 APF ACC 1 | | | 4 | 0.525 AC | 109 SF | | | 1720 SF | | | | | 2336 SF | | | | 483 SF | | | NO |
| CO REPORT NET PROPERTIES, LIC 5 0.49 AC 2189 SF 0.259 AC 2189 SF 0.6C 218 | 018 | K & R INVESTMENT PROPERTIES, LLC | 4,5 | 0.955 AC | | | 0.955 AC | | | | | | 4194 SF | | | | 2070 SF | 1512 SF | | NO |
| COL | 019 | COMMONWEALTH OF VIRGINIA | 5,6,6B | | | | | | | | | | | | | | | | | NO |
| Dec Column Colu | 020 | | 5 | | - | | | | | | | | 1247 SF | | | | 517 SF | 616 SF | | NO |
| 03 TRANCLE PLAZ, LIC | | | 5 | 0.501 AC | 21836 SF | | 0 AC | | | | | | | | | | | | | NO |
| A | 022 | | | | | | | | | | | | | | | | | | | |
| 10.5 MALLE | 023 | <u> </u> | 6 | | | | | | | | | | | | | | | | | |
| Decoration Color | | | 1 1 1 | | · · | | | 1183 SF | | | | | | | | | | | | |
| 0.7 SLJ ASKIT MANAGEMENT & RIVESIMENT COMPANY 7 22249 S 22349 S 0.5F | | | | | | | | | | | | 90 SF | 2063 SF | | | | 163 SF | | | |
| 0.00 | | | 7,7C | | | | | 3099 SF | | | 2788 SF | | | | | 2355 SF | | 10101 SF | | |
| 0.00 THE WILLIAM HOMFOWNERS ASSOCIATION, INC. 7C 9.025 AC 2119 SF 8.976 AC 4051 SF 4299 SF 4347 SF NO | | | 7 | | | | | + | | | | | 2400.07 | | | | 0=0 0= | | | |
| OFFIT MAIN INDICENTED NOT 150 15 | | | 7,8 | 15143 SF | 15143 SF | | 0 SF | | | | | | 2409 SF | | | | 979 SF | | | NO |
| 031 NOTUSED | | | | 0.027.4.0 | 2440.07 | | 0.0=6.4.0 | | | | | | 10.51 07 | 10 60 67 | | | 10.17 | | | 1 |
| 032 MGB PROPERTIES VII, LLC 78.8 0.449 AC 1199 SF 0.421 AC 0.586 SF 0.500 AC 0.587 SF 0.588 SF 0.500 AC 0.588 SF 0.500 AC 0.588 SF 0.500 AC 0.588 SF 0.500 AC 0.580 SF 0.580 AC 0.580 SF 0.580 AC 0.580 SF 0.580 AC 0.580 SF 0.580 AC 0.58 | | | 7C | 9.025 AC | 2119 SF | | 8.976 AC | | | | | | 4051 SF | 4269 SF | | | 4347 SF | | | NO |
| 033 RELIABLE ALTO SALES AND SERVICES, INC 8 7414 SF 7414 | | | 5 D 0 | 0.440.4.0 | 1100 GF | | 0.424.4.0 | | | | | (42 GF | 205 07 | | | | | 600 GF | | 110 |
| 0.34 MGB PROPERTIES VII, LLC 78,8 1.06 AC 1.326 SF 6.8F 1.029 AC 1.326 SF 6.8F 1.029 AC 1.326 SF 6.8F 1.029 AC 1.326 SF 1.725 SF 1.025 SF 1.02 | | | 7B,8 | | | | | | | | | | | | | | | 698 SF | | |
| 0.55 MGB PROPERTIES VII, LLC | | | 8 | | | C OF | | | | | | | | | | | 250 CF | | | |
| 0.36 RAMIN, LLC RAMIN, LL | | , | /B,8 | | | | | | | | | | + | | | | 379 SF | 21.45 CF | | |
| NOT USED | | | 8 | | | | | | | | | | | | | | | 2145 SF | | |
| 038 NOT USED | | | 8,9 | 20649 SF | 20649 SF | 63// SF | USF | | | | | 1997 SF | 226/ SF | | | | | | | |
| 039 DUMFRIES SHOPPING CENTER, INCORPORATED 7C,8,9,10 11.704 AC 1.266 AC 10.438 AC 8757 SF 30407 SF 103 SF 12664 SF 11318 SF NO | | | | | | | | | | | | | | | | | | | | |
| RAYMOND F. MOUNTJOY & SHERRILL M. MOUNTJOY 12,13 2.041 AC 5846 SF 1.907 AC 3922 SF 919 SF 3978 SF NO | | | 70.9010 | 11 704 A C | 1 266 AC | | 10 429 A C | | | | 9757 CE | | 20407 SE | | 102 SE | | 12664 SE | 11210 CE | | |
| 041 RAMIN, LLC 9 6694 SF 6694 SF 897 SF 0 SF 738 SF 1225 SF 1225 SF NO 042 FRALEY BUILDING FAMILY LIMITED PARTNERSHIP 9 16275 SF 1372 SF 1800 SF 14903 SF 2379 SF 2379 SF 1154 SF 600 SF NO 043 BCL ASSOCIATES, LLC 9 57000 SF 3579 SF 4448 SF 53421 SF 5405 SF 2998 SF 2877 SF NO 044 WNI HOLDINGS, LLC 9 25252 SF 1510 SF 1559 SF 23742 SF 1543 SF 2100 SF 5688 SF NO 045 WNI HOLDINGS, LLC 91.0 2 AC 4771 SF 4425 SF 1.89 AC 5672 SF 150 SF 2100 SF 5688 SF NO 046 241 FRALEY, LLC 9 14300 SF 2278 SF 1202 SF 1202 SF 1100 SF 750 SF 910 SF NO 048 DUMFRIES PLAZA EXXON, LTD. 9 0.895 AC 5771 SF 0.763 AC 5752 SF 7623 SF 14091 SF <td></td> <td> </td> <td>1 11</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>8/3/ SF</td> <td>2022 CE</td> <td></td> <td></td> <td>103 SF</td> <td>2079 CE</td> <td>12004 SF</td> <td>11318 SF</td> <td></td> <td></td> | | | 1 11 | | | | | | | | 8/3/ SF | 2022 CE | | | 103 SF | 2079 CE | 12004 SF | 11318 SF | | |
| 042 FRALEY BUILDING FAMILY LIMITED PARTNERSHIP 9 16275 SF 1372 SF 1800 SF 14903 SF 2379 SF 1154 SF 600 SF NO 043 BCL ASSOCIATES, LLC 9 57000 SF 3579 SF 4448 SF 53421 SF 5405 SF 2998 SF 2877 SF NO 044 WNI HOLDINGS, LLC 9 2525 SF 1510 SF 1559 SF 23742 SF 1543 SF 2100 SF 5688 SF NO 045 WNI HOLDINGS, LLC 9,10 2 AC 4771 SF 4425 SF 1.89 AC 5672 SF 3369 SF 5333 SF NO 046 241 FRALEY, LLC 9 14300 SF 2278 SF 1202 SF 1200 SF 1200 SF 1200 SF 1200 SF | | | 12,13 | | | 907 SE | | | | | | | + | | | 39/0 SF | | | | |
| 043 BCL ASSOCIATES, LLC 9 57000 SF 3579 SF 4448 SF 53421 SF 1 2998 SF 2877 SF NO 044 WNI HOLDINGS, LLC 9 25252 SF 1510 SF 1559 SF 23742 SF 1 1543 SF 2100 SF 5688 SF NO 045 WNI HOLDINGS, LLC 9,10 2 AC 4771 SF 4425 SF 1.89 AC 5672 SF 3369 SF 5333 SF NO 046 241 FRALEY, LLC 9 14300 SF 2278 SF 12022 SF 100 SF 770 SF NO 047 241 FRALEY, LLC 9 16900 SF 2623 SF 14277 SF 1300 SF 910 SF 90 SF NO 048 DUMFRIES PLAZA EXXON, LTD. 9 0.895 AC 5771 SF 0.763 AC 3000 SF 2100 SF 487 SF NO 049 APARTMENTS AT DUMFRIES II, LLC 10,11 6.124 AC 1.262 AC 3972 SF 4.861 AC 5752 SF 7623 SF 14091 SF 14091 SF NO | | | 9 | | | | | + | | | | /38 SF | 1 | | | | 1154 CD | 600 SE | | |
| 044 WNI HOLDINGS, LLC 9 25252 SF 1510 SF 1559 SF 23742 SF NO 045 WNI HOLDINGS, LLC 9,10 2 AC 4771 SF 4425 SF 1.89 AC 5672 SF 3369 SF 5333 SF NO 046 241 FRALEY, LLC 9 14300 SF 2278 SF 12022 SF 1100 SF 770 SF NO 047 241 FRALEY, LLC 9 16900 SF 2623 SF 14277 SF 1300 SF 910 SF 910 SF NO 048 DUMFRIES PLAZA EXXON, LTD. 9 0.895 AC 5771 SF 0.763 AC 3000 SF 2100 SF 487 SF NO 049 APARTMENTS AT DUMFRIES II, LLC 10,11 6.124 AC 1.262 AC 3972 SF 4.861 AC 5752 SF 7623 SF 14091 SF 0 NO | | | 0 | | | | | | | | | | - | | | | | | | |
| 045 WNI HOLDINGS, LLC 9,10 2 AC 471 SF 4425 SF 1.89 AC 5672 SF 3369 SF 5333 SF NO 046 241 FRALEY, LLC 9 14300 SF 2278 SF 12022 SF NO 047 241 FRALEY, LLC 9 16900 SF 2623 SF 14277 SF 1300 SF 910 SF NO 048 DUMFRIES PLAZA EXXON, LTD. 9 0.895 AC 5771 SF 0.763 AC 3000 SF 2100 SF 487 SF NO 049 APARTMENTS AT DUMFRIES II, LLC 10,11 6.124 AC 1.262 AC 3972 SF 4.861 AC 5752 SF 7623 SF 14091 SF 0 NO | | | 0 | | | | | 1 | | | | | | | | | | <u> </u> | | |
| 046 241 FRALEY, LLC 9 14300 SF 2278 SF 12022 SF NO 047 241 FRALEY, LLC 9 16900 SF 2623 SF 14277 SF 1300 SF 1300 SF 910 SF NO 048 DUMFRIES PLAZA EXXON, LTD. 9 0.895 AC 5771 SF 0.763 AC 3000 SF 2100 SF 487 SF NO 049 APARTMENTS AT DUMFRIES II, LLC 10,11 6.124 AC 1.262 AC 3972 SF 4.861 AC 5752 SF 7623 SF 14091 SF NO | | | 0 10 | | | | | 1 | | | | | - | | | | | | | |
| 047 241 FRALEY, LLC 9 16900 SF 2623 SF 14277 SF 9 1300 SF 910 SF 910 SF NO 048 DUMFRIES PLAZA EXXON, LTD. 9 0.895 AC 5771 SF 0.763 AC 3000 SF 2100 SF 487 SF NO 049 APARTMENTS AT DUMFRIES II, LLC 10,11 6.124 AC 1.262 AC 3972 SF 4.861 AC 5752 SF 7623 SF 14091 SF NO | | | 9,10 | | | 4423 86 | | 1 | | | | | | | | | | 3333 SF | | |
| 048 DUMFRIES PLAZA EXXON, LTD. 9 0.895 AC 5771 SF 0.763 AC 1 3000 SF 3000 SF NO 049 APARTMENTS AT DUMFRIES II, LLC 10,11 6.124 AC 1.262 AC 3972 SF 4.861 AC 5752 SF 7623 SF 14091 SF NO | | | 0 | | | | | + | | | | | | | | | | | | |
| 049 APARTMENTS AT DUMFRIES II, LLC 10,11 6.124 AC 1.262 AC 3972 SF 4.861 AC NO | | | 0 | | | | | 1 | | | | | | | | | | /27 SE | | |
| | | | 10.11 | | | 3072 SE | | + | | | 5752 SE | 7623 SE | | | | | 2100 SF | 1 0/ 3 Γ | | |
| 000 CONTRIBUTED CENTER, LEC 0000 SF 2903 SF 10,11 2.940 MC 0000 SF 10,11 2.940 MC 1000 MC | | | 1 ' | | | 37/2 31 | | + | | | | 1023 31 | | | | | 330 CE | 2063 SE | | 1 |
| | 0.50 | COMMILICE CENTER, LEC | 10,11 | 2.770 AC | J027 SI | 1 | 2.013 AC | | | [| 0201 31 | | 0555 81 | | | | 557 BI | 2703 BI | | |

PROJECT SHEET NO. 1C(1)

dII948I0lc2.dgn Plotted By:ong

PROJECT MANAGER <u>Hodinam Nguyen</u>, P.E.(703) 792-8161 PWC <u>Dept. of Transportation</u>
SURVEYED BY, DATE <u>Leon E.Treutle LS</u> (703)259-3224 7/17/13

DESIGN BY <u>JMT Engineering</u> (804) 323-9900

SUBSURFACE UTILITY BY, DATE <u>Leon E.Treutle LS</u> (703)259-3224 7/17/13

PRELIMINARY RIGHT OF WAY DATA SHEET

| REVISED | STATE | | STATE | SHEET NO. |
|---------|-------|-------|---------------------------------|-----------|
| | SIAIE | ROUTE | PROJECT | SHEET NO. |
| | VA. | 1 | 0001-212-249 , RW-201 | IC(2) |

DESIGN FEATURES RELATING TO CONSTRUCTION OR TO REGULATION AND CONTROL OF TRAFFIC MAY BE SUBJECT TO CHANGE AS DEEMED NECESSARY BY THE DEPARTMENT

| | | | | | | | | | AREA | | | | | | | | |
|--------------------------------------|---|-----------|-----------------------|----------------------|------------------|--------------------------|---------------------------------------|----------------------------------|---------------|------------------|----------|--------------------|--------------------|--------------------|----------|----------------------------------|---------------|
| | | | | | | | PERMANENT EASEMENTS UTILITY TEMPORARY | | | | | | ORARY CONSTRUCTION | .RY CONSTRUCTION | | | |
| PARCEL NO. | LANDOWNER | SHEET NO. | TOTAL | FEE TAKING | PRESCRIPTIVE R/W | FEE REMAINDER | DRAINAGE & SLOPE | DRAINAGE SLC & RET. WALL RET. | PE & DRAINAGE | SLOPE | JOINT | COLUMBIA | DOMINION | VERIZON | MISC. | TEMPORARY TEMPORAR ENTRANCE WALL | RY PROFFERS |
| | | | A CRES OR SQ. FEET | ACRES OR SQ. FEET | SQ. FEET | A CRES OR SQUARE FEET | | ACRES O | R SQ. FEET | | | ACRES O | R SQ. FEET | | | A CRES OR SQ. FEET | YES/NO |
| 051 THE TOWN | N OF DUMFRIES | 11 | 3.351 AC | 687 SF | | 3.335 AC | | | | 900 SF | 2383 SF | | | | | | NO |
| 052 NOT USED | | ** | 0.001110 | 007.51 | | 0.000 110 | | | | 300 81 | 25 65 51 | | | | | | |
| | S PURCHASER, LLC | 11,12 | 4.199 AC | 42620 SF | | 3.22 AC | 35970 SF | | | 288 SF | 25885 SF | | | | | | NO |
| | C VALLEY CHURCH OF CHRIST | 11 | 6.408 AC | 3578 SF | | 6.326 AC | 10668 SF | | 370 SF | | | | | 5951 SF | | | NO |
| | S VLADIMIR CORTEZ & DANILA JANNETTE GONZALEZ | 11 | 0.517 AC | 22534 SF | | 0 AC | | | | | | | | | | | NO |
| | S PURCHASER, LLC | 12 | 0.3 AC | | | 0.3 AC | 1109 SF | | | | 104 SF | | | | | | NO |
| 057 DUMFRIES | S PURCHASER, LLC | 12,13 | 0.24 AC | 25 SF | | 0.24 AC | 1461 SF | | | | 601 SF | | | | | | NO |
| 058 DUMFRIES | S PURCHASER, LLC | 12,13 | 0.282 AC | 671 SF | | 0.266 AC | 1125 SF | | | 756 SF | 909 SF | | | | | | NO |
| 059 ARMANDO | O CASTRO ESCOBAR & EFREN CASTRO ESCOBAR | 11,12 | 1.356 AC | 1.356 AC | | 0 AC | | | | | | | | | | | NO |
| 060 WILLARD | L. MOUNTJOY & BETTY B. MOUNTJOY | 12 | 1.46 AC | 2018 SF | | 1.414 AC | | | | 4671 SF | | | | 5908 SF | | 1270 SF | NO |
| 061 DUMFRIES | S PURCHASER, LLC | 13 | 0.281 AC | 1548 SF | | 0.246 AC | | | | 1495 SF | 1458 SF | | | | | | NO |
| 062 DUMFRIES | S PURCHASER, LLC | 13 | 0.267 AC | 1753 SF | | 0.226 AC | | | | 1073 SF | 1756 SF | | | | | | NO |
| 063 ACTION IN | N COMMUNITY THROUGH SERVICE OF PRINCE WILLIAM | 13 | 0.545 AC | 1556 SF | | 0.51 AC | | | 3 SF | 657 SF | 1861 SF | | | | 459 SF | | NO |
| 064 ACTION IN | N COMMUNITY THROUGH SERVICE OF PRINCE WILLIAM | 13 | 0 AC | 1395 SF | | -0.032 AC | | | 451 SF | 65 SF | 1123 SF | | | | 1008 SF | | NO |
| 065 ACTION IN | N COMMUNITY THROUGH SERVICE OF PRINCE WILLIAM | 13 | 0 AC | 668 SF | | -0.015 AC | | | 566 SF | | 39 SF | | | | 1300 SF | | NO |
| 066 FOLKS FA | AMILY TRUST & THE CHARLES EDWIN FOLKS REVOCABLE LIVING | TR 13 | 0.387 AC | 16836 SF | | 0 AC | | | 1262 SF | | 2078 SF | | | | | | NO |
| 067 HOSSEIN F | PAHLAVANINEJAD & DANA ANN PAHLAVANINEJAD | 13,14 | 0.27 AC | 11761 SF | | 0 AC | | | 1589 SF | | 2428 SF | | | | | | NO |
| 068 POSSUM P | POINT PROPERTIES, LLC | 13 | 58363 SF | 5970 SF | | 52393 SF | | | | | | | | | 719 SF | 1479 SF | NO |
| 069 ALI PAHLA | AVANI, TRUSTEE | 13 | 0.333 AC | 14520 SF | | 0 AC | | | | | | | | | | | NO |
| 070 ALI PAHLA | AVANI, TRUSTEE | 13,14C | 0.7 AC | 2719 SF | | 0.638 AC | | | 2383 SF | | 2789 SF | | | | 1070 SF | | NO |
| 071 DUMFRIES | S PURCHASER, LLC | 13,14,14C | 3.625 AC | 9870 SF | | 3.398 AC | | | | 1685 SF | 2784 SF | | | | 3662 SF | | NO |
| | VILLAGE, LLC | 13,14 | 1.009 AC | 546 SF | | 0.997 AC | | | | | | | 696 SF | | 702 SF | | NO |
| | VILLAGE, LLC | 14 | 1.161 AC | 1297 SF | | 1.131 AC | | 883 | 5 SF | | | | | | | 1226 SF | NO |
| 074 JACKSAN, | , | 14,15 | 1.953 AC | | | 1.953 AC | | | | | | 4937 SF | | | | | NO |
| | AH FAMILY LIMITED PARTNERSHIP | 14,14C | 1.795 AC | 698 SF | | 1.779 AC | | 487 | 0 SF | | 10552 SF | | | | 1889 SF | | NO |
| | DUMFRIES | 14 | 0.086 AC | | | 0.086 AC | | | | 402 SF | 630 SF | | | | | | NO |
| | EQUITY 1, LLC | 14,15 | 2.878 AC | 178 SF | | 2.874 AC | | | OSF | 1024 SF | 2085 SF | | | | | | NO |
| 078 MANZUR | | 14,15 | 1.301 AC | 1619 SF | | 1.263 AC | | 223 | 2 SF | | 6037 SF | | | | | | NO |
| | POINT PROPERTIES, LLC | 14C | 0.53 AC | 1376 SF | | 0.498 AC | | | | | | | | | 258 SF | | NO |
| 080 NOT USED | | | | | | | | | | | | | | | | | $\overline{}$ |
| 081 NOT USED | | | | | | | | | | | | | | | | | |
| 082 NOT USED | | | | | | | | | | | | | | | | | |
| 083 NOT USED | | 1.7 | 1.574.40 | | | 1.574.40 | | | 2207 GE | | | 2064.05 | | | | 4002 CF | 110 |
| 084 QUINNJAN | | 15 | 1.574 AC | | | 1.574 AC | 000 GE | | 2305 SF | | | 2064 SF | | | | 4082 SF | NO |
| | J. NEUBERGER, TRUSTEE | 15 | 18123 SF | | | 18123 SF | 809 SF | | | | | 2819 SF | | | | 2114 SF | NO |
| | HODGES-MOORE | 15 | 15300 SF | | | 15300 SF | 538 SF | | | | | 1513 SF | | | | 2693 SF | NO |
| | HODGES-MOORE | 15 | 15300 SF | 149 SF | | 15300 SF | 535 SF 544 SF | | | | 598 SF | 1510 SF 1023 SF | 107 SF | | 457 SF | 2242 SF 579 SF | NO |
| | FOOING, INC. NS COUNSELING & CONSULTING, INC. | 15 | 0.452 AC 20189 SF | 149 SF | | 0.448 AC 20189 SF | 661 SF | | | | 2235 SF | 1023 SF | 10/ SF | | 5826 SF | 3/9 SF | NO NO |
| | , | 15 | | 16988 SF | 2216 SE | | 001 SF | | | 556 CE | 2233 SF | | | 1/21 SE | 3820 SF | | |
| 090 C & J II, LL 091 FOLKS FA | | TR 15,15C | 0.39 AC 0.801 AC | 16988 SF 34892 SF | 3316 SF | 0 AC 0 AC | | | | 556 SF 181 SF | | | | 1421 SF 724 SF | | | NO NO |
| | AMILY TRUST & THE CHARLES EDWIN FOLKS REVOCABLE LIVING RY R. ALTOMARE | 15,15C | 0.801 AC 0.232 AC | 10105 SF | | 0 AC | | | | 101 5 | | | | /2 4 SF | | | NO |
| | RY R. ALTOMARE | 15,15C,16 | 1.098 AC | 21377 SF | 2393 SF | 0.607 AC | | 111 | 5 SF | | | | | 712 SF | 714 SF | | NO |
| | GIRLS CLUBS OF GREATER WASHINGTON, INC. | 15,15C,16 | 1.098 AC 1.653 AC | <u> </u> | 2373 δΓ | 1.653 AC | + | 44 , |) DI | + | | | | /12 S F | / 14 51 | 2084 SF | NO |
| | NS COUNSELING & CONSULTING, INC. | 16 | 1.033 AC 15125 SF | | | 15125 SF | 683 SF | | | | 1870 SF | | | | | 2007 81 | NO |
| | AMIDI & RAMZIA HAMIDI | 16 | 1.979 AC | | <u> </u> | 1.979 AC | 003.31 | 240 SF | 2034 SF | | 10/0 31 | 4995 SF | | | 1464 SF | 1555 SF | NO |
| | ER HOTELS, LLC | 16 | 1.577 AC | 1251 SF | | 1.492 AC | | 2809 SF | 2034 51 | | | 4323 SF | | | 915 SF | 504 SF | NO |
| | PROPERTIES, INC. | 16 | 1.321 AC 1.182 AC | 3569 SF | | 1.492 AC 1.1 AC | + | 251 SF | | | | 3383 SF | | | 719 SF | 2760 SF | NO |
| | S CAR CARE, LLC | 16,17 | 1.162 AC 1.35 AC | 1096 SF | | 1.1 AC 1.325 AC | + - | 231 31 | 152 SF | | | 4044 SF | | | 2031 SF | 2700 01 | NO |
| 100 LUCKSHM | · | 16,17 | 4.271 AC | 9525 SF | | 4.053 AC | + | 615 | 1 SF | | | וט דדטו | | 9302 SF | 2031 01 | 3766 SF 6433 SF | |
| 100 LUCKSIIM | II, LLC | 10,17 | 7.2/1 AC | J323 BI | I | T.033 AC | 1 | 013 | 1 01 | | <u> </u> | 1 | 1 |)302 BI | <u> </u> | 3700 SI 0433 SI | |

PROJECT SHEET NO. 1C(2)

PROJECT MANAGER <u>Hodinam Nguyen</u>, P.E.(703) 792-8161 PWC <u>Dept. of Transportation</u>
SURVEYED BY, DATE <u>Leon E.Treutle LS</u> (703)259-3224_7/17/13

DESIGN BY <u>JMT Engineering</u> (804) 323-9900

SUBSURFACE UTILITY BY, DATE <u>Leon E.Treutle LS</u> (703)259-3224_7/17/13

PRELIMINARY RIGHT OF WAY DATA SHEET

| REVISED | STATE | | STATE | SHEET NO. |
|---------|-------|-------|---------------------------------|-----------|
| | SIAIL | ROUTE | PROJECT | SHEET NO. |
| | VA. | 1 | 0001-212-249 , RW-201 | IC(3) |

DESIGN FEATURES RELATING TO CONSTRUCTION OR TO REGULATION AND CONTROL OF TRAFFIC MAY BE SUBJECT TO CHANGE AS DEEMED NECESSARY BY THE DEPARTMENT

| | | | | | | | | | | AREA | | | | | | | | |
|---------------|--|--------|----------------------|-------------------|------------------|--------------------------|---------------------|------|-------------------|----------|--------|---------|-------------------|---------|---------|-----------------------|-------------------|----------|
| | | | | | | | | PERI | MANENT EASEME | ENTS | | | UTILITY | | TEMP | ORARY CONSTR | UCTION | |
| PARCEL NO. | LANDOWNER | ET NO. | TOTAL | FEE TAKING | PRESCRIPTIVE R/W | FEE REM A INDER | DRAINAGE & SLOPE | | SLOPE & RET. WALL | DRAINAGE | SLOPE | JOINT | COLUMBIA DOMINION | VERIZON | MISC. | TEMPORARY ENTRANCE | TEMPORARY WALL | PROFFERS |
| | | | ACRES OR SQ. FEET | ACRES OR SQ. FEET | SQ. FEET | A CRES OR SQUARE FEET | | A | CRES OR SQ. FEE | ET | | | ACRES OR SQ. FEET | | , | ACRES OR SQ. FE | ET | YES/NO |
| 101 | SUBLIME INVESTMENTS, INC. | 17 | 1.235 AC | | | 1.235 AC | | | | 219 SF | | | 3500 SF | | 1561 SF | | | NO |
| 102 | T-COURT INVESTMENTS, LLC | 17 | 0.299 AC | | | 0.299 AC | | | | 126 SF | | | 1236 SF | | | 510 SF | | NO |
| 103 | T-COURT INVESTMENTS, LLC | 7,18 | 0.83 AC | | | 0.83 AC | | | | | | | 3874 SF | | | 1076 SF | <u> </u> | NO |
| 104 | AZAR OF VIRGINIA, LLC | 17 | 1.879 AC | 1696 SF | | 1.84 AC | | | 2854 SF | | | 2953 SF | | 7092 SF | | 1609 SF | 1993 SF | NO |
| 105 | NOT USED | | | | | | | | | | | | | | | | | |
| 106 | NOT USED | | | | | | | | | | | | | | | | | |
| 107 | NOT USED | | | | | | | | | | | | | | | | | |
| 108 | NOT USED | | | | | | | | | | | | | | | | | |
| 109 | NOT USED | | | | | | | | | | | | | | | | | |
| 110 | QUANTICO PROPERTY, LLC | 1,5 | 2.366 AC | 1607 SF | | 2.329 AC | | | | 2297 SF | | | | | | | | NO |
| 111 | NOT USED | | | | | | | | | | | | | | | | | |
| 112 | MANDERFIELD FAMILY, LLC | 9 | 0.28 AC | 1391 SF | | 0.248 AC | | | | 560 SF | | 800 SF | | | | | | NO |
| 113 | DUMFRIES PURCHASER, LLC | 12 | 0.564 AC | | | 0.564 AC | | | | | 289 SF | | | | | | | NO |
| 114 | NOT USED | | | | | | | | | | | | | | | | | |
| 115 | NOT USED | | | | | | | | | | | | | | | | | |
| 116 | NOT USED | | | | | | | | | | | | | | | | | |
| 117 | NOT USED | | | | | | | | | | | | | | | | | |
| 118 | HIGHLAND PARK AT TOWNSQUARE HOMEOWNERS ASSOCIATION, INC. | | 4.627 AC | 9515 SF | | 4.409 AC | 2011 SF | | | | | 2159 SF | | | | | | NO |
| 119 | TOWNSQUARE AT DUMFRIES, LLC | | 29827 SF | | | 29827 SF | 3330 SF | | | | | 1260 SF | | | | | | NO |
| 120 | TOWNSQUARE AT DUMFRIES BOND, LLC 5, | ,6,7 | 13.747 AC | | | 13.747 AC | 22473 SF | | | | | | | | | | | NO |
| 121 | TOWNSQUARE AT DUMFRIES, LLC | 7C | 3.108 AC | 449 SF | | 3.098 AC | | | | | | | | 808 SF | | 1308 SF | | NO |
| 122 | NOT USED | | | | | | | | | | | | | | | | | |
| 123 | | | 1.052 AC | 2739 SF | | 0.989 AC | | | | | | | | | | | | NO |
| 124 | EQUESTRES, INC. | 8 | 1.094 AC | | 1768 SF | 1.094 AC | | | | | | 366 SF | | | | | | NO |

PROJECT SHEET NO. 1C(3)

| | PROJECT MANAGER <u>Hodinam Nguyen</u> , P.E. (703) 792-8161 PWC Dept. of Transportation SURVEYED BY, DATE <u>Leon E. Treutle LS (703)259-3224 7/17/13</u> | | | DESIGN FEATURES RELATING TO CONSTRUCTION | REVISED STATE ROUTE | STATE SHEET NO |
|--|--|--|--|--|--|-----------------------------------|
| | DESIGN BY <i>JMT Engineering (804) 323-9900</i> | Existing Drainage Descriptions | | MAY BE SUBJECT TO CHANGE AS DEEMED | VA. / | |
| First Conc. Pipe Inv. In (1) = 160.56 Not Found Control Device Inv. In (1) = 160.56 Not Found Not Found Inv. In (1) = 160.56 Inv. In (2) = 160.56 Inv. In (3) = 160.56 Inv. In (3) = 160.56 Inv. In (4) = 160.56 Inv. In (5) = 160.56 Inv. In (6) = 160 | DESIGN BY ASE Explored by 128 392 1 | 10 Pt. Di | Rilm - 58.01 Rilm - 68.03 Rilm - 68.03 Rilm - 68.05 Rilm - 19.05 Rilm - 68.05 Rilm - 19.05 Rilm - 68.05 Rilm - 19.05 Rilm | OR TO RECULATION AND CONTROL. OF TRAFFIC MAY BE SUBJECT TO CHANGE AS DEEMED NECESSARY BY THE DEPARTMENT OBB | In Pl. 12" CMP Inv. In = 36.57 Inv. Out = 32.73 In Pl. 12" Conc. Pipe Inv. In = 31.58 Inv. Out = 31.10 In Pl. 15" Conc. Pipe Inv. In = 25.92 Inv. Out = 25.27 Inv. Out = 25.27 Inv. Out = 25.27 Inv. Out = 39.97 Inv. Out = 39.97 Inv. Out = 39.98 Inv. Out = 32.84 In Pl. 18" Conc. Pipe Inv. Out = 34.16 Inv. Out = 34.16 Inv. Out = 34.16 Inv. Out = 44.50 In Pl. 15" Conc. Pipe Inv. Out = 44.50 Inv. In = 39.88 In Pl. 18" Conc. Pipe Inv. Out = 37.41 Inv. In = 39.85 Inv. In = 36.82 In Pl. 18" Conc. Pipe Inv. Out = 37.41 In Pl. 18" Conc. Pipe Inv. Out = 37.41 In Pl. 18" Conc. Pipe Inv. Out = 37.41 In Pl. 18" Conc. Pipe Inv. Out = 37.41 In Pl. 15" Conc. Pipe Inv. Out = 31.66 In Pl. 54" Conc. Pipe Inv. Out = 31.66 In Pl. 54" Conc. Pipe Inv. Out = 28.11 In Pl. 28.31 In Pl. 10" PVC Pipe Inv. Out = 28.31 In Pl. 10" Endwall In Pl. 48" Conc. Pipe Inv. Out = 28.31 In Pl. 10" Endwall In Pl. 56" Conc. Pipe Inv. Out = 28.31 In Pl. 10" Endwall In Pl. 56" Conc. Pipe Inv. In = 38.53 Inv. Out = 23.28 In Pl. Endwall In Pl. 56" Conc. Pipe Inv. In = 23.28 In Pl. Endwall In Pl. 56" Conc. Pipe Inv. In = 23.28 In Pl. Endwall In Pl. 56" Conc. Pipe Inv. In = 23.28 In Pl. Endwall In Pl. 56" Conc. Pipe Inv. In = 23.28 In Pl. Endwall In Pl. 56" Conc. Pipe Inv. In = 23.28 In Pl. Endwall In Pl. 56" Conc. Pipe Inv. In = 38.53 Inv. Out = 23.28 In Pl. Endwall In Pl. 56" Conc. Pipe Inv. In = 38.51 Inv. In = 18.74 Inv. In = 18.74 Inv. In = 18.74 Inv. In = 18.75 Inv. Inv. Inv. Inv. Inv. Inv. Inv. Inv. | PROJECT SHEET NO 0001-212-249, . |
| PROJECT SHEE | Inv. Out = 153.07 Inv. Out = 153.07 Inv. Out = 153.07 Inv. In Pl. Dl Inv. In (a) = 139.66 Inv. In (b) = 131.41 Inv. Out (c) = 138.26 In Pl. 15" Conc. Pin Pl. 48" Conc. Pipe | Inv. In (a) = 160.56 Inv. In (b) = 160.66 In PI. 15" Conc. Pipe Inv. Out = 150.16 Inv. Outfall FES = 148.89 Inv. Outfall FES = 148.89 Inv. In (a) = Recessed Pipe Inv. Out (c) = Recessed Pipe Previous Structure Not Found In PI. 15" Conc. Pipe Inv. In (d) = Recessed Pipe In PI. 48" Conc. Pipe Inv. Out (e) = Recessed Pipe | Control Device Rim = 62.68 Inv. In = 59.08 In Pl. 24" Conc. Pipe Inv. Out = 59.04 Full Of Debris | 102 In Pl. 12" CPP Inv. In = 38.69 | In Pl. Endwall III9 In Pl. Dl Rim = 25.54 In Pl. I2" Conc. Pipe | PROJECT SHEET NO. |

| PROJECT MANAGER <u>Hoainam Nguyen, P.E.(703) 792-8161 PWC</u> SURVEYED BY DATE <u>Leon E.Treutle LS</u> (703)259-3224 | <u>/C_Dept. of Transportation</u> 4 7/17/13 | | | | | DESIGN FEATURES RELATING TO CONS | STATE | STATE SHEET NO. |
|--|--|--|--|--|--|---|--|-------------------|
| DESIGN BY _ JMT _Engineering (804) 323-9900 SUBSURFACE UTILITY BY, DATE _ Leon E.Treutle LS (70 | | Exis | ting Drainage De | scriptions | | MAY BE SUBJECT TO CHANGE AS DEE NECESSARY BY THE DEPARTMENT | WED VA. / OOO/- | 212-249, IE(2) |
| SURVEYED BY _ MTE _ LEGAL S. TOURS S. 2224. DESIGN BY _ MTE _ LEGAL S. 332-9900 | 135 | Family F | ## 15.00 ## 10. | In Pl. D Rim * 21.24 Upstream Structure Not Found Inv. In * 18.74 In Pl. D Rim * 19.27 In Pl. D Rim * 19.27 In Pl. D Rim * 18.30 In Pl. D Rim * 18.30 In Pl. D Rim * 18.30 In Pl. D Rim * 27.35 In Pl. D Rim * 27.35 In Pl. D Rim * 23.90 In Pl. D Rim * 26.64 Structure Full Of Sediment In Pl. D Rim * 26.64 Structure Full Of Sediment Baltom * 23.64 In Pl. D Rim * 24.34 In Pl. D Rim * 23.64 In Pl. D Rim * 27.09 In Pl. D Rim * 27.09 In Pl. D Rim * 23.97 In Pl. D Rim * 22.96 In Pl. D Rim * 22.96 In Pl. D Rim * 26.31 Inv. In (2) * 22.96 In Pl. D Rim * 26.32 Inv. In (2) * 22.96 In Pl. D Rim * 26.31 Inv. In (2) * 22.96 In Pl. D Rim * 26.32 Inv. In (2) * 22.96 In Pl. D Rim * 26.31 Inv. In (2) * 22.96 In Pl. D Rim * 26.31 Inv. In (2) * 22.96 In Pl. D Rim * 26.31 Inv. In (2) * 22.96 In Pl. D Rim * 26.30 In Pl. D Rim * 26.31 Inv. In (2) * 22.96 In Pl. B In Pl. BO' Conc. Pipe Inv. Out * 23.60 In Pl. D In Pl | 192 In. P.I. SSMH Rim = 28.66 Inv. In. (a) = Recessed Pipe Inv. In. (b) = Recessed Pipe Inv. In. (c) = Recessed Pipe Inv. Dut (c) = Recessed Pipe Inv. Dut (c) = Recessed Pipe Inv. In. (a) = Recessed Pipe Inv. In. (a) = Recessed Pipe Inv. In. (a) = Recessed Pipe Inv. Dut (c) = Recessed Pipe Inv. Out (c) = Recessed Pipe Inv. Out (c) = Recessed Pipe Inv. Out (c) = Retention Pi. Endwall 194 | OR TO REGULATION AND CONTROL OF MAY BE SUBJECT TO CHANGE AS DEE | TRAFFIC ROUTE VA. / OOO/- | 212-249, : IE(2) |
| INV. UUT = 19.47 | Rim = 14.42 In Pl. 18" Conc. Pipe Inv. Out = 12.27 | [162] In Pl. Dl Rim = 21.39 In Pl. 15" Conc. Pipe Inv. Out = 17.99 | Inv. In = 19.37 In Pl. 24" Conc. Pipe Inv. Out = 16.33 Inv. Outfall FES = 15.98 | Upstream Structure Not Found In PI. 15" Conc. Pipe Inv. In (a) = 22.56 In PI. 15" CMP Inv. In (b) = 26.16 In PI. 15" Conc. Pipe Inv. Out (c) = 22.51 | Inv. In (b) = 19.41 In Pl. 24" CPP Inv. Out (c) = 19.26 Downstream Structure Not Found | Retention Area Bottom = 27.30 | 236 In Pl. SSMH Rim = 77.06 (Reverse Flow In Structure) Inv. In (a) = 71.76 Inv. In (b) = 72.46 In Pl. 24" Conc. Pipe | |
| | | | | | | | Inv. Out (c) = 71.96 | PROJECT SHEET NO. |
| | | | | | | | O | 001-212-249 IE(2) |

PROJECT MANAGER <u>Hoainam Nguyen, P.E.(703)792-8161 PWC Dept. of Transportation</u>
SURVEYED BY, DATE <u>Leon E.Treutle LS (703)259-3224_7/17/13</u>

DESIGN BY <u>JMT Engineering (804) 323-9900</u>

SUBSURFACE UTILITY BY, DATE <u>Leon E.Treutle LS (703)259-3224_7/17/13</u>

Existing Drainage Descriptions

 REVISED
 STATE
 STATE
 PROJECT
 SHEET NO.

 VA.
 /
 0001-212-249, RW-201, C-501
 :
 /E(3)

| DESIGN FEATURES RELATING TO CONSTRUCTION |
|--|
| OR TO REGULATION AND CONTROL OF TRAFFIC |
| MAY BE SUBJECT TO CHANGE AS DEEMED |
| NECESSARY BY THE DEPARTMENT |

| | | | | | | Mı | IR TO REGULATION AND CONTROL OF TRAFFIC IAY BE SUBJECT TO CHANGE AS DEEMED IECESSARY BY THE DEPARTMENT |
|---|--|--|---|--|---|---|---|
| 237 In Pl. Endwall In Pl. 15" Conc. Pipe Inv. Out = 77.13 Inv. Outfall = 74.99 238 In Pl. Dl | 252 In PI. DI Rim = II4.23 Inv. In (a) = Recessed Inv. In (b) = Recessed In PI. 24" Conc. Pipe Inv. Out = Recessed | 67 In PI. DI Rim = II2.07 In PI. 8" PVC Pipe From BIdg. Inv. In (a) = I09.82 In PI. 8" PVC Pipe From BIdg. Inv. In (b) = I09.82 | 284 In Pl. Dl Rim = II8.06 In Pl. 8" PVC Pipe From Bldg. Inv. In (a) = IIO.06 Inv. In (b) = I07.81 In Pl. 30" Conc. Pipe | 299 In Pl. Dl Rim = 106.21 Inv. In = Recessed In Pl. 15" Conc. Pipe Inv. Out = 100.41 | 314 In Pl. Dl Rim = 98.81 Inv. In (a) = 90.11 Inv. In (b) = 90.11 In Pl. 18" Conc. Pipe Inv. Out (c) = 86.51 | 330 In Pl. SSMH Rim = 91.06 In Pl. 15" Conc. Pipe Inv. In = 85.61 In Pl. 15" Conc. Pipe Inv. Out = 78.58 | 347 In Pl. Dl Rim = 91.85 In Pl. 24" Conc. Pipe Inv. In (a) = 84.87 (Upstream Not Found) In Pl. 18" Conc. Pipe |
| <u>238</u> In Pl. Dl Rim = III.04 In Pl. 18" CPP Inv. In = 106.49 | Bottom = 106.33 | In Pl. 15" CPP Inv. Out (c) = 109.77 | Inv. Out = 107.81 285 In Pl. Dl | 300 In Pl. Storm Vault Rim = 109.37 In Pl. 18" Conc. Pipe | 315 In Pl. Dl Rim = 99.25 | 331 In Pl. Dl Rim = 41.85 | Inv. In (b) = 88.25 In Pl. 24" Conc. Pipe Inv. Out (c) = 84.36 |
| In Pl. Recessed Pipe Inv. Out = 105.89 239 In Pl. SSMH Rim = 110.20 | Rim = 113.57 Inv. In = 106.77 In Pl. 24" Conc. Pipe Inv. Out = 106.77 | 68 In Pl. Dl Rim = 110.62 Inv. In = 107.22 In Pl. 15" CPP Inv. Out = 107.17 | Rim = 116.35 Inv. In = Recessed In PI. Recessed Pipe Inv. Out = Recessed Bottom = 107.35 | Bottom = 99.97 301 In Pl. Dl Rim = 104.35 Inv. In (a) = 99.75 | In Pl. 15" Conc. Pipe Inv. In = 91.90 In Pl. 18" Conc. Pipe Inv. Out = 91.80 | In Pl. 15" Conc. Pipe Inv. = 39.00 In Pl. Dl Rim = 41.19 | 348 In Pl. Drainage Control Device Rim = 143.51 In Pl. 21" Conc. Pipe Inv. Out = 133.36 |
| Inaccessible 240 In Pl. SSMH W/Control Device Rim = 110.46 | 254 In Pl. Dl Rim = II2.09 Inv. In = I06.54 In Pl. 18" Conc. Pipe Inv. Out = I06.54 | 69 In PI. DI Rim = III.35 Inv. In = 106.65 In PI. 15" CPP Inv. Out = Inaccessible | 286 In Pl. Dl Rim = 114.23 In Pl. 15" Conc. Pipe Inv. Out = 110.78 | Inv. In (b) = Recessed In Pl. 24" Conc. Pipe Inv. Out (c) = 99.67 | 316 In Pl. Dl Rim = 99.25 Inv. In = 91.09 In Pl. 18" Conc. Pipe Inv. Out = 90.94 | Inv. In = 38,19 In Pl. 15" Conc. Pipe Inv. Out = 38,09 | In Pl. Grated Control Device Rim = 143.15 Bottom = 133.93 |
| Bottom = 97.36 241 In Pl. SSMH W/Control Device Rim = 110.43 In Pl. 36" Conc. Pipe | 255 In Pl. SSMH Rim = 109.44 Inv. In = 103.94 In Pl. 18" Conc. Pipe Possible | | 287 In Pl. Dl Rim = 116.90 In Pl. 15" Conc. Pipe Inv. Out = 112.45 | Rim = 110.40 Bottom = 102.35 In Pl. Dl Rim = 103.50 Inv. In = 99.20 | 317 In Pl. Dl Rim = 97.31 In Pl. 15" Conc. Pipe Inv. Out = 89.86 | 333 In PI. DI Rim = 48.33 In PI. 15" CPP Inv. Out = 45.91 | 350 In Pl. Stormwater Control Device Rim = 143.40 Bottom = 134.35 In Pl. Underground |
| ### 101.21 In Pl. DI Rim = 101.21 Inv. In = 95.86 In Pl. 36" Conc. Pipe | 256 In PI. SSMH Rim = III.78 | P.71 In PI. SSMH Rim = III.27 Bottom = 106.42 | 288 In Pl. Dl Rim = II2.67 Inv. In (a) = Recessed Inv. In (b) = 107.02 | In Pl. 18" Conc. Pipe Inv. Out = 98.76 304 In Pl. Dl Rim = 102.33 | 318 In Pl. Dl Rim = 90.89 Inv. In (a) = Recessed Pipe Inv. In (b) = Recessed Pipe In Pl. 30" Conc. Pipe | 334 In Pl. SSMH Rim = 49.57 Inv. In = Recessed Pipe In Pl. 15" CPP Inv. Out = 45.47 | Retention Vault Rim = 29.78 Bottom = Inaccessible 352 In Pl. Underground Retention Vault |
| In Pl. 36" Conc. Pipe Inv. Out = 88.11 | Bottom = 104.08 | 72 In Pl. SSMH Rim = III.49 Bottom = 104.09 | Inv. In (c) = 108.37 In Pl. 18" Conc. Pipe Inv. Out (d) = 105.92 | Inv. In = 102.33 Inv. In = 97.75 In Pl. 18" Conc. Pipe Inv. Out = 97.63 | Inv. Out (c) = 79.64 [°] 319 In Pl. Dl | 335 In Pl. SSMH Rim = 50.21 Inv. In = 44.61 | Rim = 29.87 Bottom = Inaccessible |
| 243 In Pl. SSMH Rim = 93.02 Inv. In = Inaccessible In Pl. 36" Conc. Pipe Inv. Out = Inaccessible Inv. Outfall = 87.57 | Rim = 109.06 Inv. In (a) = 103.36 Inv. In (b) = 102.74 In Pl. 24" CPP | 73 In PI. SSMH Rim = III.71 Bottom = 103.91 74 In PI. DI | 289 In Pl. Dl Rim = III.43 Inv. In = I04.33 In Pl. Large CMP (72"+) Inv. Out = 100.23 | 305 In Pl. Dl Rim = 103.14 Inv. In = 96.42 In Pl. 21" Conc. Pipe Inv. Out = 96.42 | Rim = 80.88 Inv. In = Recessed Pipe In Pl. 30" Conc. Pipe Inv. Out = 70.63 | In PI. 15" CPP Inv. Out = 44.61 336 In PI. DI Rim = 47.96 | 353 In Pl. Underground Retention Vault Rim = 29.91 Bottom = 18.31 |
| In Pl. Endwall 244 In Pl. 15" Metal Pipe Inv. In = 82,17 | 258 In Pl. Dl Rim = 93.62 Inv. In = 89.12 In Pl. 24" CPP | Rim = 113.24 In Pl. 15" CPP Inv. Out = 109.09 75 In Pl. Dl | 290 In PI. DI Rim = IIO.98 Possible Retention Area Bottom = 99.78 | 306 In Pl. Dl Rim = 103.71 Inv. In = 95.96 In Pl. 24" Conc. Pipe | 320 In Pl. 12" CPP Inv. In = 94.34 Inv. Out = 90.04 | In Pl. 18" CPP Inv. Out = 45.86 | |
| Inv. Out = Buried 245 In Pl. Dl Rim = 94.02 | Inv. Out = 89.02 259 | Rim = 113.53 Inv. In = 108.13 In Pl. 15" CPP Inv. Out = 107.53 | 291 In Pl. Dl Rim = 114.00 In Pl. 15" Conc. Pipe | Inv. Out = 95.86 | 321 In Pl. 18" CMP Inv. In = 89.84 Inv. Out = 84.24 322 In Pl. 18" CMP | Rim = 48.40 Inv. In = Recessed Pipe In Pl. 15" CPP Inv. Out = 44.95 | |
| In Pl. 15" CPP Inv. Out = 90.90 246 In Pl. Dl | Inv. In (b) = Recessed Pipe In Pl. 24" CPP Inv. Out (c) = Recessed Pipe | 76 In Pl. SSMH Rim = 116.18 Bottom = 102.63 | Inv. Out = 108.50 | 307 | Inv. In = 83.68 Inv. Out = 82.37 | 338 In PI. SSMH Rim = 48.53 Inv. In (a) = 44.13 | |
| Rim = 105.33 In Pl. 15" CPP Inv. Out = 102.41 | | 77 In PI. SSMH Rim = 116.23 Bottom = 104.48 | Rim = 112.99 In Pl. Large CMP (72'+) Bottom = 106.27 | <u>308</u> | 323 In Pl. 15" CMP Inv. In = 82.33 Inv. Out = 81.99 In Pl. FES | Inv. In (b) = 44.13 In PI. 15" CPP Inv. Out (c) = 44.08 Downstream Structure Not Found | |
| 247 In Pl. Dl Rim = 99.73 Possible Inv. In = 93.58 Retention In Pl. 15" CPP | Rim = 94.31 Bottom = 84.91 [26] In Pl. SSMH | 78 In Pl. SSMH Rim = 116.31 Bottom = 104.56 | 293 In Pl. Dl Rim = IIO.49 Possible Retention Area Bottom = 99.99 | The Original Survey Storm Structures Have Been Modified Or Removed Along This Area On Dumfries Road Due To Construction. | 324 In Pl. Dl Rim = 81.82 In Pl. 36" Conc. Pipe Inv. In = 73.82 In Pl. 30" Conc. Pipe | 339 In Pl. Dl Rim = 25.72 In Pl. 12" CPP Inv. Out = 22.17 | |
| 11N. Out = 95.51 248 In Pl. Dl Rim = 96.71 | Bottom = 81.99 Possible Retention Rim = 92.70 Rim = 92.70 | 79 In PI. SSMH Rim = 122.78 Bottom = 103.68 | Rim = 107.30 In Pl. 18" Conc. Pipe Inv. In = 98.65 In Pl. 18" Conc. Pipe | 309 | Inv. Out = 73.62 Inv. In = 85.00 | 340 In Pl. 12" CPP Inv. In = 33.94 Inv. Out = 33.90 | |
| Inv. In (a) = 89. Inv. In (b) = 89. In Pl. I5" CPP Inv. In (c) = 89.3 Upstream Structure | Bottom = 82.00 | Rim = 122.94 Bottom = 103.84 | Inv. Out = 97.40 —295 In Pl. Dl | 310 | Inv. Out = 83.61 326 In Pl. Endwall In Pl. 3' X 3' Box Culvert | 341 In Pl.12" Conc. Pipe Inv. = 49.69 End_of Pipe | |
| Not Found In Pl. Recessed Pipe Inv. Out (d) = Inaccessible Possib | In Pl. 15" CPP Inv. Out = 90.45 ble 264 In Pl. SSMH | RII In PI. DI Rim = 118.76 In PI. Pipe (Inaccessible) Possible Top Of Sediment = 115.12 Retention Area | Rim = 103.67 In Pl. Large CMP Exact Size Unknown Possible Underground Retention Area | 311 In Pl. SSMH | Inv. = 85.46 Next Structure Not Found | Not Found 342 | |
| 249 In PI. DI Retent Rim = 94.95 Area Inv. In (a) = 89.60 Inv. In (b) = Recessed Pipe In PI. 18" CPP | Bottom = 98.38 265 In Pl. Dl | 82 In Pl. Dl | Bottom = 94.27 296 | Rim = 104.04 Inv. In = 98.69 In Pl. 18" Conc. Pipe Inv. Out = 97.86 | Rim = 109.28 (Upsream Inv.Approx. 30% Filled Concrete) Inv.In = 104.08 | 343 In Pl. 12" Conc. Pipe Inv. In = 49.05 Inv. Out = 48.08 | |
| Inv. In (c) = 89.35 Downstream Structure Not Found | Rim = 114.27 In Pl. 15" Conc. Pipe Inv. Out = 108.77 | Inv. Out = Inaccessible Top Of Sediment = II3.33 83 In Pl. Dl | 297 In Pl. Dl Rim = 106,19 In Pl. 15" Conc. Pipe Inv. In (a) = 101.04 | 312 In Pl. Dl Rim = 102.29 In Pl. 15" Conc. Pipe Inv. Out = 96.95 | In Pl. 15" Conc. Pipe Inv. Out = 102.33 In Pl. Dl Rim = 109.67 | 344 In Pl. Dl Rim = 154.34 In Pl. 15" Conc. Pipe Inv. Out = 149.59 | |
| 250 In Pl. Dl Rim = 121,85 In Pl. 36" Conc. Pipe Inv. Out = 107,85 | Rim = 108.72 Inv. In = 104.97 Downstream Structure Not Found In PI.18" CMP | Rim = 120.69 Inv. In = 112.54 In Pl. 30" Conc. Pipe Inv. Out = 107.69 | In Pl. 24" Conc. Pipe Inv. In (b) = 103,14 In Pl. 24" Conc. Pipe Inv. Out (c) = 100,47 | 313 In Pl. Dl Rim = 100.41 Inv. In = 93.51 In Pl. 15" Conc. Pipe | Inv. In = 95.82 In Pl. 15" Conc. Pipe Inv. Out = 94.01 (Approx.) Outfall Under Water In Hole | 345 In Pl. Dl Rim = 110.93 In Pl. 15" Conc. Pipe | |
| 251 In Pl. Dl Rim = 118.09 Inv. In = 107.49 In Pl. 36" Conc. Pipe Inv. Out = 107.49 | Inv. Out = 108.77 | | 298 In Pl. Dl Rim = 107.81 In Pl. Pipe (Inaccessible) | Inv. Out = 93.51 | 329 In Pl. Stormwater Control Device Top = 99.62 Inlet = 97.57 | Inv. Out = 106.23 | |
| iii ri. 36 Conc. ripe Inv. Out = 107.49 | | | Top Of Sediment = 102.11 | | וני וב - וסוווו | Rim = 103.56 Bottom = 89.01 Unknown Connectivity | PROJECT SHEET NO. 0001-212-249 IE(3) |
| | | | | | | | ICHED AND HNADDDOVED AND ADE NOT TO DE LICED |

In PI.DI Rim = 60.39

(Off set Pipes) Inv. In (a) = 52.79 Inv. In (b) = 53.74 In Pl. 18" Conc. Pipe Inv. Out (c) = 52.66

Existii

| | STA | ATE L | | SIAIE | SHEET NO | . I |
|---------------------------|----------------|---------|-----------|--------------------------------|-----------|-----|
| | 317 | 'IE R(| OUTE | PROJECT | Janeel NO | |
| ing Drainage Descriptions | V | Α. | / | 0001-212-249, RW-201, C-501 | IE(4) | |
| | ESIGN FEATURES | RELATIN | NG TO CON | ISTRUCTION | | |

OR TO REGULATION AND CONTROL OF TRAFFIC MAY BE SUBJECT TO CHANGE AS DEEMED NECESSARY BY THE DEPARTMENT

| 401 | In PI.DI Rim = 121.79 (Full of Debris) In PI.Size Unknown CMP Inv.Out = 120.01 | 416 | In PI.DI Rim = 62.34 In PI.I5" Conc. Pipe Inv.Out = 59.29 | 431 | In PI. Endwall In PI.15" Conc. Pipe Inv.In = 74.68 In PI. Endwall Inv.Out = 72.70 | 449 | In PI. DI Rim = 82.49 (System Pipe Sizes Do Not Match Observed Water Flow) In PI.15" Conc. Pipe |
|-----|---|--------------|--|---------------|---|----------------|---|
| 402 | In PI. DI Rim = II9.66 (Silt Trap.No Visibility) (Full of Silt) | 417 | In PI.DI Rim = 60.14 (Full of Debris) In PI.15" Conc.Pipe Inv.Out = 54.64 | 432 | In Pl. SSMH Rim = 76.28 In Pl. 15" Conc. Pipe Inv. Out = 72.53 | # 450 | Inv. In (a) = 71.69 Inv. In (b) = 75.34 In PI. 36" Conc. Pipe Inv. Out (c) = 75.04 |
| | Inv. In = II7.86 In Pl. 18" Conc. Pipe Inv. Out = II7.41 | 418 | In PI. DI Rim = 57.71 In PI. 15" Conc. Pipe Inv. Out = 50.74 | 433 | In PI. DI Rim = 75.25 Inv. In = 72.30 | *450 [45] | NOT USED In Pl. Unk Rim = 72.13 (Full of Silt) |
| 403 | In Pl. 18" Conc. Pipe Inv. Outfall = 116.92 | 419 | In PI. DI Rim = 57.66 | | In Pl. 15" Conc. Pipe Inv. Out = 72.20 | | In Pl. 30" Conc. Pipe Inv.Out = Unknown |
| 404 | In PI. 24" Conc. Pipe Inv. = 116.33 | | (Offset Pipes) In Pl. 42" Conc. Pipe Inv. In (a) = 50.36 | <i>*434</i> - | *435 NOT USED | <i>*452-*</i> | 453 NOT USED |
| 405 | In PI.6" CPP Inv. = III.89 | | Inv. In (a) = 50.56 Inv. In (b) = 50.51 In Pl. 42" Conc. Pipe Inv. Out (c) = 50.28 | <u>436</u> | In PI.SSMH Rim = 75.50 Inv.In = 71.03 In PI.I8"Conc.Pipe | 454 | In PI. SSMH Rim = 105. In PI. 24" Conc. Pipe Inv. In (a) = 94.56 |
| 406 | In PI. DI Rim = 64.08 (Full of Debris) Inv. Out = 61.48 | <u>420</u> | In PI.SSMH Rim = 59.21 Inv.In = 49.11 In PI.54" Conc.Pipe | *437 | Inv. Out = 71.03 | | In Pl. 18" Conc. Pipe Inv. In (b) = 96.11 In Pl. 30" Conc. Pipe Inv. Out (c) = 93.91 |
| 407 | In PI. DI Rim = 58.61 (Full of Debris) In.PI.Pipe Size Unknown Inv.In.= 56.61 | 421 | Inv. Out = 47.81 In Pl. SSMH Rim = 10.61 (Moderate Silting and | <u>438</u> | In PI.UNK Underedge Drain Rim = 71.34 | <u>456</u> | In Pl. Dl Rim = 106.12 In Pl. 15" Conc. Pipe Inv. Out = 101.81 |
| 408 | In Pl. 24" Conc. Pipe Inv. Out = 56.61 In Pl. Dl Rim = 60.83 | | Surcharge of approx 0.7feet) (Offset Pipes) Inv. In = 1.96 In Pl. 60" CPP Inv. Out = 1.86 | 439 | In Pl. DI Rim = 89.26 (Full of Debris) In Pl.15" Conc. Pipe | 457 | In PI.DI Rim = 104.47 (In Front of Curb Inlet) Inv. In = 100.77 |
| | Inv. In = 54.83 In Pl. 24" Conc. Pipe Inv. Out = 51.63 | 422 | Next Structure Not Found In Pl. Endwall (Structure Is Completely Buried) | 440 | Inv. Out = 85,56 In Pl. Dl Rim = 84,06 | | In PI.15" Conc. Pipe Inv. Out = 100.72 |
| 409 | In PI. DI Rim = 60.92 Inv. In = 51.64 In PI. 18" Conc. Pipe Inv. Out = 51.55 | <i>423</i> | (Full of Silt) In Pl. 60" CPP Inv. Outfall = -1.17 In Pl. Endwall | | Inv. In = 80.68 In Pl. 15" Conc. Pipe Inv. Out = 80.54 | <i>458</i> | In Pl. DI Rim = 103.09 (In Front of Curb Inlet) (Offset Pipes) Inv. In = 98.89 |
| 410 | In PI.DI Rim = 55.73 (Offset Pipes) Inv.In = 50.98 | πΔJ | (Weir Structure With Double Pipes) (Full of Debris) In Pl. 30" Conc. Pipe Inv. = 5.84 In Pl. Endwall Inv. Out = 4.85 | 441 | In PI. SSMH Rim = 84.60 Inv. In = 80.15 In PI. 15" Conc. Pipe Inv. Out = 76.07 | <u>459</u> | In Pl. 15" Conc. Pipe Inv. Out = 95.46 In Pl. Dl Rim = 94.97 (In Front of Curb Inlet) |
| 411 | In Pl. 18" Conc. Pipe Inv. Out = 50.53 | 424 | In Pl. Endwall (Weir Structure With Double Pipes) (Full of Debris) | <i>442</i> | In Pl. DI Rim = III.66 (In Front of Curb Inlet) | | Inv. In = 91.22 In Pl. 15" Conc. Pipe Inv. Out = 91.17 |
| | Rim = 58.98 (Offset Pipes) Inv. In (a) = 47.48 Inv. In (b) = 48.63 Inv. In (c) = 47.28 | | In Pl. 30" Conc. Pipe Inv. = 5.79 In Pl. Endwall Inv. Out = 4.78 | 443 | In Pl. 15" Conc. Pipe Inv. Out = 108.21 In Pl. Dl | 460 | In PI.DI Rim = 93.35 Inv.In = 90.32 In PI.I8" Conc.Pipe |
| | In Pl. 54" Conc. Pipe Inv. Out (d) = 47.28 | <i>425</i> | In Pl. 15" Plastic Pipe Inv. In = 13.17 Inv. Out = 13.42 | <u> </u> | Rim = 109.68 Inv. In = 104.73 In Pl. 15" Conc. Pipe Inv. Out = 104.68 | [46 <i>l</i>] | Inv. Out = 90.30 |
| 412 | In PI. DI Rim = 48.92 (Offset Pipes) | <i>426</i> | In PI. DI Rim = 21.09 In PI.18" Conc. Pipe | | - *445 NOT USED | 462 | Underedge drain Rim = 100.86 In Pl. UNK |
| | Inv. In = 42,12 In PI. 54" Conc. Pipe Inv. Out = 41,52 | 427 | Inv. Out = 18.31 In Pl. SSMH Rim = 68.35 (Officet Bines) | 446 | In PI.SSMH Rim = 106.25 Inv.In (a) = 90.20 Inv.In (b) = 90.85 In PI.42" Conc.Pipe | [102] | Underedge drain Rim = 86.70 |
| 413 | In Pl. DI Rim = 66.93 In Pl. 18" Conc. Pipe | [40 d | (Offset Pipes) In Pl. 15" Conc. Pipe Inv. Out = 61.97 | 447 | Inv. Out (c) = 88.85 In Pl. SSMH Rim = 100.30 | | |
| | Inv. In = 61.43 In PI. 18" Conc. Pipe Inv. Out = 61.35 In PI. DI | <u>428</u> | In PI. SSMH Rim = 62.98 (Offset Pipes) Inv. In (a) = 59.13 Inv. In (b) = 59.13 | | (Structure On Top Of Another Structure) (Offset Pipes) (Recessed Pipe) | | |
| 414 | In Pl. Dl Rim = 63.20 Inv. In (a) = 59.10 Inv. In (b) = 59.10 In Pl. 18" Conc. Pipe Inv. Out (c) = 59.05 | <i>429</i> | In Pl. 18" Conc. Pipe Inv. Out (c) = 59.03 In Pl. Endwall In Pl. 18" Conc. Pipe | | Inv. In = 80.75 (Recessed Pipe) In Pl. 42" Conc. Pipe Inv. Out = 80.65 | | |
| 415 | In PI. DI | | Inv. Outfall = 58.39 | 448 | In PI.SSMH Rim = 87.03 | | |

430 In Pl. Endwall In Pl. 15" Conc. Pipe Inv. In = 68.84 In Pl. Endwall Inv. Out = 66.27

In PI. SSMH Rim = 87.03 Inv. In = 79.88 In PI. 42" Conc. Pipe Inv. Out = 75.71

| PROJECT 0001-212-249 | SHEET NO |
|-------------------------|----------|
| 0001-212-249 | IE(4) |

VA.

NECESSARY BY THE DEPARTMENT

DESIGN FEATURES RELATING TO CONSTRUCTION OR TO REGULATION AND CONTROL OF TRAFFIC MAY BE SUBJECT TO CHANGE AS DEEMED

ROUTE

REVISED

STATE

0001-212-249,

RW-201, C-501

PROJECT

IF(I)

PROJECT MANAGER Hoainam Nguyen, P.E. (703) 792-8161 PWC Dept. of Transportation SURVEYED BY, DATE _ Leon E.Treutle LS (703)259-3224_7/17/13 _____ SUBSURFACE UTILITY BY, DATE <u>Leon E.Treutle LS (703)259-3224 7/17/</u>13

Survey Horizontal and Vertical Controls

SURVEY ALIGNMENTS

| POINT ID. | STATION | BEARING | PROJECT NORTH (Y) | COORDINATES EAST (X) |
|--------------|-------------------------------|-----------------|----------------------|-------------------------|
| SS | Route I 50+00 . 000 | N 8° 42′ 44" E | 324,376.683 | 3,614,682.736 |
| PI | 56+00.000 | N 8° 42′ 44″ E | 324,969.760 | 3,614,773.619 |
| PI | <i>64+60.</i> 935 | N 15° 42′ 24″ E | 325,820.761 | 3,614,904.026 |
| PI | 83·50 . 3/4 | N 36° 30′ 57" E | 327,639.591 | 3,615,415,505 |
| PI | 91+78 . 517 | N 41° 56′ 44" E | 328,305,212 | 3,6/5,908.323 |
| PI | II5+60 . 75I | N 37°07′27"E | 330,077.070 | 3,617,500.665 |
| PI | 126+73 . 923 | N 44° 34′ 15" E | 330,964.635 | 3,618,172.514 |
| PI | 136+03 . 171 | N 36° II′ 43" E | 331,626.616 | 3,618,824.651 |
| PI | 154+94 . 426 | N 55° 52′ 45″ E | 333,/52.876 | 3,619,941.512 |
| PΙ | <i>160+38.</i> 743 | | <i>333,458.205</i> | 3,620,392.128 |

Virginia Department of Transportation Horizontal Control Control Station I. D. 212 - 2039 Project 001-212-249 **VDOT Project Coordinates** City/County Prince William Date 05-15-2012 East (X) 3614773.620 ft. North (Y) 324969.760 ft.

DETAILED SKETCH (N.T.S.)

Established By Woolpert, Inc. Vertical Datum Based On NAVD88 Elevation 155.365 ft. Horizontal Datum Based On NAD83 (CORS) Zone North South (circle one) Horizontal Closure

Azimuth to Station 212-2040 is 08°42'44" Latitude: 38°33'11.35264" N (5 Decimal Places) Longitude: 77°19'58.74858" W (5 Decimal Places) 1. Reduce the Easting Metric Values By 2.5 Million Geoid Separation (N): -32.365 m

Ellipsoid Height (h): 14.990 m (WGS 84) Control Based On: Station (Name or PID) Project (Monument No.)

Station 56+00.00

Route 1 Survey Alignment

Virginia State Plane Coordinates - NAD 83 Metric Values East (X) 3601719.100 m North (Y) 2099045.038 m Ortho. Elevation (H) 47.355 m

LD-200 (REV. 8/2000) **Virginia Department of Transportation Horizontal Control** Control Station I. D. 212 - 2040 Project 001-212-249 **VDOT Project Coordinates** City/County Prince William Date 05-15-2012 East (X) 3614904.028 ft. Established By Woolpert, Inc. North (Y) 325820.761 ft.

Geoid Separation (N): -32.362 m Meters. The South and North Zone Northing Metric Ellipsoid Height (h): 11.865 m (WGS 84) 2. Multiply These Values by the U. S. Survey Foot (3.280833333) Control Based On: Station (Name or PID)

LD-200 (REV. 8/2000)

Ortho. Elevation (H) 7.879 m

3. Multiply These Values by Combined Scale and Elevation Factor (1.00006) for the County. Reverse This Procedure to Transform VDOT Project Coordinates to NAD 83 Metric Plane

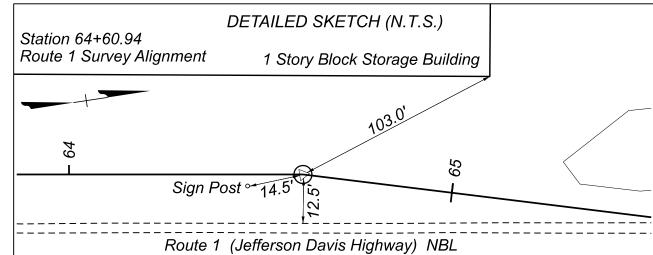
To convert state plane metric units to VDOT project

values, use the following formula.

Values By 1 and 2 Million Respectively.

* Sketch and Detailed Description Below *

Vertical Datum Based On NAVD88 Elevation 145.101 ft. Horizontal Datum Based On NAD83 (CORS) Zone North South (circle one) Horizontal Closure Azimuth to Station 212-2039 is 188°42'44" Latitude: 38°33'19.74747" N (5 Decimal Places) To convert state plane metric units to VDOT project values, use the following formula. Longitude: 77°19'56.97091" W (5 Decimal Places) 1. Reduce the Easting Metric Values By 2.5 Million Meters. The South and North Zone Northing Metric Values By 1 and 2 Million Respectively. 2. Multiply These Values by the U. S. Survey Foot (3.280833333) 3. Multiply These Values by Combined Scale and Project (Monument No.) Elevation Factor (1.00006) for the County. Virginia State Plane Coordinates - NAD 83 Metric Values Reverse This Procedure to Transform East (X) 3601758.846 m VDOT Project Coordinates to NAD 83 Metric Plane North (Y) 2099304.408 m Ortho. Elevation (H) 44.227 m * Sketch and Detailed Description Below



Station Is A Standard VDOT Disk Set In Concrete On West Side Of Route 1 Approx. 700' North Of The North Side Of Quantico Gateway Drive. Station Is 14.5' North Of A Sign Post, 12.5' West Of Edge Of Pavement And 21.5' Southeast Of The Northeast Corner Of A One Story Block Storage Building.

LD-200 (REV. 8/2000) **Virginia Department of Transportation Horizontal Control**

VDOT Traffic Box ☐ 18.3' 8.3'21.5' Wooden Sign Post

Route 1 (Jefferson Davis Highway) NBL

Station Is A Standard VDOT Disk Set In Concrete On West Side Of NBL Of Route 1

Approx. 100' South Of The South Side Of Quantico Gateway Drive. Station Is 18.3' North Of Center Of A VDOT Traffic Box, 8.3' West Of Back Of Curb And 21.5'

DETAILED SKETCH (N.T.S.)

Parking

Of Pavement And 15.0' Northeast Of A 5' Chain Link Fence.

Station Is A Standard VDOT Disk Set In Concrete On East Side Of Route 1

Approx. 430' North Of The North End Of The Bridge Over Quantico Creek.

Station Is 26.0' South Of The End Of A Guardrail, 16.5' Southeast Of Edge

Route 1 (Fraley Boulevard) NBL

5' Chain Link Fence

Control Station I. D. 212 - 2041 Project 001-212-249 City/County Prince William Date 05-15-2012 Established By Woolpert, Inc. Geoid 12A

Vertical Datum Based On NAVD88 Horizontal Datum Based On NAD83 (CORS) Azimuth to Station 2042 is 37°07'27"

Southwest Of A Wooden Sign Post.

Latitude: 38°34'01.48866" N (5 Decimal Places) Longitude: 77°19'23.59935" W (5 Decimal Places) Geoid Separation (N) : -32.359 m

Ellipsoid Height (h): -27.945 m (WGS 84) Control Based On: Station (Name or PID) _____ or Project (Monument No.) Virginia State Plane Coordinates - NAD 83 Metric Values

East (X) 3602550.258 m North (Y) 2100601.655 m Ortho. Elevation (H) 4.414 m

Station 115+60.75

Route 1 Survey Alignment

VDOT Project Coordinates

East (X) 3617500.674 ft. North (Y) 330077.067 ft. Elevation 14.481 ft. Zone North South (circle one)

Horizontal Closure To convert state plane metric units to VDOT project values, use the following formula.

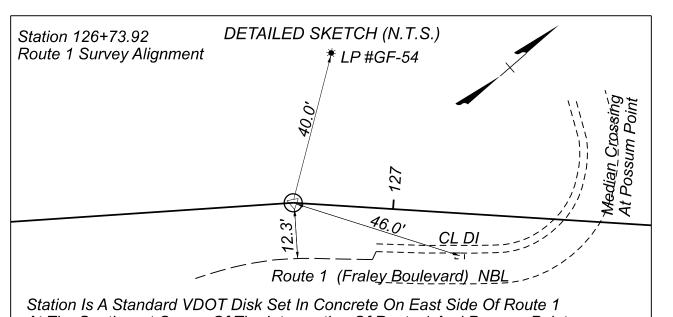
1. Reduce the Easting Metric Values By 2.5 Million Meters. The South and North Zone Northing Metric Values By 1 and 2 Million Respectively. 2. Multiply These Values by the U. S. Survey Foot (3.280833333)

3. Multiply These Values by Combined Scale and Elevation Factor (1.00006) for the County. Reverse This Procedure to Transform VDOT Project Coordinates to NAD 83 Metric Plane

Guardrail

* Sketch and Detailed Description Below *

Virginia Department of Transportation Horizontal Control Control Station I. D. 212 - 2042 Project 001-212-249 **VDOT Project Coordinates** Route 1 City/County Prince William Date 05-15-2012 East (X) 3618172.524 ft. North (Y) 330964.630 ft. Established By Woolpert, Inc. Vertical Datum Based On NAVD88 Elevation 25.851 ft. Geoid 12A Horizontal Datum Based On NAD83 (CORS) Zone North South (circle one) Azimuth to Station 2041 is 217°07'27" Horizontal Closure Latitude: 38°34'10.17603" N (5 Decimal Places) To convert state plane metric units to VDOT project values, use the following formula. Longitude: 77°19'14.99768" W (5 Decimal Places) 1. Reduce the Easting Metric Values By 2.5 Million Geoid Separation (N): -32.358 m Meters. The South and North Zone Northing Metric Values By 1 and 2 Million Respectively. Ellipsoid Height (h): -24.478 m (WGS 84) 2. Multiply These Values by the U. S. Survey Foot (3.280833333) Control Based On: Station (Name or PID) _____ or 3. Multiply These Values by Combined Scale and Project (Monument No.) Elevation Factor (1.00006) for the County. Virginia State Plane Coordinates - NAD 83 Metric Values Reverse This Procedure to Transform East (X) 3602755.026 m VDOT Project Coordinates to NAD 83 Metric Plane North (Y) 2100872.169 m



* Sketch and Detailed Description Below *

At The Southwest Corner Of The Intersection Of Route 1 And Possum Point. Station Is 40.0' Southeast Of Light Pole #GF-54, 12.3' Northwest Of Edge Of Pavement And 46.0' Southwest Of The Center Of A Drop Inlet.

> SHEET NO. PROJECT *IF(I)* 0001-212-249

VA.

NECESSARY BY THE DEPARTMENT

Northern Virginia

: <u>South Corporate Limi</u>

Horizontal Datum Based On <u>NAD 8</u>

Vertical Datum Based On NAVD 88

: <u>H. A. Spence, L.S</u>

Survey By : Woolpert, Inc.

<u>North Corporate Limit</u>

Route I Jefferson Davis Highway
001-212-249, P101

DESIGN FEATURES RELATING TO CONSTRUCTION OR TO REGULATION AND CONTROL OF TRAFFIC MAY BE SUBJECT TO CHANGE AS DEEMED

ROUTE

STATE

0001-212-249,

RW-201, C-501

PROJECT

IF(2)

REVISED

Pro ject

District

Operator

Date

Scale

UPC#

County

From

Survey Horizontal and Vertical Controls

Virginia Department of Transportation Horizontal Control Control Station I. D. 212 - 2043 Project 001-212-249 **VDOT Project Coordinates** City/County Prince William Date 05-15-2012 East (X) 3619941.520 ft. Established By Woolpert, Inc. North (Y) 333152.871 ft.

Horizontal Datum Based On NAD83 (CORS) Azimuth to Station 2044 is 55°52'45"

Latitude: 38°34'31.57911" N (5 Decimal Places) Longitude: 77°18'52.37076" W (5 Decimal Places) Geoid Separation (N): -32.353 m

Ellipsoid Height (h): 0.026 m (WGS 84) Control Based On: Station (Name or PID) Project (Monument No.)

East (X) 3603294.184 m North (Y) 2101539.106 m Ortho. Elevation (H) 32.380 m

PROJECT MANAGER Hoainam Nguyen, P.E. (703) 792-8161 PWC Dept. of Transportation

LD-200 (REV. 8/2000)

SURVEYED BY, DATE _ Leon E. Treutle LS (703)259-3224_7/17/13 _ _ _ _

SUBSURFACE UTILITY BY, DATE <u>Leon E.Treutle LS (703)259-3224</u> 7/17/13

> **Virginia Department of Transportation Horizontal Control** Control Station I. D. 212 - 2044 Project 001-212-249 **VDOT Project Coordinates** City/County Prince William Date 05-15-2012 East (X) 3620392.137 ft. Established By Woolpert, Inc. North (Y) 333458.200 ft. Elevation 81.463 ft.

Vertical Datum Based On NAVD88 Horizontal Datum Based On NAD83 (CORS) Azimuth to Station 2043 is 235°52'45" Latitude: 38°34'34.53939" N (5 Decimal Places)

LD-200 (REV. 8/2000)

Longitude: 77°18'46.64744" W (5 Decimal Places) Geoid Separation (N): -32.354 m Ellipsoid Height (h): -7.524 m (WGS 84) Control Based On: Station (Name or PID)

Project (Monument No.) Virginia State Plane Coordinates - NAD 83 Metric Values East (X) 3603431.524 m North (Y) 2101632.165 m

Ortho. Elevation (H) 24.830 m

Station 160+38.74

Route 1 Survey Alignment

Zone North South (circle one) To convert state plane metric units to VDOT project values, use the following formula.

LD-200 (REV. 8/2000)

Control Station I. D. IP 1

Established By Woolpert, Inc.

Latitude: 38°33'37.66026"

Longitude: 77°19'50.24112"

Geoid Separation (N): -32.358 m

Project (Monument No.)

Back Of Curb.

Ellipsoid Height (h): -10.179 m (WGS 84)

Control Based On: Station (Name or PID)

Virginia State Plane Coordinates - NAD 83 Metric Values

Vertical Datum Based On NAVD88

Azimuth to Station IP 2 is 36°30'57"

Horizontal Datum Based On NAD83 (CORS)

Values By 1 and 2 Million Respectively. 2. Multiply These Values by the U. S. Survey Foot (3.280833333) 3. Multiply These Values by Combined Scale and Elevation Factor (1.00006) for the County. Reverse This Procedure to Transform

1. Reduce the Easting Metric Values By 2.5 Million

Meters. The South and North Zone Northing Metric

VDOT Project Coordinates to NAD 83 Metric Plane

* Sketch and Detailed Description Below

Horizontal Closure

East (X) 3601914.736 m VDOT Project Coordinates to NAD 83 Metric Plane North (Y) 2099858.755 m Ortho. Elevation (H) 22.179 m * Sketch and Detailed Description Below * DETAILED SKETCH (N.T.S.) Station 83+50.31 Route 1 Survey Alignment Route 1 (Fraley Boulevard) NBL Bus Stop Light-Pole 📈

Station Is A Pin And Cap On The East Side Of Route 1 NBL Approximately 100' South Of The Intersection With Graham Park Road. Station Is 17.0' Southwest Of Southwest Corner Of A Bus Stop Shelter, 18.0' West Of A Light Pole And 8.5' Southeast Of

Virginia Department of Transportation Horizontal Control

VDOT Project Coordinates

To convert state plane metric units to VDOT project

1. Reduce the Easting Metric Values By 2.5 Million

Values By 1 and 2 Million Respectively.

3. Multiply These Values by Combined Scale and

Elevation Factor (1.00006) for the County.

Reverse This Procedure to Transform

Meters. The South and North Zone Northing Metric

2. Multiply These Values by the U. S. Survey Foot (3.280833333)

East (X) 3615415.509 ft.

North (Y) 327639.591 ft.

Zone North South (circle one)

values, use the following formula.

Elevation 72.764 ft.

Horizontal Closure

Project 001-212-249

Geoid 12A

N (5 Decimal Places)

W (5 Decimal Places)

City/County Prince William Date 05-15-2012

Geoid 12A Elevation 106.232 ft. Vertical Datum Based On NAVD88 Zone North South (circle one)

> Horizontal Closure To convert state plane metric units to VDOT project values, use the following formula.

1. Reduce the Easting Metric Values By 2.5 Million Meters. The South and North Zone Northing Metric Values By 1 and 2 Million Respectively. 2. Multiply These Values by the U. S. Survey Foot (3.280833333) 3. Multiply These Values by Combined Scale and Elevation Factor (1.00006) for the County.

Virginia State Plane Coordinates - NAD 83 Metric Values Reverse This Procedure to Transform VDOT Project Coordinates to NAD 83 Metric Plane * Sketch and Detailed Description Below

DETAILED SKETCH (N.T.S.) Station 154+94.43 Route 1 Survey Alignment Fire Hydrant PP #KK-88 Elec. Meter Post -----Route 1 (Fraley Boulevard) Station Is A Standard VDOT Disk Set In Concrete On The Northeast Corner Of The

Intersection Of Route 1 And Route 234. Station Is 20.0' West Of Power Pole #KK-88,

LD-200 (REV. 8/2000)

28.5' North Of Electric Meter Post And 63.5' Northeast Of A Fire Hydrant.

Virginia Department of Transportation Horizontal Control Control Station I. D. IP 2 Project 001-212-249 **VDOT Project Coordinates** City/County Prince William Date 05-15-2012 East (X) 3615908.328 ft. North (Y) 328305.211 ft. Established By Woolpert, Inc. Vertical Datum Based On NAVD88 Geoid 12A Elevation 46.861 ft. Horizontal Datum Based On NAD83 (CORS) Zone North South (circle one) Azimuth to Station IP 1 is 216°30'57" Horizontal Closure ____ Latitude: 38°33'44.17711" To convert state plane metric units to VDOT project N (5 Decimal Places) values, use the following formula.

DETAILED SKETCH (N.T.S.)

Route 1 NBL (Fraley Boulevard)

Station Is A Pin And Cap On The West Side Of Route 1 NBL Approximately 700' East

Of It's Intersection With Graham Park Road. Station Is 25.8' Northeast of Light Pole,

10.0' East of Edge Of Pavement Of Parking Lot And 23.0' Southeast of Power Pole

Asphalt Parking

Meters. The South and North Zone Northing Metric

VDOT Project Coordinates to NAD 83 Metric Plane

Bldg. #17970

Values By 1 and 2 Million Respectively.

3. Multiply These Values by Combined Scale and

Elevation Factor (1.00006) for the County.

Reverse This Procedure to Transform

* Sketch and Detailed Description Below

Longitude: 77°19'43.93051" W (5 Decimal Places) 1. Reduce the Easting Metric Values By 2.5 Million Geoid Separation (N): -32.358 m Ellipsoid Height (h): -18.074 m (WGS 84) 2. Multiply These Values by the U. S. Survey Foot (3.280833333)

Control Based On: Station (Name or PID) _____ or Project (Monument No.) Virginia State Plane Coordinates - NAD 83 Metric Values

East (X) 3602064.939 m North (Y) 2100061.625 m Ortho. Elevation (H) 14.283 m

Station 91+78.52

BP91.

Route 1 Survey Alignment

LD-200 (REV. 8/2000)

Virginia Department of Transportation Horizontal Control Control Station I. D. IP 3 Project 001-212-249 City/County Prince William Date 05-15-2012 East (X) 3618824.663 ft. North (Y) 331626.610 ft. Established By Woolpert, Inc. Vertical Datum Based On NAVD88 Geoid 12A Elevation 70.589 ft.

Horizontal Datum Based On NAD83 (CORS) Azimuth to Station 2043 is 36°11'43" Latitude: 38°34'16.63605"

11.3' North Of Corner Of Concrete Walk.

Longitude: 77°19'06.68017" Geoid Separation (N): -32.358 m Ellipsoid Height (h): -10.842 m (WGS 84)

Control Based On: Station (Name or PID) _____ or Project (Monument No.) Virginia State Plane Coordinates - NAD 83 Metric Values

East (X) 3602953.786 m North (Y) 2101073.928 m

VDOT Project Coordinates Zone North South (circle one) Horizontal Closure To convert state plane metric units to VDOT project N (5 Decimal Places) values, use the following formula. W (5 Decimal Places) 1. Reduce the Easting Metric Values By 2.5 Million Meters. The South and North Zone Northing Metric Values By 1 and 2 Million Respectively. 2. Multiply These Values by the U. S. Survey Foot (3.280833333)

DETAILED SKETCH (N.T.S.)

Route 1 (Fraley Boulevard)

Station Is A Standard VDOT Disk Set In Concrete On The East Side Of Route 1 NBL

Approximately 640' North Of It's Intersection With Route 234 Across From "Park And

Ride" Entrance. Station Is 5.3' East Of Back Of Curb, 14.5' South Of Manhole And

∠Park & Ride`

Entrance

3. Multiply These Values by Combined Scale and Elevation Factor (1.00006) for the County. Reverse This Procedure to Transform VDOT Project Coordinates to NAD 83 Metric Plane Ortho. Elevation (H) 21.516 m * Sketch and Detailed Description Below *

DETAILED SKETCH (N.T.S.) Station 136+03.17 Route 1 Survey Alignment

Station Is A Pin And Cap On The Northeast Corner Of The Intersection Of Route 1 And Stage Coach Road. Station Is 11.5 East Of Edge Of Pavement Of Route 1, 9.0' West Of Edge Of Pavement Of Parking Lot And 39.0' North Of Sign.

PLANIMETRIC LEGEND

Advertising Sign o BM Bench Mark Bollard Post • PHOTO CP Photo Control Point Control Station Cell Tower Drainage Flow Arrow (Storm Drainage) Filler Cap (Gas Stations) Flow Arrow (Streams & Rivers) Flag Pole Secondary Control Point Filler Pipe (Gas Stations) Gas Tank Access Manhole (Gas Stations) Gravesite Marker Guard Post Gas Vent Pipe (Gas Stations) Mine Entrance Node Point Property Line Symbol Found Monumentation Property Monument Metal or Wooden Post Right of Way Monument Iron Right of Way Pin Railroad Mile Marker Railroad Right of Way Monument Railroad Signal Pole or Gate Railroad Telegraph Pole Railroad Telephone Pole Shrub Storm Sewer Manhole Tree Wetland Flag Automatic Wetland Flag Manual Elevation Tick Mark Connected Plat Symbol Brush Line Pipe Culverts * - City Line County Line ======= Curb Only Curb and Gutter Fence Line Guardrail Hedge Row Jersey Barrier Obscure Areas Paved Ditches Railroad Right of Way State Line Edges of Water Sidewalks Wetlands * Designate size of culverts

> SHEET NO. **PROJECT**

> > 0001-212-249

IF(2)

(Variable from 12" to 120")

PLANS

External =

Mid. Ord. =

P.C. Station P.T. Station

Long Chord =

Curve RTE1MLALI6

P.I. Station

External =

Mid. Ord. =

P.C. Station

Back Ahead

Tangent

Long Chord Mid. Ord. =

P.C. Station P.T. Station

Curve RTE1MLALI12

P.I. Station

External =

Mid. Ord. =

P.C. Station

Long Chord

Delta

. Station

= N

Chord Bear = N 5° 49'06.00"

Back = N 3° 08' 41.24" Ahead = N 40° 16' 43.86" Chord Bear = N 21° 42' 42.55"

Long Chord

Tangent

3,614,638.75 Sta

324,733.20 E

326,248.52 E

327,196.74 E

Curve Data

Curve Data

Curve Data x - - - - - - x

Curve Data

328,961.83 E

328,829.61 E 329,101.49 E

* - - - - - - *

326,654.46 L

326,416.24

Course from PT RTE1MLALI9 to PC RTE1MLALI12 N 40° 16'43.86" E Dist 1,468.54

332,062.15

Course from PT RTE1MLALI6 to PC RTE1MLALI9 N 3° 08'41.24" E Dist 91.07

388.75 9.47 251+42.48 N 255+31.84 N

268+68.66 N 5° 20' 49.52'' (LT) 1° 25' 56.62''

266+81.87 N

278+18.04 N

37° 08' 02.62" (RT)

104.10 271+46.24 N

300+84.31 N 3° 58' 13.30'' (LT)

346.41

3.00 299+11.00 N

= N 40° 16' 43.86''

Ahead = N 36° 18'30.55'' Chord Bear = N 38° 17'37.20''

1° 08' 45.30''

8° 29' 30.76'' 3° 08' 41.24''

Course from PT RTE1MLALI3 to PC RTE1MLALI6 N 8° 29'30.76" E Dist 1,150.03

= N 19° 38'46.60'' | = N 8° 29'30.76''

Chord Bear = N 14° 04'08.68" E

250+00.00

3,614,752.32

3,614,978.57

3,615,030.66

3,616,526.45

3,616,414.40 3,616,629.07

Horizontal Construction Alignment Data

ROUTE I MAINLINE (continued)

| Curve Data | 7 | * | * | | |
|---|--|-------------------------------------|---------------------------------|-----------------------|------------------------------------|
| Curve RTE1MLALI_13 P.I. Station Delta = Degree = Tangent = Length = Radius = External = Long Chord = | 304+30.79 3° 58'13.30'' 1° 08'45.30' 173.31 346.48 5,000.00 3.00 346.4 | (RT) 3 | 329,241.15 | E | 3,616,731.69 |
| Mid. Ord. = P.C. Station P.T. Station C.C. | 3.00 302+57.48 306+03.96 N 36° 18' 30.55'' 40° 16' 43.86'' 38° 17' 37.20'' | E E | 329,101.49 329,373.37 E | E E 3,620,658.2 | 3,616,629.07 3,616,843.74 28 |
| Course from PT I | RTE1MLALI <u>.</u> 13 to f | | 16 N 40° 16 e Data | 43.86" E [| Dist 1,084.91 |
| Curve RTE1MLALI_16 P.I. Station Delta = Degree = Tangent = Length = | 318+62.18 3° 58' 13.30'' 1° 08' 45.30' 173.31 346.48 | (RT) | 330,333.27 | E | 3,617,657.19 |
| Radius = External = Long Chord = Mid. Ord. = P.C. Station P.T. Station C.C. Back = N Ahead = N | 5,000.00 3.00 346.4 3.00 316+88.87 320+35.35 N 40° 16' 43.86'' 44° 14' 57.16'' | 1 N N 326,968.51 E E | 330,201.05 330,457.41 E 3 | E E ,621,359.68 | 3,617,545.14 3,617,778.12 |
| Chord Bear = N | 42° 15' 50.51'' E | | e Data | | |
| Curve RTE1MLALI_17 | 7 | * | * | | |
| P.I. Station Delta = Degree = Tangent = Length = Radius = External = Long Chord = Mid. Ord. = | 322+08.66 | , (LT) 3 | 330,581.55 | E | 3,617,899.05 |
| P.C. Station P.T. Station C.C. Back = N Ahead = N Chord Bear = N | 320+35.35 323+81.83 N 44° 14'57.16'' { | <u>-</u> E | 330,457.41 330,713.77 E | E E ,614,196.56 | 3,617,778.12 3,618,011.10 |
| Course from PT I | RTE1MLALI <u>1</u> 7 to : | 232 N 40° 16 | 6' 43.86'' E D | ist 1,194.95 | |
| Point 232 | Ν | 331,625.41 E | 3,618,78 | 33.64 Sta | 335+76.78 |
| Course from 232 | to 233 N 40° | 40' 54.94'' E | Dist 1,176.29 | | |
| Point 233 | Ν | 332,517.44 E | 3,619,5 | 550.42 Sta | 347+53.07 |
| Course from 233 | | 17' 26.99'' E [| Dist 262.06 | | |
| Point 234 | Ν | 332,717.33 | E 3,619, | 719.88 Sta | 350+15.13 |
| Course from 234 | to 237 N 40° | 39' 11.86'' E D | oist 815.10 | | |
| | | | | | 358+30.24 |

REVISED STATE ROUTE PROJECT 0001-212-249, *IG(1)* VA. RW-201, C-501

DESIGN FEATURES RELATING TO CONSTRUCTION OR TO REGULATION AND CONTROL OF TRAFFIC MAY BE SUBJECT TO CHANGE AS DEEMED NECESSARY BY THE DEPARTMENT

Johnson, Mirmiran & Thompson Richmond, Virginia **ROADWAY ENGINEER**

QUANTICO GATEWAY DRIVE

1 DESCRIBE CHAIN QUANTICO

Chain QUANTICO contains: QUANTICO1 CUR QUANTICO_3 QUANTICO5

Beginning chain QUANTICO description Feăture: - 25 Scale Baselines

Point QUANTICO1 N

325,105.38 E

3,614,807.89 Sta

100+00.00

13+31.54

Course from QUANTICO1 to PC QUANTICO_3 S 69° 58'17.36" W Dist 256.72

Curve Data

Curve QUANTICO_3 104+42.30 N 324,953.89 E 3,614,392.33 P.I. Station 62° 38' 18.16'' (RT) 18° 47' 07.81'' Taňaent Length Radius External = Long Chord = Mid. Ord. = P.C. Station P.T. Station 44.44 102+56.72 N 105+90.16 N 325,304.01 E 3,614,462.23 = S 69° 58'17.36" W = N 47° 23'24.48" W Chord Bear = N 78° 42' 33.56" W

Course from PT QUANTICO 3 to QUANTICO5 N 47° 23' 24.48" W Dist 68.53 Point QUANTICO5 325,125.92 E 3,614,205.31 Sta 106+58.69

______ Ending chain QUANTICO description

GRAHAM PARK ROAD (LEFT)

1 DESCRIBE CHAIN GRAHAMPARKL

Chain GRAHAMPARKLT contains: GRAHAMPARKLT1 GRAHAMPARKLT2

Beginning chain GRAHAMPARKLT description Feature: - 25 Scale Baselines

327,734.03 E Point GRAHAMPARKLT1 N 3,615,485.98 Sta 10+00.00 Course from GRAHAMPARKLT1 to GRAHAMPARKLT2 N 45° 00'50.47" W Dist 331.54

Point GRAHAMPARKLT2 N

327,968.41 E 3,615,251.49 Sta

Ending chain GRAHAMPARKLT description

GRAHAM PARK ROAD (RIGHT)

1 DESCRIBE CHAIN GRAHAMPARKRT

Chain GRAHAMPARKRT contains: GRAHAMPARKRT1 GRAHAMPARKRT2

Beginning chain GRAHAMPARKRT description Feature: - 25 Scale Baselines

Point GRAHAMPARKRT1 N 327,739.08 E 3,615,490.26 Sta 10+00.00

Course from GRAHAMPARKRT1 to GRAHAMPARKRT2 S 60° 11'41.58" E Dist 890.41 Point GRAHAMPARKRT2 N 327,296.50 E 3,616,262.89 Sta 18+90.41

______ Ending chain GRAHAMPARKRT description

> **PROJECT** SHEET NO. NTS 0001-212-249 *IG(1)*

| ### PECC. E. DUKE STREET | BSURFACE UTILITY BY, DATE <i>Lead F. Treutle LS_(703)259-3224_7/L</i> | 7 7_/ <u>13</u> | |
|--|---|---|--|
| Chan El C.R. DINCES DIKES | RELOC. E. DUKE STREET | | |
| Discrete Or Duke 3 Dukes | | | |
| Course From DUKET to PC DUKE 3 N 49" 43" 1614" W Dist 74.60 | DUKE1 CUR DUKE_3 DUKE5 | | |
| Course from DUKEL SP COUKE 3 N 49° 43° 614" W Bist 74.60 Durve Bota | Beginning chain DUKE description Feature: - 25 Scale Baselines | | ======= |
| Station | , and the second se | 43'16.14" W Dist 74.60 | 10+00.00 |
| Mid. Cr. | P.I. Station 11+55.88 N Delta = 74° 26' 40.52'' (RT) Degree = 53° 32' 50.85'' Tangent = 81.28 Length = 139.03 Radius = 107.00 External = 27.37 Long Chord = 129.45 | 328,424.20 E | 3,615,866.52 |
| ### Paint DUKE5 N 328,520.74 E 3,615,910.97 Sto 12-38.63 Ending chain DUKE description ################################### | Mid. Ord. = 21.80 P.C. Station 10+74.60 N P.T. Station 12+13.63 N C.C. N 328, Back = N 49° 43' 16.14'' W Ahead = N 24° 43' 24.37'' E Chord Bear = N 12° 29' 55.88'' W | | 3,615,928.53 3,615,900.52 7.71 |
| ### WILLIAMSTOWN DRIVE (* 1 DESCRIBE CHAN WILLIAMSTOWN Chain WILLIAMSTOWN contains: WILLIAMSTOWN WILLIAMSTOWN Beginning chain WILLIAMSTOWN description Fedture: - 25 Scale Baselines Point WILLIAMSTOWN1 N 329,442,34 E 3,616,902,19 Sta 10-00.00 Course from WILLIAMSTOWN1 to WILLIAMSTOWN2 S 49° 25'47,36" E Dist 360,26 Point WILLIAMSTOWN2 N 329,208.04 E 3,617,175,85 Sta 13-60,26 Ending chain WILLIAMSTOWN description ################################### | - | | a 12+38.63 |
| ### WILLIAMSTOWN DRIVE | Ending chain DUKE description | | ======== |
| CANAL ROAD ROAD RT (* 1 DESCRIBE CHAIN CANALRT Chain CANALRT contains: CANALRT1 CANALRT2 Beginning chain CANALRT description Feature: - 25 Scale Baselines Point CANALRT1 N 330,627.28 E 3,617,936.08 Sta 10+00.00 Course from CANALRT1 to CANALRT2 S 27° 42' 30.36" E Dist 234.51 Point CANALRT2 N 330,419.66 E 3,618,045.13 Sta 12+34.51 Ending chain CANALRT description CANAL ROAD ROAD LT (* 1 DESCRIBE CHAIN CANAL_TEST Chain CANAL_TEST contains: Beginning chain CANAL_TEST description Fedture: - 25 Scale Baselines Point 288 N 330,838.68 E 3,617,835.87 Sta 200+00.00 Course from 288 to 289 S 27° 42' 30.36" E Dist 230.12 | WILLIAMSTOWN1 WILLIAMSTOWN2 | | |
| Chain CANALRT contains: CANALRT1 CANALRT description Feature: - 25 Scale Baselines Point CANALRT1 N 330,627.28 E 3,617,936.08 Sta 10+00.00 Course from CANALRT1 to CANALRT2 S 27° 42′ 30.36″ E Dist 234.51 Point CANALRT2 N 330,419.66 E 3,618,045.13 Sta 12+34.51 Ending chain CANALRT description CANAL ROAD ROAD LT (* 1 DESCRIBE CHAIN CANAL_TEST Chain CANAL_TEST contains: Beginning chain CANAL_TEST description Feature: - 25 Scale Baselines Point 288 N 330,838.68 E 3,617,835.87 Sta 200+00.00 Course from 288 to 289 S 27° 42′ 30.36″ E Dist 230.12 | WILLIAMSTOWN1 WILLIAMSTOWN2 Beginning chain WILLIAMSTOWN description Feature: - 25 Scale Baselines Point WILLIAMSTOWN1 N 329,442.3 Course from WILLIAMSTOWN1 to WILLIAMSTOWN1 | TOWN2 S 49° 25' 47.36" E | Dist 360.26 |
| Course from CANALRT1 to CANALRT2 S 27° 42' 30.36" E Dist 234.51 Point CANALRT2 N 330,419.66 E 3,618,045.13 Sta 12+34.51 Ending chain CANALRT description CANAL ROAD ROAD LT (* 1 DESCRIBE CHAIN CANAL_TEST Chain CANAL_TEST contains: 288 289 Beginning chain CANAL_TEST description Feature: - 25 Scale Baselines Point 288 N 330,838.68 E 3,617,835.87 Sta 200+00.00 Course from 288 to 289 S 27° 42' 30.36" E Dist 230.12 | WILLIAMSTOWN1 WILLIAMSTOWN2 Beginning chain WILLIAMSTOWN description Feature: - 25 Scale Baselines Point WILLIAMSTOWN1 N 329,442.3 Course from WILLIAMSTOWN1 to WILLIAMSTOWN2 N 329,208 | TOWN2 S 49° 25' 47.36" E | Dist 360.26 |
| Point CANALRT2 N 330,419.66 E 3,618,045.13 Sta 12+34.51 Ending chain CANALRT description CANAL ROAD ROAD LT (* 1 DESCRIBE CHAIN CANAL_TEST Chain CANAL_TEST contains: 288 289 Beginning chain CANAL_TEST description Fedture: - 25 Scale Baselines Point 288 N 330,838.68 E 3,617,835.87 Sta 200+00.00 Course from 288 to 289 S 27° 42' 30.36" E Dist 230.12 | Beginning chain WILLIAMSTOWN description Feature: - 25 Scale Baselines Point WILLIAMSTOWN1 N 329,442.3 Course from WILLIAMSTOWN1 to WILLIAMSTOWN2 N 329,208 Ending chain WILLIAMSTOWN description CANAL ROAD ROAD RT (* 1 DESCRIBE CHAIN CANALRT Chain CANALRT contains: CANALRT CANALRT2 | TOWN2 S 49° 25' 47.36" E | Dist 360.26 |
| Ending chain CANALRT description CANAL ROAD ROAD LT (* 1 DESCRIBE CHAIN CANAL_TEST Chain CANAL_TEST contains: 288 289 Beginning chain CANAL_TEST description Feature: - 25 Scale Baselines Point 288 N 330,838.68 E 3,617,835.87 Sta 200+00.00 Course from 288 to 289 S 27° 42' 30.36" E Dist 230.12 | Beginning chain WILLIAMSTOWN description Feature: - 25 Scale Baselines Point WILLIAMSTOWN1 N 329,442.3 Course from WILLIAMSTOWN1 to WILLIAMSTOWN2 N 329,208 Ending chain WILLIAMSTOWN description CANAL ROAD ROAD RT (* 1 DESCRIBE CHAIN CANALRT Chain CANALRT contains: CANALRT1 CANALRT2 Beginning chain CANALRT description Feature: - 25 Scale Baselines Point CANALRT1 N 330,627.28 | FOWN2 S 49° 25' 47.36" E 8.04 E | Dist 360.26 13+60.26 =================================== |
| Course from 288 to 289 S 27° 42' 30.36" E Dist 230.12 | Beginning chain WILLIAMSTOWN description Feature: - 25 Scale Baselines Point WILLIAMSTOWN1 N 329,442.3 Course from WILLIAMSTOWN1 to WILLIAMSTOWN2 N 329,208 Ending chain WILLIAMSTOWN description CANAL ROAD ROAD RT (* 1 DESCRIBE CHAIN CANALRT Chain CANALRT contains: CANALRT1 CANALRT2 Beginning chain CANALRT description Feature: - 25 Scale Baselines Point CANALRT1 N 330,627.28 Course from CANALRT1 to CANALRT2 S 2 | FOWN2 S 49° 25' 47.36" E 8.04 E | Dist 360.26 13+60.26 =================================== |
| | WILLIAMSTOWN1 WILLIAMSTOWN description Feature: - 25 Scale Baselines Point WILLIAMSTOWN1 N 329,442.3 Course from WILLIAMSTOWN1 to WILLIAMSTOWN1 to WILLIAMSTOWN2 N 329,208 Ending chain WILLIAMSTOWN description CANAL ROAD ROAD RT (* 1 DESCRIBE CHAIN CANALRT Chain CANALRT contains: CANALRT1 CANALRT2 Beginning chain CANALRT description Feature: - 25 Scale Baselines Point CANALRT1 N 330,627.28 Course from CANALRT1 to CANALRT2 S 2 Point CANALRT2 N 330,41 Ending chain CANALRT description CANAL ROAD ROAD LT (* 1 DESCRIBE CHAIN CANAL_TEST Chain CANAL_TEST contains: 288 289 | FOWN2 S 49° 25' 47.36" E 8.04 E | Dist 360.26 13+60.26 =================================== |
| | WILLIAMSTOWN1 WILLIAMSTOWN description Feature: - 25 Scale Baselines Point WILLIAMSTOWN1 N 329,442.3 Course from WILLIAMSTOWN1 to WILLIAMSTOWN2 N 329,208 Ending chain WILLIAMSTOWN description CANAL ROAD ROAD RT (* 1 DESCRIBE CHAIN CANALRT Chain CANALRT contains: CANALRT1 CANALRT2 Beginning chain CANALRT description Feature: - 25 Scale Baselines Point CANALRT1 N 330,627.28 Course from CANALRT1 to CANALRT2 S 2 Point CANALRT2 N 330,41 Ending chain CANALRT description CANAL ROAD ROAD LT (* 1 DESCRIBE CHAIN CANAL_TEST Chain CANAL_TEST contains: 288 289 Beginning chain CANAL_TEST description Feature: - 25 Scale Baselines | FOWN2 S 49° 25' 47.36" E 8.04 E 3,617,175.85 Sta 27° 42' 30.36" E Dist 234.51 9.66 E 3,618,045.13 Sta | Dist 360.26 13+60.26 10+00.00 12+34.51 ========= |

| 110000000000000000000000000000000000000 | an atrustion | Alignment Data |
|---|---|-----------------|
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| 1101120111010 | oriori doriori | Turginion Daid |

POSSUM POINT ROAD

1 DESCRIBE CHAIN POSSUM

Chain POSSUM contains: POSSUM1 CUR POSSUM_3 POSSUM5

Beginning chain POSSUM description

Feăture: - 25 Scale Baselines ______

330,976.50 E 3,618,233.74 Sta 200+00.00

Course from POSSUM1 to PC POSSUM_3 S 71° 44'05.62" E Dist 179.97

Curve Data * - - - - - - * Curve POSSUM_3 P.I. Station 202+91.38 N 40° 44' 41.16'' (LT) 330,885.18 E 3,618,510.43 19° 05′ 54.94′′ Tangent External = Long Chord = Mid. Ord. = P.C. Station P.T. Station 330,920.09 330,927.77 201+79.97 N 331,204.98 E Back = S 71° 44' 05.62'' Ahead = N 67° 31' 13.22'' I Chord Bear = N 87° 53' 33.80''

Course from PT POSSUM_3 to POSSUM5 N 67° 31' 13.22" E Dist 139.66 330,981.18 E Point POSSUM5 3,618,742.42 Sta

Ending chain POSSUM description

MAIN STREET RIGHT TURN LANE

1 DESCRIBE CHAIN MAINSTRT

Chain MAINSTRT contains: MAINSTRT1 CUR MAINSTRT 3 CUR MAINSTRT 4 MAINSTRT6

Beginning chain MAINSTRT description Feăture: - 25 Scale Baselines

Curve MAINSTRT_3

Chord Bear = N 57°

Ending chain MAINSTRT description

Point MAINSTRT1 N 3,617,933.40 Sta 10+00.00 330,915.43 E

Course from MAINSTRT1 to PC MAINSTRT_3 N 63° 42'20.86" E Dist 5.00

Curve Data * - - - - - - *

330,992.81 E 3,618,090.00 P.I. Station 11+74.67 N 11° 47′ 57.43′′ (LT) Delta 3° 29′ 21.80′′ Degree Tangent Length Radīus External = Long Chord Mid. Ord. = 10+05.00 N 13+43.15 N 330,917.64 P.C. Station 331,097.49 Station 332,389.75 E = N 63° 42'20.86'' = N 51° 54'23.43''

> Curve Data x - - - - - - x

Curve MAINSTRT_4 14+92.01 N 331,189.33 E 3,618,340.69 ⊃.I. Station 37' 39.58'' (LT) Delta 3° 55' 08.40'' Degree langent _enğth ≺adius External = Long Chord Mid. Ord. = P.C. Station P.T. Station 13+43.15 16+39.85 Station 332,248.09 Back = N 51° 54' 23.43'' (Ahead = N 40° 16' 43.86'' (Chord Bear = N 46° 05' 33.64''

Course from PT MAINSTRT_4 to MAINSTRT6 N 40° 16' 43.86" E Dist 45.88 3,618,466.59 Sta Point MAINSTRT6 331,337.89 E 16+85.72 ______

REVISED STATE ROUTE PROJECT 0001-212-249, IG(2) VA. RW-201, C-501

DESIGN FEATURES RELATING TO CONSTRUCTION OR TO REGULATION AND CONTROL OF TRAFFIC MAY BE SUBJECT TO CHANGE AS DEEMED NECESSARY BY THE DEPARTMENT

Johnson, Mirmiran & Thompson Richmond, Virginia **ROADWAY ENGINEER**

CANAL ROAD RD. SERVICE ROAD

1 DESCRIBE CHAIN CANALRTSVC

Chain CANALRTSVC contains: CANALRTSVC1 CUR CANALRTSVC_3 CUR CANALRTSVC_6 CUR CANALRTSVC_9 CANALRTSVC11

Beginning chain CANALRTSVC description Feature: - 25 Scale Baselines

Point CANALRTSVC1 N 330,502.02 E 3,618,001.87 Sta 20+00.00

Course from CANALRTSVC1 to PC CANALRTSVC_3 N 62° 13' 34.10" E Dist 51.14

Curve Data * - - - - - *

Curve CANALRTSVC_3 330,593.80 E P.I. Station 21+96.97 N 3,618,176.15 31' 45.39'' (RT) Degree Tangent Length External = Long Chord = 29.56 P.C. Station T. Station 330,252.45 E Back = N 62° 13' 34.10" E Ahead = S 67° 14' 40.50" E Chord Bear = N 87° 29' 26.80" E

Course from PT CANALRTSVC_3 to PC CANALRTSVC_6 S 67° 14' 40.50" E Dist 86.69

Curve Data

* - - - - - - ×

Curve CANALRTSVC_6 P.I. Station 24+19.44 N 330,500.34 E 3,618,398.97 12' 42.26'' (LT) Delta 38' 52.40'' Degree Tangent Length Radīus External = Lona Chord 18.19 0.21 24+10.34 24+28.53 Mid. Ord. = P.C. Station 3,618,390.57 3,618,407.65 P.T. Station 330,688.30 3,618,467.93 Back = S 67° 14' 40.50" Ahead = S 72° 27' 22.76" Chord Bear = S 69° 51' 01.63" E

Course from PT CANALRTSVC_6 to PC CANALRTSVC_9 S 72° 27' 22.76" E Dist 36.05

Curve Data x - - - - - - x

Curve CANALRTSVC_9 P.I. Station 24+78.20 N 330,482.63 E 3,618,455.01 7° 47' 31.09'' (RT) Delta 28° 38' 52.40'' Degree Tangent Length Radius External = 0.46 Long Chord 0.46 Mid. Ord. = P.C. Station 24+64.58 24+91.78 Station 330,296.03 27' 22.76'' 39' 51.68'' 33' 37.22'' = S 72° = S 64° Ahead Chord Bear = S 68°

Course from PT CANALRTSVC_9 to CANALRTSVC11 S 64° 39' 51.68" E Dist 22.73 Point CANALRTSVC11 N 330,467.07 E 3,618,487.87 Sta 25+14.51

Ending chain CANALRTSVC description

> PROJECT SHEET NO. IG(2) NTS 0001-212-249

REVISED

Horizontal Construction Alignment Data

RELOC.MAIN ST. NORTH

1 DESCRIBE CHAIN MAINST1 Chain MAINST1 contains: CUR MAINST11 D813 D814 D815

Beginning chain MAINST1 description

Curve Data x - - - - - - x Curve MAINST11 100+94.22 N ° 07' 48.75" (LT) 330,996.95 E 3,618,157.08 External = Long Chord Mid. Ord. = 100+00.00 N 101+85.16 N Station Course from PT MAINST11 to D813 S 63° 42' 20.86" W Dist 247.83

Point D813 330,845.43 E 104+32.99

Course from D813 to D814 S 65° 06' 51.74" W Dist 81.36

Point D814 330,811.19 E 3,617,776.61 Sta 105+14.35 Course from D814 to D815 S 63° 42' 20.86" W Dist 135.65

MAIN ST.CUL-DE-SAC

Ending chain MAINST1 description

1 DESCRIBE CHAIN MAINCUL-DE-SAC Chain MAINCUL-DE-SAC contains: 111 112 113

Beginning chain_MAINCUL-DE-SAC description Feature: - 25 Scale Baselines

325,409.89 E 3,614,558.65 Sta Course from 111 to 112 S 19° 26' 17.59" E Dist 193.06 325,227.83 E 3,614,622.90 Sta Course from 112 to 113 S 78° 09'08.93" E Dist 96.96 325,207.92 E 3,614,717.80 Sta

_____ Ending chain MAINCUL-DE-SAC description

ROUTE PROJECT 0001-212-249, IG(3) VA. RW-201, C-501 DESIGN FEATURES RELATING TO CONSTRUCTION

STATE

OR TO REGULATION AND CONTROL OF TRAFFIC MAY BE SUBJECT TO CHANGE AS DEEMED NECESSARY BY THE DEPARTMENT

Johnson, Mirmiran & Thompson Richmond, Virginia ROADWAY ENGINEER

MAIN ST.CUL-DE-SAC EOP

1 DESCRIBE CHAIN MAINCULEOP

Curve MAINCULEOP2

Chain MAINCULEOP contains: D816 CUR MAINCULEOP1 CUR MAINCULEOP2 CUR MAINCULEOP3

Beginning chain MAINCULEOP description

325,295.35 E 3,614,574.51 Sta Point D816 10+00.00

Course from D816 to PC MAINCULEOP1 S 19° 20' 43.04" E Dist 96.15

Curve Data

Curve MAINCULEOP1 325,116.84 E 3,614,637.69 Tangent External = P.C. Station P.T. Station

Curve Data

325,266.16 E 3,614,749.30 Length External = Long Chord = Mid. Ord. = P.C. Station P.T. Station Back = N 36° 46' 34.78" E Ahead = N 86° 48' 18.97" W Chord Bear = N 25° 00' 52.09" W

Curve Data * - - - - - - * Curve MAINCULEOP3 P.I. Station 325,273.36 E 3,614,620.32 20' 39.51" (RT) Length External = Long Chord

Mid. Ord. = P.C. Station 13+11.84

Ending chain MAINCULEOP description

PROJECT SHEET NO. NTS 0001-212-249 IG(3)

Horizontal Construction Alignment Data

STATE REVISED ROUTE PROJECT 0001-212-249, IG(4) VA. RW-201, C-501

DESIGN FEATURES RELATING TO CONSTRUCTION OR TO REGULATION AND CONTROL OF TRAFFIC MAY BE SUBJECT TO CHANGE AS DEEMED NECESSARY BY THE DEPARTMENT

Johnson, Mirmiran & Thompson Richmond, Virginia **ROADWAY ENGINEER**

TOWNSQUARE COURT

2 DESCRIBE CHAIN TOWNSQUARE

Chain TOWNSQUARE contains: TOWNSQUARE1 CUR TOWNSQUARE_3 TOWNSQUARE5

Beginning chain TOWNSQUARE description Feăture: - 25 Scale Baselines

326,557.95 E 3,614,995.82 Sta Point TOWNSQUARE1 N 9+52.57

Course from TOWNSQUARE1 to PC TOWNSQUARE_3 S 79° 35'02.29" E Dist 259.39

Curve Data *----*

Curve TOWNSQUARE_3 P.I. Station 12+79.33 N 22° 24' 46.97'' (LT) 16° 51' 06.12'' 326,498.87 E 3,615,317.19 Degree = Tangent = 132.16 6.48 12+11.97 N Long Chord = P.C. Station 326,845.45 E Chord Bear = N 89° 12' 34.23"

Course from PT TOWNSQUARE_3 to TOWNSQUARE5 N 78° 00'10.74" E Dist 147.06 3,615,526.93 Sta Point TOWNSQUARE5 326,543.44 E

______ Ending chain TOWNSQUARE description

TRIPOLI BOULEVARD

Chain TRIPOLI1 contains:

Beginning chain TRIPOLI1 description

1 DESCRIBE CHAIN TRIPOLI1

3,618,969.75 Sta 331,841.92 E 99+87.48 Course from D806 to D807 N 45° 47' 21.10" W Dist 278.23

102+65.71

Point D807 332,035.93 E 3,618,770.32 Sta

______ Ending chain TRIPOLI1 description

ORIGINAL STAGE COACH ROAD

1 DESCRIBE CHAIN STAGEENTR1

Chain STAGEENTR1 contains: STAGEENTR11 CUR STAGEENTR1_3 STAGEENTR15

Beginning chain STAGEENTR1 description Feature: - 25 Scale Baselines

Point STAGEENTR11 N 331,705.76 E 3,619,118.89 Sta 100+00.00

Course from STAGEENTR11 to PC STAGEENTR1_3 S 28° 30' 21.36" W Dist 22.22

Curve Data

x - - - - - - - **x** Curve STAGEENTR1_3 P.I. Station 100+64.38 N 45° 43' 13.97'' (RT) 331,649.18 E 3,619,088.16 Degree Tangent External = Long Chord = Mid. Ord. = P.C. Station P.T. Station Back = S 28° 30' 21.36" W Ahead = S 74° 13' 35.34" W Chord Bear = S 51° 21' 58.35" W

Course from PT STAGEENTR1 3 to STAGEENTR15 S 74° 13' 35.34" W Dist 189.84 331,586.12 E 3,618,864.90 Sta 102+91.86 Point STAGEENTR15 N

______ Ending chain STAGEENTR1 description

> SHEET NO. PROJECT NTS 0001-212-249 IG(4)

DESIGN FEATURES RELATING TO CONSTRUCTION OR TO REGULATION AND CONTROL OF TRAFFIC

MAY BE SUBJECT TO CHANGE AS DEEMED

NECESSARY BY THE DEPARTMENT

Johnson, Mirmiran & Thompson

Richmond, Virginia **ROADWAY ENGINEER**

10+00.00

ROUTE

STATE

0001-212-249,

RW-201, C-501

PROJECT

IG(5)



PROJECT MANAGER <u>Hoainam Nguyen, P.E. (703) 792-8161 PWC Dept. of Transportation</u> SURVEYED BY, DATE Leon E.Treutle LS \(\)(703)259-3224 \(T\)(17\)(13\) DESIGN BY JMT_Engineering_(804) 323-9900______ SUBSURFACE UTILITY BY, DATE Lean E. Treutle LS (703)259-3224_7/17/13

Entrance Alignment Construction Data

| Entrance | | Cta | 256+ | 06 11 | Fralo | v Rlvd |
|----------|---------------|-------|------|-------|--------------------|---------|
| | \mathcal{Q} | >1U., | <ンロブ | UD.41 | Γ I UIC | V DIVU. |

1 DESCRIBE CHAIN 256+06RT

Chain 256+06RT contains:

Beginning chain 256+06RT description Feature: - 25 Scale Baselines ______

3,614,792.17 Sta Point 114 325,000.11 E 10+00.00

Course from 114 to 115 S 81° 30' 29.24" E Dist 100.00

324,985.34 E 3,614,891.07 Sta 11+00.00

______ Ending chain 256+06RT description

Entrance @ Sta.257+37.12 Fraley Blvd.

2 DESCRIBE CHAIN 257+37RT

Chain 257+37RT contains: 116 117

Beginning chain 257+37RT description

Feăture: - 25 Scale Baselines

______ 325,129.39 E 3,614,811.47 Sta

Course from 116 to 117 S 81° 30' 29.24" E Dist 100.00

3.614.910.38 Sta Point 117 325,114.62 E 11+00.00

______ Ending chain 257+37RT description

Entrance @ Sta.258+69.12 Fraley Blvd.

3 DESCRIBE CHAIN 258+69RT

Chain 258+69RT contains: 118 119

Beginning chain 258+69RT description Feature: - 25 Scale Baselines ______

325,259.94 E 3,614,830.97 Sta 10+00.00

Course from 118 to 119 S 81° 30' 29.24" E Dist 100.00

325,245.17 E 3,614,929.87 Sta

______ Ending chain 258+69RT description

Entrance @ Sta.259+69.11 Fraley Blvd.

<* 4 DESCRIBE CHAIN 259+69RT</pre>

Chain 259+69RT contains: 120 121

Beginning chain 259+69RT description Feature: - 25 Scale Baselines

325,358.84 E 3,614,845.73 Sta 10+00.00

Course from 120 to 121 S 81° 30' 29.24" E Dist 100.00

Point 121 325,344.07 E 3,614,944.63 Sta 11+00.00

Ending chain 259+69RT description

Entrance @ Sta.261+53.23 Fraley Blvd.

Chain 261+43RT contains: 122 123

Beginning chain 261+43RT description Feăture: - 25 Scale Baselines

Point 122 325,540.94 E 3,614,872.92 Sta

Course from 122 to 123 S 81° 30′ 29.24′′ E Dist 120.00

325,523.22 E 3,614,991.60 Sta 11+20.00

______ Ending chain 261+43RT description

Entrance @ Sta. 262+17.61 Fraley Blvd.

6 DESCRIBE CHAIN 262+17LT

Chain 262+17LT contains: 124 125

Beginning chain 262+17LT description Feature: - 25 Scale Baselines

Point 124 325,604.61 E 3,614,882.43 Sta 10+00.00

Course from 124 to 125 N 81° 30' 29.24" W Dist 110.00

325,620.85 E

______ Ending chain 262+17LT description

Entrance @ Sta. 264+69.59 Fraley Blvd.

7 DESCRIBE CHAIN 264+70RT

Beginning chain 264+70RT description Feature: - 25 Scale Baselines

Point 126 325,853.82 E 3,614,919.64 Sta

Course from 126 to 127 S 81° 30′ 29.24′′ E Dist 80.00

N 325,842.01 E 3,614,998.76 Sta

Ending chain 264+70RT description

Entrance @ Sta.267+17.35 Fraley Blvd.

<* 8 DESCRIBE CHAIN 267+17RT</p>

Chain 267+17RT contains: 128 129

Beginning chain 267+17RT description Feature: - 25 Scale Baselines

Point 128 326,098.89 E 3,614,956.07 Sta 10+00.00

Course from 128 to 129 S 81° 48' 05.13" E Dist 120.00

Point 129 326,081.78 E 3,615,074.84 Sta 11+20.00

Ending chain 267+17RT description

Entrance @ Sta.271+89.00 Fraley Blvd.

<* 9 DESCRIBE CHAIN 271+89LT</pre>

Chain 271+89LT contains: 130 131 132

Beginning chain 271+89LT description Feature: - 25 Scale Baselines

326,568.63 E 3,614,996.61 Sta Point 130 10+00.00

Course from 130 to 131 N 85° 58' 25.85" W Dist 93.23

326,575.17 E 3,614,903.61 Sta 10+93.23

Course from 131 to 132 N 79° 27' 50.28" W Dist 156.77

326,603.84 E 3,614,749.48 Sta 12+50.00

______ Ending chain 271+89LT description

NTS

0001-212-249

SHEET NO.

PROJECT

PROJECT MANAGER <u>Hoainam Nguyen, P.E. (703) 792-8161 PWC Dept. of Transportation</u> SURVEYED BY, DATE Leon E.Treutle LS _(703)259-3224 7/17/13_____ DESIGN BY JMT_ Engineering_(804).323-9900______ SUBSURFACE UTILITY BY, DATE Lean F. Treutle LS (703)259-3224_7/17/L3

REVISED STATE ROUTE PROJECT 0001-212-249, IG(6) VA. RW-201, C-501

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Johnson, Mirmiran & Thompson Richmond, Virginia **ROADWAY ENGINEER**

Entrance Alignment Construction Data

Mall Entrance #1 (Sheet 6) © Sta. 274+15.00 Fraley Blvd.

<* 1 DESCRIBE CHAIN 274+15LT</pre>

Chain 274+15LT contains: 133 134

Beginning chain 274+15LT description Feature: - 25 Scale Baselines

326,792.51 E Point 133 3,615,026.51 Sta 10+00.00

Course from 133 to 134 N 86° 05'53.48" W Dist 400.00

326,819.73 E 3,614,627.44 Sta 14+00.00

-----Ending chain 274+15LT description

Mall Entrance #2 (Sheet 6B) @ Sta.12+16.60 of Mall Entr #1

2 DESCRIBE CHAIN MALLRT

Chain MALLRT contains: MALLRT1 MALLRT2

Beginning chain MALLRT description Feature: - 25 Scale Baselines ______

Point MALLRT1 N 326,807.25 E 3,614,810.41 Sta 20+00.00

Course from MALLRT1 to MALLRT2 N 10° 48' 48.67" E Dist 300.00

Point MALLRT2 327,101.93 E 3,614,866.70 Sta

______ Ending chain MALLRT description

Mall Entrance #3 (Sheet 6B) © Sta.12+80.41 of Mall Entr #1

<* 3 DESCRIBE CHAIN MALLPARK2</p>

Chain MALLPARK2 contains: MALLPARK22

Beginning chain MALLPARK2 description Feature: - 25 Scale Baselines

Course from MALLPARK21 to MALLPARK22 N 17° 37' 50.73" E Dist 125.00

Point MALLPARK21 N 326,811.59 E 3,614,746.75 Sta

Point MALLPARK22 N 326,930.72 E 3,614,784.61 Sta

40+00.00

______ Ending chain MALLPARK2 description

Mall Entrance #4 (Sheet 6B) @ Sta.21+34.00 of Mall Entr #2

4 DESCRIBE CHAIN MALLPARK1

Chain MALLPARK1 contains: MALLPARK11 MALLPARK12

Beginning chain MALLPARK1 description Feature: - 25 Scale Baselines

Point MALLPARK11 N 327,007.29 E 3,614,848.62 Sta 50+00.00

Course from MALLPARK11 to MALLPARK12 S 70° 02'48.85" E Dist 70.00

326,983.40 E 3,614,914.42 Sta Ν Point MALLPARK12 50+70.00

Ending chain MALLPARK1 description

Mall Entrance #5 (Sheet 6B) @ Sta. 22+03.65 of Mall Entr #2

5 DESCRIBE CHAIN MALL2

Chain MALL2 contains: MALL21 MALL22

Beginning chain MALL2 description

Point MALL21 N 326,938.87 E 3,614,835.55 Sta 30+00.00

Course from MALL21 to MALL22 N 80° 06' 56.60" W Dist 60.00

326,949.17 E 3,614,776.44 Sta Point MALL22

______ Ending chain MALL2 description

> PROJECT SHEET NO. NTS IG(6) 0001-212-249

VA.

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RW-201, C-501

PROJECT

IG(7)

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PROJECT MANAGER <u>Hoainam Nguyen, P.E. (703) 792-8161 PWC Dept. of Transportation</u> SURVEYED BY, DATE Leon E. Treutle LS 1/2031259-3224 7/17/13_____ DESIGN BY JMT_ Engineering_(804) 323-9900_______ SUBSURFACE UTILITY BY, DATE Leon E. Treutle LS (703)259-3224 7/17/13

Entrance Alignment Construction Data

Entrance @ Sta.286+14.02 Fraley Blvd.

1 DESCRIBE CHAIN 286+14LT

Chain 286+14LT contains: 139 140

Point 139

Beginning chain 286+14LT description Feature: - 25 Scale Baselines

Course from 139 to 140 N 49° 43' 16.14" W Dist 100.00

3,615,499.60 Sta Point 140 327,904.78 E 11+00.00

327,840.13 E

3,615,575.89 Sta

10+00.00

11+20.00

______ Ending chain 286+14LT description

Entrance @ Sta.286+81.78 Fraley Blvd.

2 DESCRIBE CHAIN 286+82LT

Chain 286+82LT contains: 137 138

Beginning chain 286+82LT description

327,891.82 E Point 137 3,615,619.70 Sta 10+00.00

Course from 137 to 138 N 49° 43' 16.14" W Dist 120.00

Point 138 327,969.41 E

Ending chain 286+82LT description

Entrance @ Sta.288+96.70 Fraley Blvd.

<* 3 DESCRIBE CHAIN 288+97RT</p>

Chain 288+97RT contains: 141 142

Beginning chain 288+97RT description Feature: - 25 Scale Baselines ______

328,055.79 E 3,615,758.65 Sta 10+00.00 Point 141

Course from 141 to 142 S 49° 43' 16.14" E Dist 100.00

Point 142 327,991.14 E 3,615,834.94 Sta 11+00.00

Ending chain 288+97RT description

Entrance @ Sta.289+78.30 Fraley Blvd.

<* 4 DESCRIBE CHAIN 289+78LT</pre>

Chain 289+78LT contains: 143 144

Beginning chain 289+78LT description Feature: - 25 Scale Baselines

Point 143 328,118.04 E 3,615,811.40 Sta 10+00.00

Course from 143 to 144 N 76° 30' 03.94" W Dist 125.00

Point 144 328,147.22 E 3,615,689.86 Sta 11+25.00

______ Ending chain 289+78LT description

Entrance @ Sta.293+98.86 Fraley Blvd.

DESCRIBE CHAIN 294+00RT

Chain 294+00RT contains: 147 148

Beginning chain 294+00RT description Feature: - 25 Scale Baselines

328,438.89 E 3,616,083.30 Sta

3,616,178.66 Sta

10+00.00

11+25.00

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Point 147

Course from 147 to 148 S 49° 43' 16.14" E Dist 125.00

______ Ending chain 294+00RT description

Entrance @ Sta.294+30.42 Fraley Blvd.

6 DESCRIBE CHAIN 294+30LT

Chain 294+30LT contains: 145 146

Point 148

Beginning chain 294+30LT description

Feăture: - 25 Scale Baselines

Point 145 328,462.97 E 3,616,103.70 Sta 10+00.00

328,358.08 E

Course from 145 to 146 N 49° 43' 16.14" W Dist 100.00

328,527.62 E Point 146 3,616,027.41 Sta 11+00.00

______ Ending chain 294+30LT description

Entrance @ Sta.296+75.85 Fraley Blvd.

7 DESCRIBE CHAIN 296+75RT

Chain 296+75RT contains: 151 152

Beginning chain 296+75RT description Feature: - 25 Scale Baselines ______

328,650.21 E 3,616,262.38 Sta 10+00.00

Course from 151 to 152 S 49° 43' 16.14" E Dist 100.00

Point 152 3.616.338.67 Sta

Ending chain 296+75RT description

Entrance @ Sta.297+02.60 Fraley Blvd.

8 DESCRIBE CHAIN 297+03LT

Chain 297+03LT contains:

Beginning chain 297+03LT description

Feăture: - 25 Scale Baselines ______

328,670.62 E 3,616,279.67 Sta 10+00.00

Course from 149 to 150 N 49° 43' 16.14" W Dist 110.00

Point 150 328,741.73 E 3,616,195.75 Sta 11+10.00

______ Ending chain 297+03LT description

Entrance @ Sta.298+34.52 Fraley Blvd.

<* 9 DESCRIBE CHAIN 298+35LT</pre>

Chain 298+35LT contains:

Beginning chain 298+35LT description Feăture: - 25 Scale Baselines

Point 153 328,771.26 E 3,616,364.96 Sta 10+00.00

Course from 153 to 154 N 49° 43' 16.14" W Dist 150.00

Point 154 328,868.24 E 3,616,250.52 Sta 11+50.00

PLANS

______ Ending chain 298+35LT description

THESE PLANS ARE UNFINISHED AND UNAPPROVED AND ARE NOT TO BE USED FOR ANY TYPE OF CONSTRUCTION OR THE ACQUISITION OF RIGHT OF WAY.

SHEET NO. NTS 0001-212-249

PROJECT

PROJECT MANAGER <u>Hoainam Nguyen, P.E. (703) 792-8161 PWC Dept. of Transportation</u> SURVEYED BY, DATE Leon E. Treutle LS 1/2031259-3224 7/17/13_____ DESIGN BY JMT_Engineering_(804) 323-9900_______ SUBSURFACE UTILITY BY, DATE Lean E. Treutle LS (703)259-3224_7/17/13

Entrance Alignment Construction Data

Entrance @ Sta.300+69.38 Fraley Blvd.

<* 1 DESCRIBE CHAIN 300+69LT</pre>

Chain 300+69LT contains: 155 156

Beginning chain 300+69LT description Feature: - 25 Scale Baselines

-----328,952.03 E 3,616,514.86 Sta

Course from 155 to 156 N 51° 45' 54.51" W Dist 120.00

329,026.30 E 3,616,420.61 Sta Point 156 11+20.00 -----

Ending chain 300+69LT description

Entrance @ Sta.303+54.14 Fraley Blvd.

2 DESCRIBE CHAIN 303+54RT

Chain 303+54RT contains: 157 158

Beginning chain 303+54RT description

329,178.82 E Point 157 3,616,687.05 Sta 10+00.00

Course from 157 to 158 S 49° 28' 28.49" E Dist 120.00

329,100.84 E Point 158 3,616,778.27 Sta

______ Ending chain 303+54RT description

Entrance @ Sta.317+22.00 Fraley Blvd.

<* 3 DESCRIBE CHAIN 317+22RT</p>

Chain 317+22RT contains:

Beginning chain 317+22RT description Feature: - 25 Scale Baselines ______

330,226.25 E 3,617,566.64 Sta

Course from 159 to 160 S 49° 10' 10.77" E Dist 100.00

330,160.87 E 3,617,642.31 Sta Point 160 11+00.00

______ Ending chain 317+22RT description

Entrance @ Sta.320+33.00 Fraley Blvd.

<* 4 DESCRIBE CHAIN 320+33RT</pre>

Chain 320+33RT contains: D810 D811

Beginning chain 320+33RT description

N 330,455.73 E 3,617,776.48 Sta Point D810 10+00.00

Course from D810 to D811 S 45° 52' 09.75" E Dist 140.76

N 330,357.72 E 3,617,877.51 Sta Point D811

Ending chain 320+33RT description

Entrance @ Sta.334+23.00 Fraley Blvd.

5 DESCRIBE CHAIN 334+23LT

Chain 334+23LT contains: 195 CUR 334+23LT_3 196

Beginning chain 334+23LT description

Feăture: - 25 Scale Baselines

______ 331,508.09 E 3,618,684.22 Sta 10+00.00

Course from 195 to PC 334+23LT_3 N 49° 43'16.14" W Dist 194.41

Curve Data

Curve 334+23LT_3 331,644.93 E 3,618,522.75 Degree Tangent External = Long Chord = Mid. Ord. = 11+94.41 N P.C. Station P.T. Station C.C. 331,633.78 E = N 49° 43'16.14'' W = N 11° 39'09.64'' W Chord Bear = N 30° 41' 12.89'' W Course from PT 334+23LT_3 to 196 N 11° 39'09.64" W Dist 12.37 331,673.94 E 3,618,516.76 Sta 12+40.00

Ending chain 334+23LT description

Entrance @ Sta.335+83.43 Fraley Blvd.

6 DESCRIBE CHAIN 335+83LT

Chain 335+83LT contains: 238 239

Beginning chain 335+83LT description Feature: - 25 Scale Baselines

331,630.45 E 3,618,787.97 Sta

Course from 238 to 239 N 45° 53' 03.99" W Dist 175.02

331,752.28 E 3,618,662.32 Sta Point 239 11+75.00

Ending chain 335+83LT description

Entrance @ Sta.336+62.06 Fraley Blvd.

<* 7 DESCRIBE CHAIN 336+62LT</pre>

Chain 336+62LT contains: 242 243

Beginning chain 336+62LT description Feature: - 25 Scale Baselines

Point 242 331,690.08 E 3,618,839.23 Sta 9+99.74

Course from 242 to 243 N 46° 03' 30.49" W Dist 160.26

Point 243 331,801.29 E 3,618,723.84 Sta 11+60.00

Ending chain 336+62LT description

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Johnson, Mirmiran & Thompson Richmond, Virginia **ROADWAY ENGINEER**

> SHEET NO. NTS 0001-212-249

VA.

DESIGN FEATURES RELATING TO CONSTRUCTION OR TO REGULATION AND CONTROL OF TRAFFIC

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STATE

0001-212-249,

RW-201, C-501

PROJECT

IG(9)

REVISED

Johnson, Mirmiran & Thompson Richmond, Virginia

ROADWAY ENGINEER

PROJECT MANAGER <u>Hoainam Nguyen, P.E. (703) 792-8161 PWC Dept. of Transportation</u> SURVEYED BY, DATE Leon E. Treutle LS 1/2031259-3224 7/17/13_____ DESIGN BY JMT_Engineering_(804) 323-9900_______ SUBSURFACE UTILITY BY, DATE Lean E. Treutle LS_(703)259-3224_7/17/L3

Entrance Alignment Construction Data

Entrance @ Sta.337+36.08 Fraley Blvd.

<* 8 DESCRIBE CHAIN 337+36LT</p>

Chain 337+36LT contains: 244 245

Point 244

Beginning chain 337+36LT description

Feăture: - 25 Scale Baselines

Course from 244 to 245 N 47° 49' 23.92" W Dist 150.48

Point 245 331,847.25 E 11+50.00

331,746.21 E

3,618,887.48 Sta

9+99.52

______ Ending chain 337+36LT description

Entrance @ Sta.341+35.82 Fraley Blvd.

9 DESCRIBE CHAIN 341+36LT

Chain 341+36LT contains: 254 255

Beginning chain 341+36LT description Feature: - 25 Scale Baselines

Point 254 332,049.36 E 9+98.30 3,619,148.06 Sta

Course from 254 to 255 N 49° 29' 31.56" W Dist 121.70

Point 255 332,128.41 E 3,619,055.53 Sta 11+20.00

Ending chain 341+36LT description

Entrance @ Sta.344+48.85 Fraley Blvd.

<* 10 DESCRIBE CHAIN 344+49RT</p>

Chain 344+49RT contains: 256 257

Beginning chain 344+49RT description Feature: - 25 Scale Baselines

______ 332,286.73 E 3,619,352.11 Sta Point 256 10+00.00

Course from 256 to 257 S 49° 29' 31.56" E Dist 97.35

332,223.50 E 3,619,426.12 Sta Point 257 10+97.35

______ Ending chain 344+49RT description

Entrance @ Sta.345+20.23 Fraley Blvd.

(* 11 DESCRIBE CHAIN 345+20LT

Chain 345+20LT contains: 258 259

Beginning chain 345+20LT description Feature: - 25 Scale Baselines

Point 258 332,340.87 E 3,619,398.64 Sta 9+97.13

Course from 258 to 259 N 49° 29' 31.56" W Dist 92.87

332,401.19 E 3,619,328.03 Sta

Ending chain 345+20LT description

Entrance @ Sta.346+22.88 Fraley Blvd.

12 DESCRIBE CHAIN 346+23LT

Chain 346+23LT contains: 262 263

Beginning chain 346+23LT description Feature: - 25 Scale Baselines

332,418.71 E 3,619,465.55 Sta 9+96.82 Point 262

Course from 262 to 263 N 49° 29' 31.56" W Dist 83.18

332,472.74 E Point 263 3,619,402.31 Sta 10+80.00

Ending chain 346+23LT description

Entrance @ Sta.347+02.16 Fraley Blvd.

* 13 DESCRIBE CHAIN 347+02RT

Chain 347+02RT contains: 264 267

Beginning chain 347+02RT description

332,478.84 E 3,619,517.23 Sta 10+00.00

Course from 264 to 267 S 49° 29' 31.56" E Dist 124.54

332,397.94 E 3,619,611.92 Sta 11+24.54

-----Ending chain 347+02RT description

Entrance @ Sta.348+86.80 Fraley Blvd.

14 DESCRIBE CHAIN 348+87LT

Chain 348+87LT contains: 270 271

Beginning chain 348+87LT description

Feature: - 25 Scale Baselines ______

Point 270 332,619.45 E 3,619,636.90 Sta

Course from 270 to 271 N 49° 29' 31.56" W Dist 93.07

Point 271 332,679.90 E 3,619,566.14 Sta 10+90.00

Ending chain 348+87LT description

Chain 349+83RT contains: 272 273

Beginning chain 349+83RT description Feature: - 25 Scale Baselines

3,619,698.77 Sta Point 272 332,692.42 E 10+00.00

Course from 272 to 273 S 49° 29' 31.56" E Dist 77.30

Point 273 Ν 332,642.21 E 3,619,757.54 Sta 10+77.30

Ending chain 349+83RT description

Entrance @ Sta.351+64.88 Fraley Blvd.

16 DESCRIBE CHAIN 351+65LT

Chain 351+65LT contains: 274 277

Beginning chain 351+65LT description

Point 274 332,830.94 E 3,619,817.45 Sta

Course from 274 to 277 N 49° 29' 31.56" W Dist 92.96

332,891.33 E 3,619,746.77 Sta 10+90.00 ______

Ending chain 351+65LT description

SHEET NO. 0001-212-249 NTS

PROJECT MANAGER <u>Hoainam Nguyen, P.E. (703) 792-8161 PWC Dept. of Transportation</u> SURVEYED BY, DATE Leon E.Treutle LS _(703)259-3224 7/17/13_____ DESIGN BY JMT_Engineering_(804) 323-9900_______ SUBSURFACE UTILITY BY, DATE Lean F. Treutle LS (703)259-3224 7/17/13

REVISED ROUTE PROJECT 0001-212-249, VA. IG(IO) RW-201, C-501

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Johnson, Mirmiran & Thompson Richmond, Virginia **ROADWAY ENGINEER**

Side Road Entrance Alignment Construction Data

Entrance @ Sta. II+29.80 Graham Park Rd. Rt (Sheet 7)

(* 1 DESCRIBE CHAIN ENTR11+29

Chain ENTR11+29 contains: 189 190

Beginning chain ENTR11+29 description Feature: - 25 Scale Baselines

Point 189

327,674.56 E 3,615,602.88 Sta

20+00.00

Course from 189 to 190 S 43° 05' 59.69" W Dist 120.00

Point 190

327,586.94 E

3,615,520.89 Sta

21+20.00

Ending chain ENTR11+29 description

Entrance @ Sta.12+42.62 Graham Park Rd.Rt (Sheet 7C)

<* 2 DESCRIBE CHAIN ENTR12+42</pre>

Chain ENTR12+42 contains: 135 136

Beginning chain ENTR12+42 description Feature: - 25 Scale Baselines

327,618.48 E

3,615,700.78 Sta

Course from 135 to 136 S 41° 58' 03.23" W Dist 100.00

Point 136

Point 135

327,544.13 E

3,615,633.91 Sta

31+00.00

40+00.00

30+00.00

______ Ending chain ENTR12+42 description

Entrance @ Sta.13+01.56 Graham Park Rd. Rt

(Sheet 7C)

<* 3 DESCRIBE CHAIN ENTR13+02</pre>

Chain ENTR13+02 contains: ENTR141 CUR ENTR13+021 ENTR142

Beginning chain ENTR13+02 description

327,589.19 E Point ENTR141 N 3,615,751.93 Sta

Course from ENTR141 to PC ENTR13+021 N 29° 48' 18.42" E Dist 40.09

Curve Data

Curve ENTR13+021 327,647.34 E 3,615,785.24

External = Mid. Ord. = P.C. Station 40+40.09 N 40+93.61 N

Course from PT ENTR13+021 to ENTR142 N 45° 08' 14.57" E Dist 70.43

Point ENTR142

327,716.01 E

3,615,854.24 Sta

Ending chain ENTR13+02 description

Entrance @ Existing Duke St. Turn Around (Sheet 8)

DESCRIBE CHAIN DUKETURN

Chain DUKETURN contains: ENTR315 ENTR316

Point ENTR315

Beginning chain DUKETURN description Description: Duke St. turn lane alignment.

Course from ENTR315 to ENTR316 S 65° 12'13.09" E Dist 46.71

Point ENTR316 328,268.56 E 3,615,847.26 Sta 10+46.71

328,288.14 E

3,615,804.86 Sta

10+00.00

______ Ending chain DUKETURN description

Entrance @ Sta.20+49.80 Canal Service Rd. (Sheet 13)

5 DESCRIBE CHAIN ENTR20+49

Chain ENTR20+49 contains:

Beginning chain ENTR20+49 description Feature: - 25 Scale Baselines

330,525.22 E 3,618,045.93 Sta Point 193 10+00.00

Course from 193 to 194 S 59° 33' 45.34" E Dist 50.00

Point 194 330,499.89 E 3,618,089.04 Sta 10+50.00

______ Ending chain ENTR20+49 description

Entrance @ Sta.103+57.90 Reloc.Main St. N. (Sheet 13)

DESCRIBE CHAIN ENTR103+57

Chain ENTR103+57 contains:

Beginning chain ENTR103+57 description

Feăture: - 25 Scale Baselines ______

Point 163 10+00.00 330,878.96 E 3,617,918.28 Sta

Course from 163 to 164 N 26° 17' 39.14" W Dist 70.00

3,617,887.27 Sta 330,941.72 E 10+70.00

Ending chain ENTR103+57 description

PROJECT SHEET NO. NTS 0001-212-249 IG(10)

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Johnson, Mirmiran & Thompson Richmond, Virginia ROADWAY ENGINEER

Side Road Entrance Alignment Construction Data

Entrance @ Sta.12+92.53 Main St.RT Lane (Sheet 14)

<* 1 DESCRIBE CHAIN ENTR12+92</pre>

Chain ENTR12+92 contains: 167 168

Beginning chain ENTR12+92 description Feature: - 25 Scale Baselines

Point 167 N 331,066.88 E 3,618,183.23 Sta 10+00.00

Course from 167 to 168 N 34° 43' 02.50'' W Dist 80.00

Point 168 N 331,132.64 E 3,618,137.66 Sta 10+80.00

Ending chain ENTR12+92 description

Entrance @ Sta.102+32.36 Orginal Stage Coach Rd. (Sheet 15)

<* 2 DESCRIBE CHAIN ENTR102+32</pre>

Chain ENTR102+32 contains: 282 283

Beginning chain ENTR102+32 description

Point 282 N 331,602.29 E 3,618,922.16 Sta 10+00.00

Course from 282 to 283 N 15° 46′ 24.66′′ W Dist 55.00

Point 283 N 331,655.22 E 3,618,907.21 Sta 10+55.00

Ending chain ENTR102+32 description

Entrance @ Sta.102+71.74 Orginal Stage Coach Rd. (Sheet 15)

<* 3 DESCRIBE CHAIN ENTR102+71</p>

Chain ENTR102+71 contains: 248 249

Beginning chain ENTR102+71 description

Feăture: - 25 Scale Baselines

Point 248 N 331,591.66 E 3,618,884.53 Sta 0+00.00

Course from 248 to 249 S 15° 46' 24.66" E Dist 30.00

Point 249 N 331,562.79 E 3,618,892.69 Sta 0+30.00

Ending chain ENTR102+71 description

Entrance @ Sta.100+55.17 Old Stage Coach Rd. (Sheet 15C)

4 DESCRIBE CHAIN ENTR100+55

Chain ENTR100+55 contains: 246 247

Beginning chain ENTR100+55 description Feature: - 25 Scale Baselines

Feăture: - 25 Scale Baselines

Point 246 N 331,660.37 E 3,619,088.11 Sta 10+00.00

Course from 246 to 247 S 50° 05' 55.42" E Dist 80.00

Point 247 N 331,609.05 E 3,619,149.49 Sta 10+80.00

Ending chain ENTR100+55 description

Entrance @ Sta.10+91.52 of Entr 347+02.16 Fraley Blvd. (Sheet 16)

5 DESCRIBE CHAIN ENTR10+94

Chain ENTR10+94 contains: 268 269

Point 268

Beginning chain ENTR10+94 description Feature: - 25 Scale Baselines

Course from 268 to 269 S 40° 30' 28.44" W Dist 60.00

Point 269 N 332,373.77 E 3,619,547.84 Sta 20+60.00

332,419.39 E

3,619,586.82 Sta

20+00.00

Ending chain ENTR10+94 description

NTS PROJECT SHEET NO. 1G(11)

Transportation Management Plan General Notes

Temporary Traffic Control Plan

General Notes:

TMP/SOC Type B Project Information:

a Identify the project's TMP Type:

This project's TMP/SOC plan has been designed in conformance with a Type B TMP/SOC plan.

Identify the work zone location, length, and widths: The project location is as shown on Sheet IA.

The work zone areas have been delineated as shown on the TMP/SOC plan sheets IK series series. The work zone lengths and widths vary by location as shown on the TMP/SOC plan sheets on the IK series.

c Note the hours the Construction Area will be active:

Construction Area shall be considered when any impact to traffic occurs (Ist Cone in Road).

Unless preapproved, the allowable hours for all lane and/or shoulder closures shall be in accordance with the applicable hours as stated in the Lane Closures in NOVA District Memorandum, dated September 29, 2016. All lane and/or shoulder closures shall be completely removed on a daily basis with all lanes being fully open to traffic at all times beyond the allowable and/or approved lane closure hours.

Any/all requests for deviation from the allowable lane closure hours, regardless of prior approval, shall be submitted to VDOT NOVA District Traffic Operations Engineering for review a minimum of fourteen (14) days in advance of the work.

Lane closures or shoulder work shall not begin if heavy traffic or significant queuing and backups are already present along the roadways). Lane closures will not be permitted during inclement weather or emergency situations.

The contractor shall continually monitor all lane closures and/or detour routes, if applicable, at all times and make spot adjustments as needed to ease undue backups, delays, or queuing and reopen lanes immediately, if necessary.

All lane and/or shoulder closure hours of operation may be adjusted by VDOT at any time if significant traffic impacts routinely develop as a result of the project or citizen complaints are received.

Any anticipated or incidental impacts to the existing VDOT maintained traffic signal network as a result of the project requires notification and coordination with VDOT Traffic Operations Engineering/TOE through TOE Supervisor, Mr. Ta-Cheng Hsu.

- d The TMP/SOC plan, during construction, shall be in accordance with Sections 512,701,703 & 704 of the Virginia Department of Transportation Road and Bridge Specifications, dated 2020, the Virginia Work Area Protection Manual, dated 2011, Revised November 2020, the Manual on Uniform Traffic Control Devices (MUTCD), 2009 Edition Revised May 2012, and the Virginia Department of Transportation Road and Bridge Standards, dated 2016.
- e Note any existing entrances, existing intersections, or existing pedestrian access points that will be affected by the Construction Area or by the traffic control devices:

Access to and from all roadways, side streets, driveways and properties within and adjacent to the work area shall be maintained at all times throughout the duration of the project.

Existing Entrances:

All existing commercial or private entrances shall remain open for the duration of

construction unless otherwise indicated on this plan.

Existing Intersections:
There are four signalized intersections within the project limits. They are the intersections of:

Richmond Highway (Route I) @ Quantico Gateway Drive

Fraley Boulevard (Route I) @ Graham Park Road Fraley Boulevard (Route I) & Main Street @ Possum Point Road

Richmond Highway (Route I) & Dumfries Road (Route 234)

There are five unsignalized intersections within the limits of this project. They are the intersections of:

Richmond Highway (Route I) @ Townsquare Court Fraley Boulevard (Route I) @ Dr David Cline Lane

Fraley Boulevard (Route I) @ Williamstown Drive

Fraley Boulevard (Route I) @ Canal Road

Fraley Boulevard (Route I) @ Relocated Old Stage Coach Road & Tripoli Boulevard

All intersections are to remain open during construction unless otherwise indicated on this plan. Intersections may be reduced to one lane when the construction zone is active, using VWAPM TTC-28.2 or as approved by the Engineer. When the construction zone is not active, all intersections shall be open.

Existing Pedestrian Access Points:

When working in and around any crosswalks, sidewalks or trails, continuously monitor and maintain safe pedestrian and bicycle access at all times.

Within the project limits, pedestrian access points are very limited. Most pedestrian access points occur at the intersections. Where possible, pedestrians will be directed to cross Richmond Highway/Fraley Boulevard and use the pedestrain facilities on the other side of the street. For all other locations, the sidewalk is to be closed at the project's limits as there are not enough pedestrian paths within the projects to maintain until the project is completed. If requested by the Engineer, the project shall install a 4 orange safety fence wherever directed to discourage pedestrians from walking through the project, at no additional cost to the project.

Existing Bus Stops:

There are several public bus stops within the project limits. Coordination with PWC-DOT and PRTC is required to determine if any bus stops will require relocation during construction.

Along Northbound Richmond Highway/Fraley Boulaverd, Approximate Sta. 255.50, 293.50, 308.00, 329.00, 348.25 Along Southbound Richmond Highway/Fraley Boulevard, Approximate Sta. 329.50, 347.00

Identify the major types of travelers: The roadway carries large diverse types of travelers. In the peak hours however, commuters

are the prevailing traveler type for this roadway.

The Contractor, at no additional cost to the project, which shall be considered incidental to the cost of the project, shall:

Designate a person assigned to the project who will have the primary responsibility, with sufficient authority, for implementing the TMP/SOC and other safety and mobility aspects of the permit work. This person shall be designated the "Project Safety Officer." Ensure that personnel assigned to the project are trained in traffic control to a level commensurate with their responsibilities in accordance with VDOT's work zone traffic control training quidelines.

Inform the Engineer of any work requiring lane shifts, lane closures, and/or phase changes a minimum of two working days prior to implementing this activity.

Perform reviews of the Construction Area to ensure compliance with contract documents at regularly scheduled intervals at the direction of the Engineer. Contractor shall maintain a copy of the temporary traffic control plan at the work site at all times.

Coordinate with Prince William County Police Department, Prince William County Fire/Rescue Department, and Virginia State Police for any lane closures and any detours of any nature at least seven working days prior to implementing a lane closure.

Schedule all phases of construction in such a manner that water, sanitary sewer, cable, fiber cable/optic cable, any overhanging utilities, and any underground utility services will not be interrupted.

- h During working hours, all construction equipment is to stay outside of the construction area clear zone as designated in the VWAPM, Appendix A. Construction equipment is not to block or obstruct sight distance at any intersection or private entrance along the project when the construction work zone is active.
- Disposal Site And Staging Area Locations Shall Be The Responsibility Of The Contractor. No Work Shall Be Performed Until Such Sites Have Been Accepted By The Engineer. The Sites Cannot Be Placed On Any Wetlands, E&s Sediment Basins/traps, Or Other Environmentally Sensitive Areas.
- Contractor is to maintain two lanes of traffic (one in each direction) on Richmond Highway/Fraley Boulevard and is to maintain a minimum of one lane of traffic on all street connections when the construction zone is active unless otherwise specified by the Engineer. When the construction zone is not active, the Contractor shall ensure all street connections maintain a minimum of two lanes of traffic (one in each direction). During construction of this project, the travelway shall have a minimum clear roadway width in accordance with VDOT standard GS-10 unless otherwise approved by the Engineer. For commercial connections, or private entrances, a minimum width no less than the existing width shall be maintained at all times. unless approved by the Engineer.
- Concrete Traffic Barrier Service shall be installed and removed so as to not present any blunt end or hazard to the motoring public. The placement and removal of Concrete Traffic Barrier Service are to be coordinated by the Project Safety Officer. When Concrete Traffic Barrier Service is installed, Impact Attenuators shall be placed at the beginning of the Concrete l rattic Barrier Service.Project Satety Utticer shall ensure Concrete I rattic Barrier Service is installed in accordance with VDOT's and manufacturer's specifications to prevent deflection. Additionally, Project Safety Officer shall ensure Concrete Traffic Barrier Service, when installed with flares, shall be in accordance with VDOT's Virginia Work Area Protection Manual.

Note: There are portions of the project in which the Contractor will be working within 2' behind the Concrete Traffic Barrier Service. The Concrete Traffic Barrier Service shall be pinned in these locations and the Contractor shall implement VWAPM TTC 23.2 and TTC 24.2 when working in these areas.

- Contractor shall follow the geotechnical recommendations for the project. Materials designated as unsuitable material as detailed in the geotechnical recommendations shall be disposed of offsite and are not to be used for any part of construction. Existing surface, aggregate base, and sub base material which will be demolished or obliterated during construction and which are suitable for maintenance of traffic, should be utilized prior to the use of commercial material.
- Each phase of construction shall be completed to the installation of intermediate course asphalt prior to the start of the next phase unless otherwise directed by the Engineer.
- Contractor shall ensure positive drainage for the duration of the project. Contractor shall add any additional temporary measures necessary to facilitate proper, positive drainage for the duration of construction.
- Where Group 2 Channelizing Devices are used to separate the Construction Area and traffic.a minimum clear zone area as defined in the VWAPM is to be maintained.
- The Contractor is to coordinate with Prince William County for location(s) of the construction staging area(s). Contractor is responsible for obtaining easements and permits associated for these location(s). Contractor is solely responsible for the cost to acquire easement for staging area and it shall not be paid for as a separate item. Potential staging areas for construction are at stormwater management basins and storm water management
- All areas excavated below the existing pavement surface and within the clear zone at the conclusion of each workday shall be backfilled to form an approximate 6:1 wedge against the existing pavement or newly constructed pavement surface for the safety and protection of vehicular traffic. All costs for placing, maintaining, and removing 6:1 wedge shall be included in the price bid for other items in the contract and no additional compensation will be allowed.
- IMPLEMENTING THE TRANSPORTATION MANAGEMENT PLAN During the first day of the new work zone traffic pattern, the project's Manager and project's Maintenance of Traffic Coordinator shall inspect the work zone to ensure compliance with the TMP. On the third to fifth day of implementation of the TMP's new work zone traffic pattern, the District Work Zone Safety Coordinator and the project's Maintenance of Traffic Coordinator shall conduct an on-site review of the work zone's performance and recommend to the Contractor any required changes to the TMP to enhance the work zone's safety and mobility. All such changes shall be documented. An on-site review of the project's work zone traffic control by the District Work Zone Safety Coordinator, Project's Manager/Maintenance of Traffic Coordinator, District Safety Engineer, and the Contractor shall be conducted within 48 hours of any fatal incident/crash within the work zone.

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| | SIAIE | ROUTE | PROJECT | SHEET |
| | VA. | 1 | 0001-212-249, RW-201,C-501 | |
| DESIGN FEATU | RES RELA | ATING TO | CONSTRUCTION | |

DESIGN FEATURES RELATING TO CONSTRUCTION OR TO REGULATION AND CONTROL OF TRAFFIC MAY BE SUBJECT TO CHANGE AS DEEMED NECESSARY BY THE DEPARTMENT

Johnson, Mirmiran & Thompson Richmond, Virginia ROADWAY ENGINEER

EVALUATION OF THE TRANSPORTATION MANAGEMENT PLAN A performance assessment of the TMP including area-wide impacts on adjacent roadways shall be performed by the Regional Traffic Engineering and Operations sections during construction. As circumstances dictate, a review of the overall effectiveness of the project's TMP shall be completed during the Post Construction Meeting and included with the Post Construction Report. A copy of the specific information on the effectiveness of the TMP will be forwarded to the State Traffic Engineer for review. A copy of the TMP Interim/Post Construction Report Form can be obtained from the Regional Traffic Engineer.

- PUBLIC COMMUNICATIONS PLAN The Contractor shall be responsible for:
- a Notifying the Project Manager/Residency Administrator two weeks in advance of any scheduled work plans and traffic delays.
- b Notifying the Project Manager/Residency Administrator, Regional Operations Manager, and the

Public Affairs staff of any unscheduled traffic delays

The contractor shall be responsible for maintaining project lane closure information on LCAMS and VaTraffic/511 throughout the duration of the project in accordance with IIM-OD-16-03, dated December 16, 2016.

All lane and/or shoulder closures shall be entered into LCAMS at least ten (10) days in advance of the proposed lane and/or shoulder and no later than close of business Wednesday the week prior to the closure stating the location purpose specific lane(s) to be closed time and duration of closure. Any conflicts generated from LCAMS shall be resolved no later than close of business Thursday the week prior to the closure.

The contractor shall contact the VDOT NOVA District Transportation Operations Center (TOC) 15-45 minutes prior to executing every lane and/or shoulder closure and contact TOC again once daily work has been completed and lane and/or shoulder closures have been removed.

The contractor shall notify the Virginia State Police (VSP), if necessary, and the various local area law enforcement and emergency services of all closures and times for situational awareness.

Depending on the time of year, the contractor shall notify and coordinate with the local area schools of all closures and times to avoid issues with school building access and/or bus schedules and routes...

TRANSPORTATION OPERATIONS

The Contractor shall be responsible for implementing and providing the following:

a The Contractor Must Contact The VDOT Northern Region Transportation Operations Center (TOC15-45 Minutes Prior To Executing Lane And/OR Shoulder Closures And Contact The Toc 15-45 Minutes After The Work Has Been Completed And Lane And/or Shoulder Closures Have Been Removed.

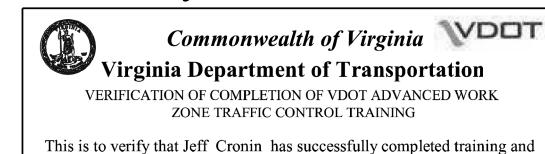
The Contractor Must Contact TOC One Week In Advance For Lane Closure Information. This Is Usually By The Wednesday Prior To The Operation. The Contractor Shall Have A Person Designated As The Point Of Contact And Should Be Trained To Place The Required Information In LCAMS.

- b Post a list of local emergency response agencies inside the project's construction office/trailer.
- c Immediately report any traffic incidents that may occur in the work zone.
- Notify the project's Maintenance of Traffic Coordinator, Project Manager, Resident Administrator, District Work Zone Safety Coordinator, District Traffic Engineer, The Regional Operations Manager and Public Affairs Manager of any incidents and expected traffic delays.
- e Within 24 hours of any incidents within the construction work zone, a review of the traffic controls shall be completed and necessary adjustments made to reduce the frequency and severity of any future incidents.

CONTACT NUMBERS

Hoainam Nguyen, P.E. Mohammad Ayyoubi (703) 792-8161 Project Manager (703) 792-7193 Construction Manager Construction Safety Manager **TBD** TBD Public Relations TBD VDOT Residency Administrator - Construction TBD TBD TBD District Work Zone Safety Coordinator(s) Emergency Call Non-Emergency Numbers: Prince William County Police (703) 792-6500

TMP/SOC Designer



Prince William County Fire & Rescue

This is to verify that Jeff Cronin has successfully completed training and an examination by the Department on the proper practices and methods for the installation, maintenance, removal of temporary traffic control devices and flagging operations.

R. J. Khour 9/30/2025 **Expiration Date: Verification Number:** 093021124 State Traffic Engineer

SHEET NO. **PROJECT** 0001-212-249

(703) 792-6800

PLANS

PROJECT MANAGER <u>Hoainam Nguyen, P.E.(703) 792-8161 PWC Dept. of Transportation</u> SURVEYED BY, DATE <u>Leon E.Treutle LS (703)259-3224 7/I7/I3</u>

DESIGN BY <u>JMT Engineering (804) 323-9900</u>

SUBSURFACE UTILITY BY, DATE <u>Leon E.Treutle LS (703)259-3224 7/I7/I3</u>

Sequence of Construction Narrative

| REVISED | STATE | | STATE | SHEET NO. |
|---------|-------|-------|-------------------------------|-----------|
| | SIAIE | ROUTE | PROJECT | SHEET NO. |
| | VA. | / | 0001-212-249, RW-201,C-501 | IK(I) |

DESIGN FEATURES RELATING TO CONSTRUCTION OR TO REGULATION AND CONTROL OF TRAFFIC MAY BE SUBJECT TO CHANGE AS DEEMED NECESSARY BY THE DEPARTMENT

PHASE I

-Leave NB and SB traffic in existing locations; maintain 2 lanes in each direction with a minimum II' width. Set up temporary construction devices and erosion control measures.

-Begin construction of drainage as shown for Phase I and proceed with construction on the west side of the existing NB lanes, including Graham Park Road Left, North Main Street tie-in, Tripoli Blvd. and Waters Lane. Construct pavement in the median of NB Rte Lat intersection with Main Street/Possum Point Road. Place pavement up to the intermediate layer.

-Construct stormwater management ponds and retaining walls on the west side.

-Construct the bridge on the west side, see Bridge plans.

PHASE 2

-Revise temporary construction devices and shift NB traffic onto newly constructed lanes from Phase I; maintain 2 lanes in each direction with a minimum II' width.

-Begin construction on the east side of the NB lanes, including Graham Park Road Right, Williamstown Drive, Canal Road, Possum Point Road, Relocated Old Stage Coach Road Cul-de-sac. Place pavement up to the intermediate layer. -Construct stormwater management ponds and retaining walls on the east side.

-Construct the bridge on the east side, see Bridge plans.

PHASE 2A

-Revise temporary construction devices and shift NB traffic onto 2 minimum II' width newly constructed lanes from Phase 2. Complete a minimum 2-II lanes on west side from sta. 278+00 to sta. 303+00 for use by shifted SBL traffic in Phase 3.

PHASE 3

-Revise temporary construction devices and shift SB traffic onto newly constructed lanes from Phase I and 2A; maintain 2 lanes in each direction with a minimum II' width.

-Finalize construction of median.

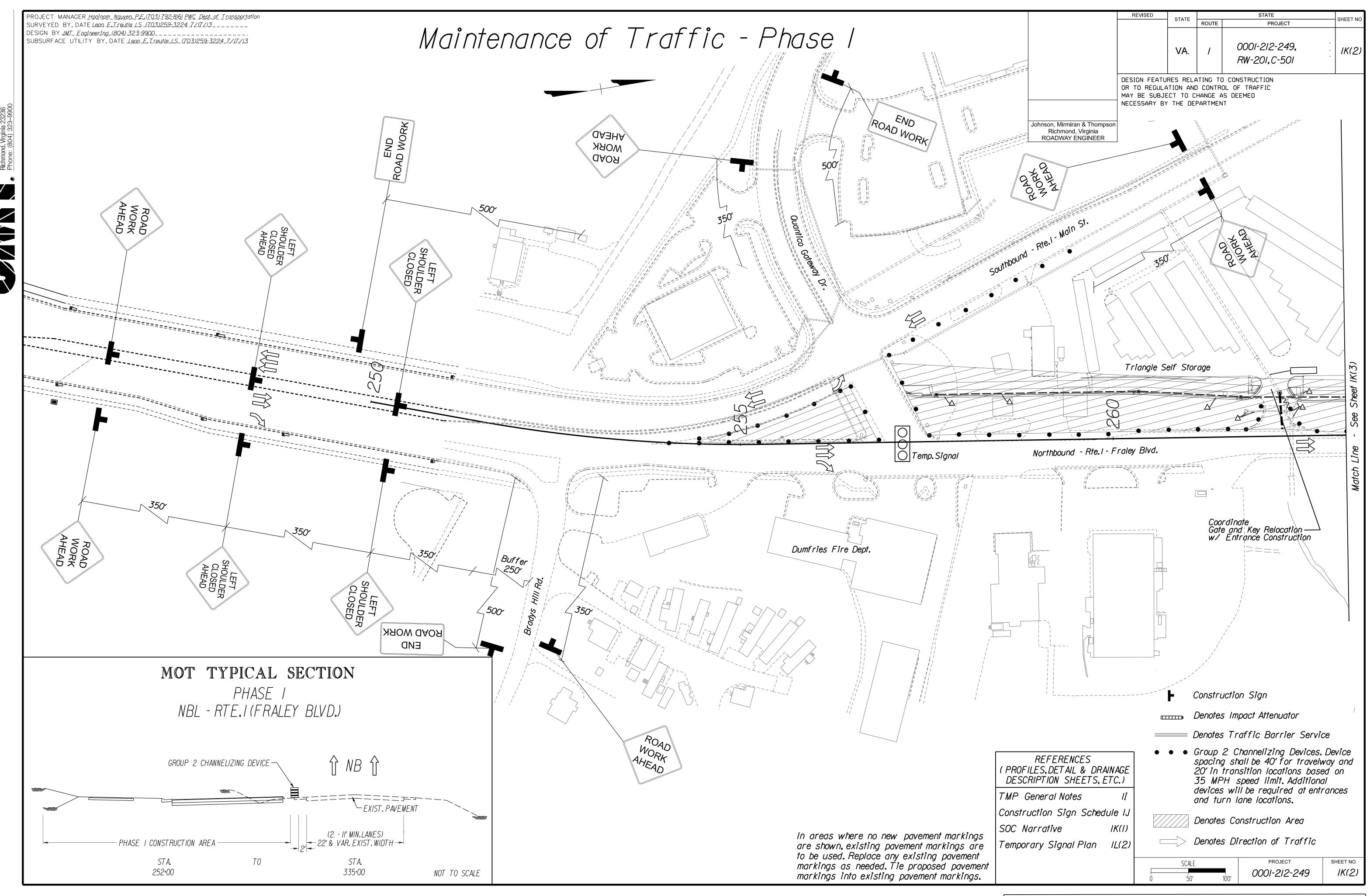
-Construct Quantico Gateway Drive, Main Street South Cul-de-sac and Main Street North tie-in.

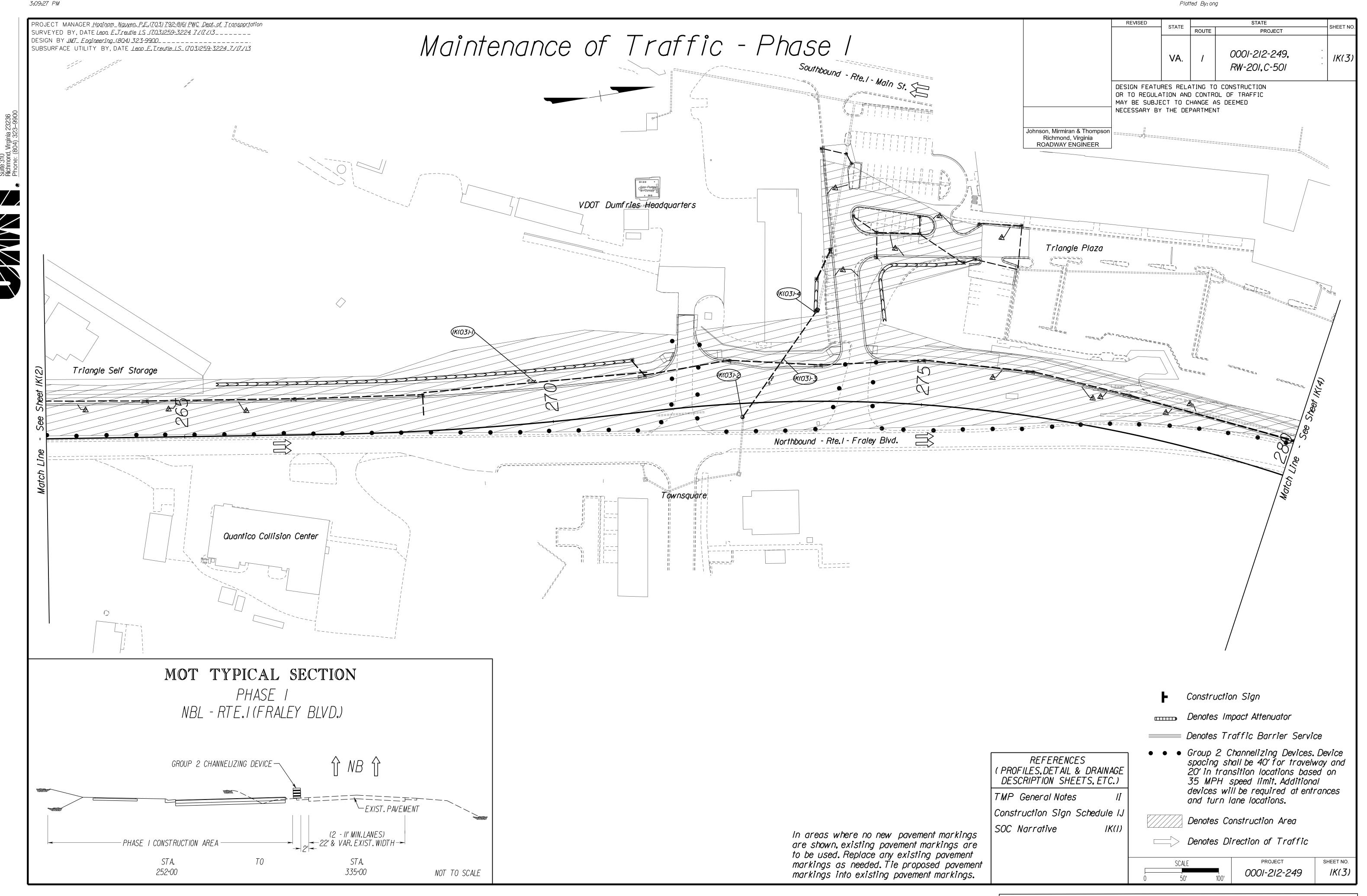
-Finalize all stormwater management ponds.

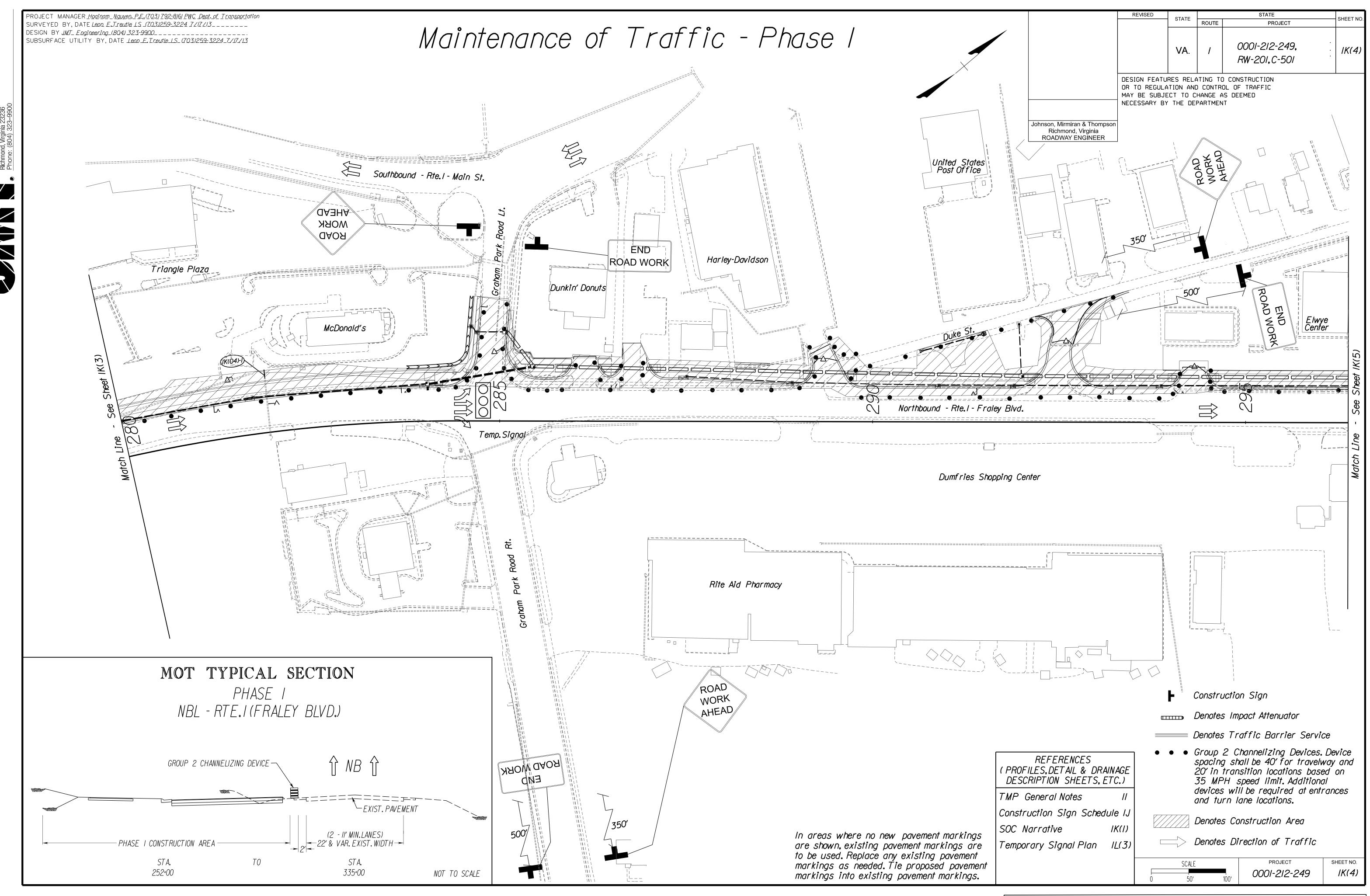
-Finalize the bridge construction, see Bridge plans

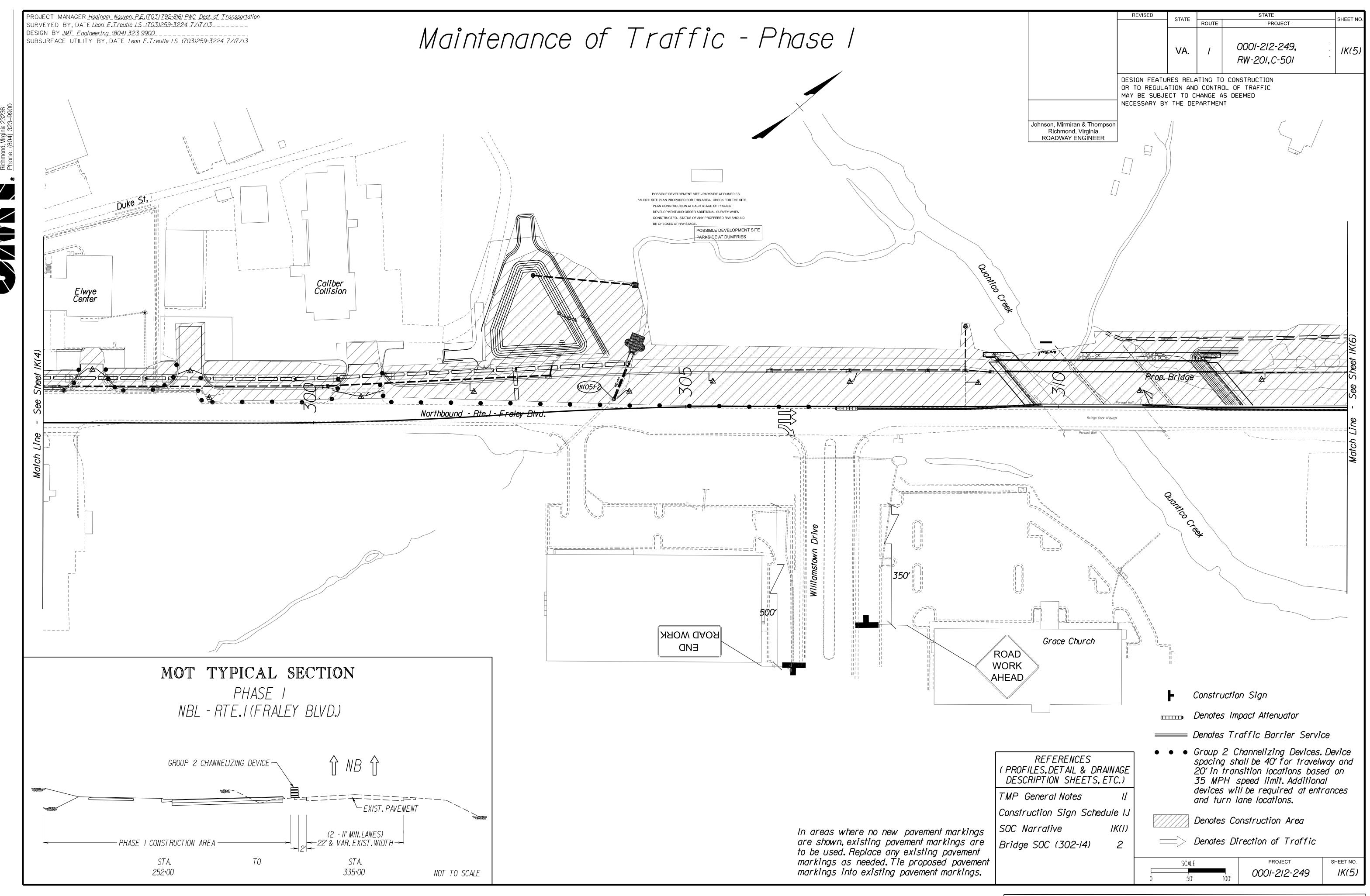
-Place final pavement surface layer and pavement markings.

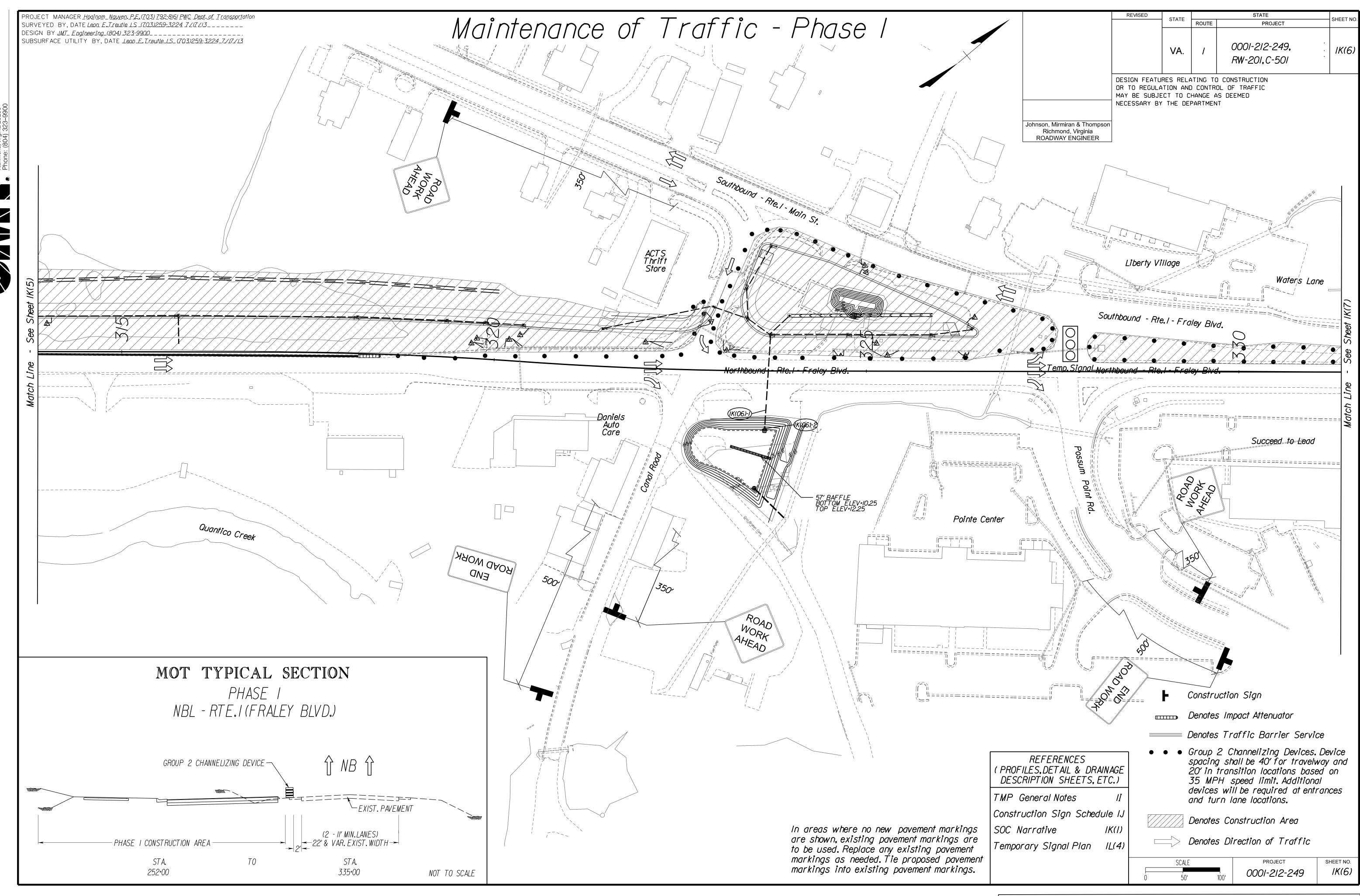
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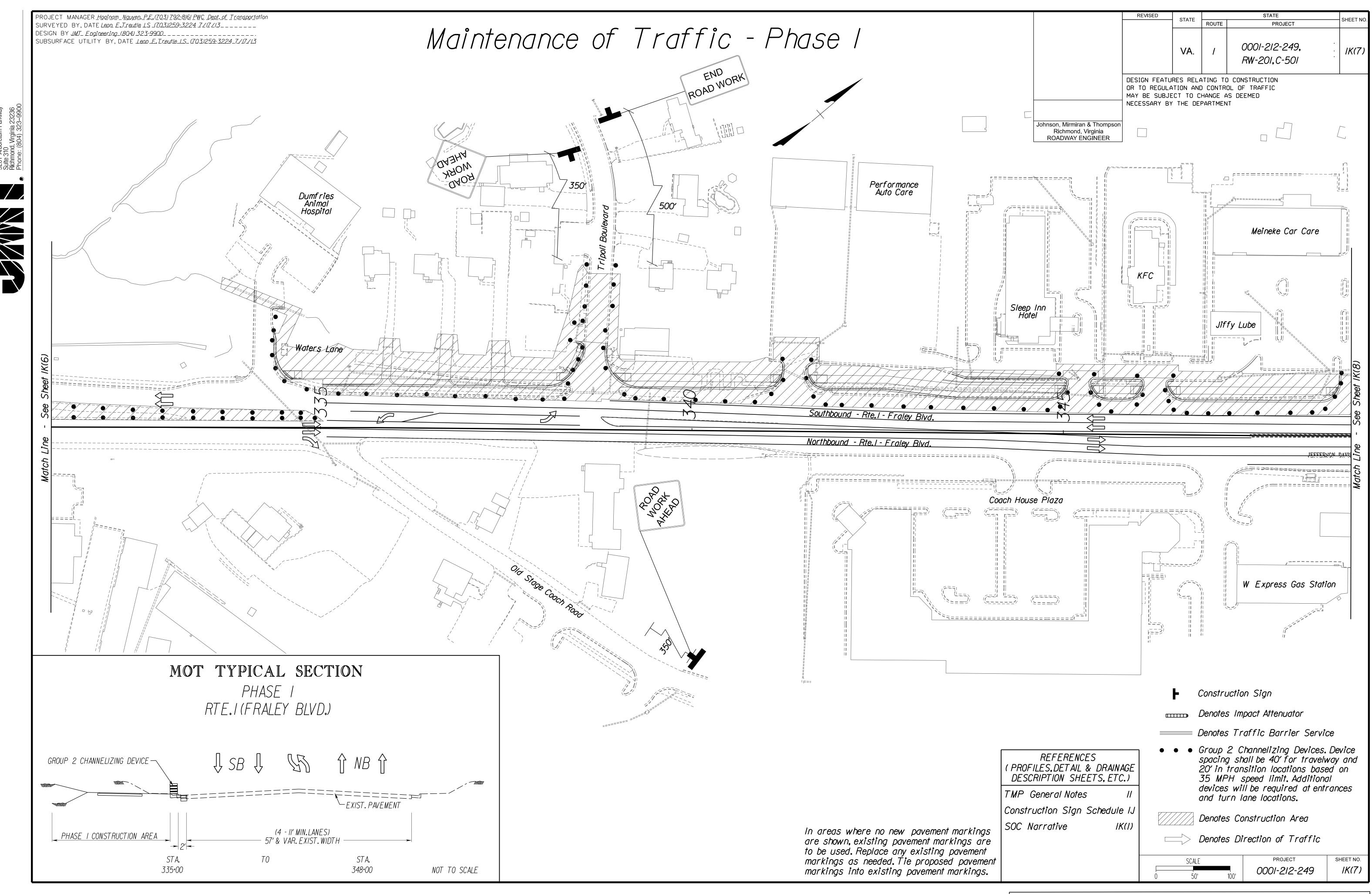


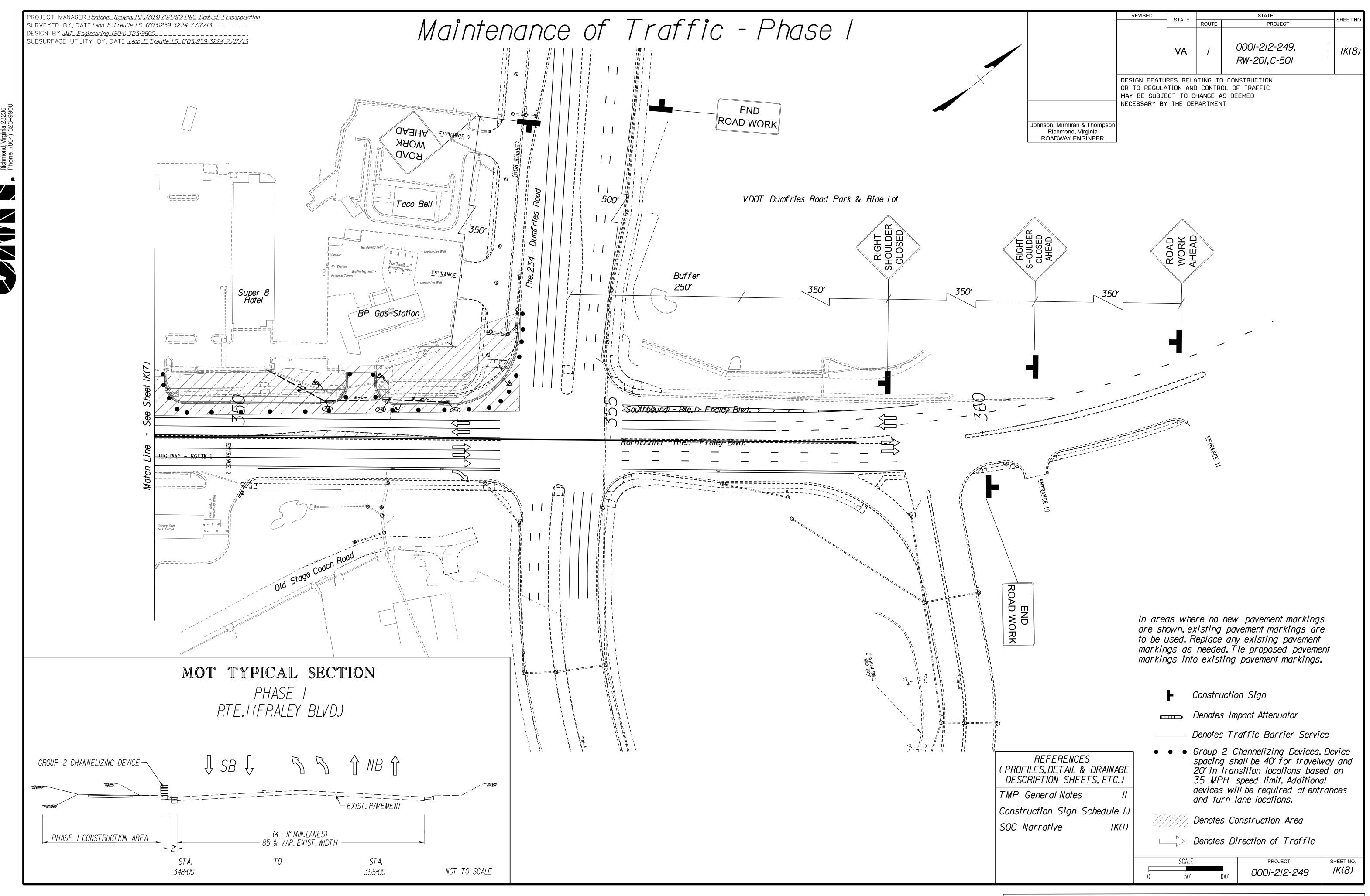


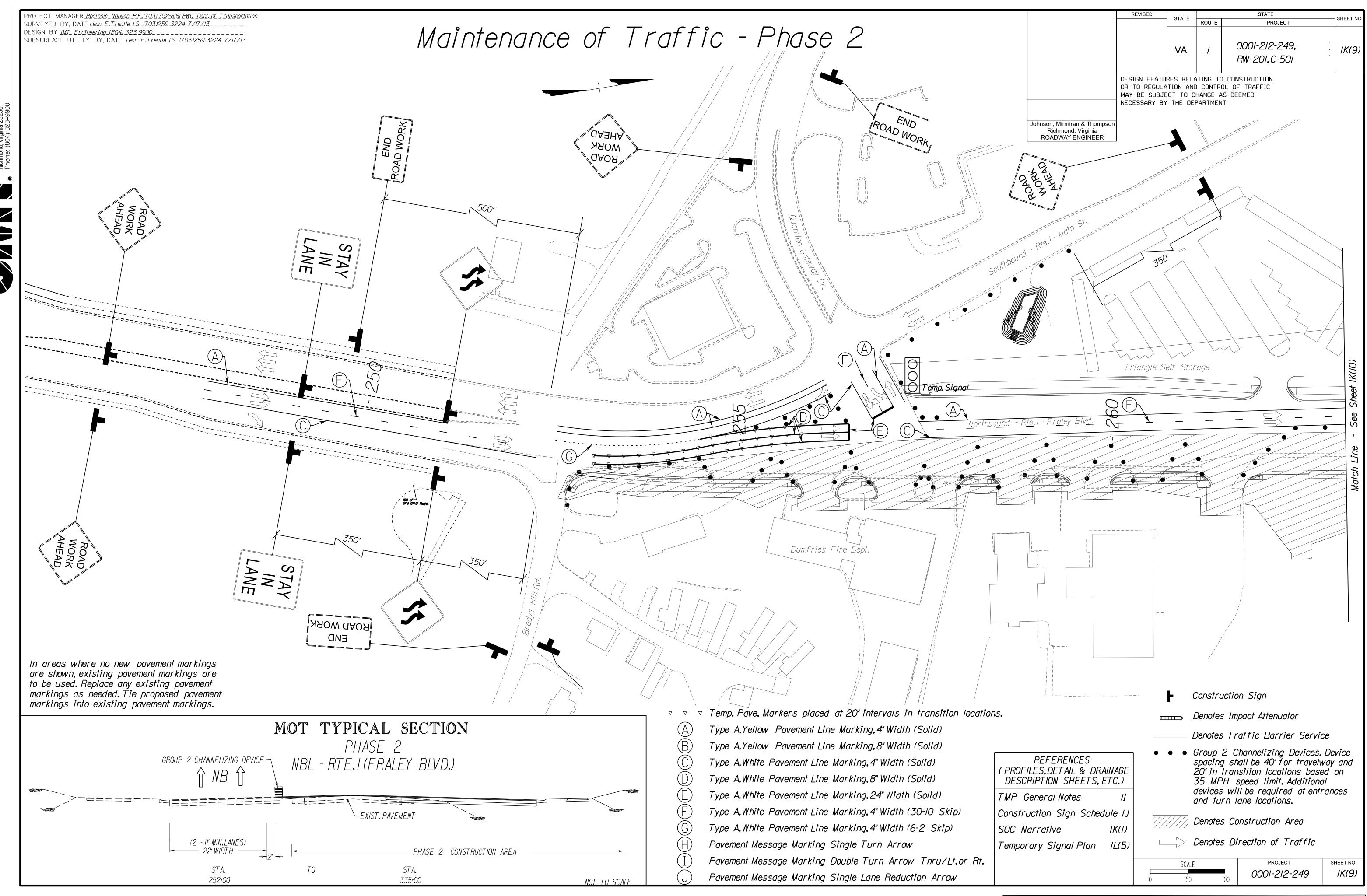


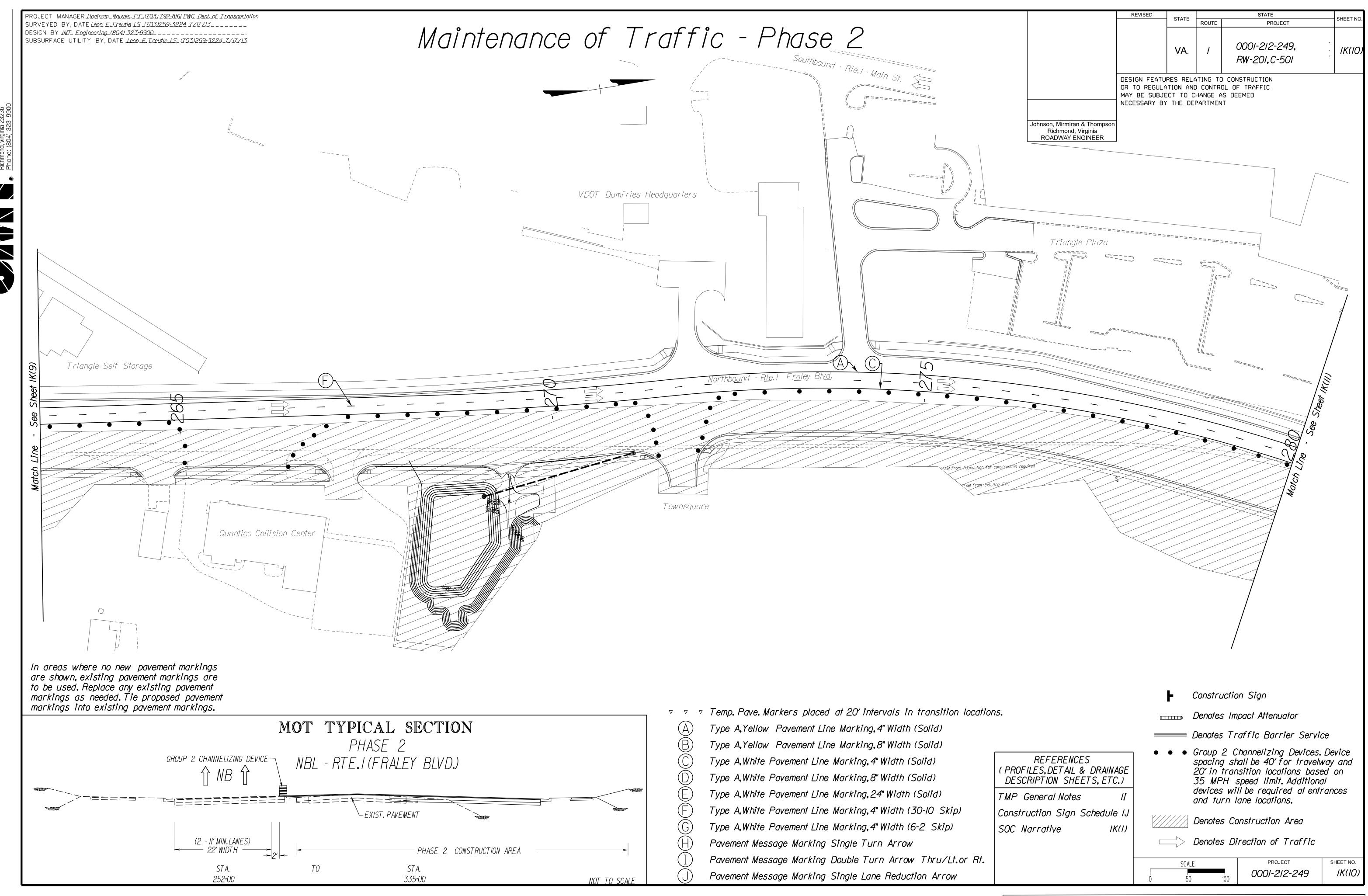


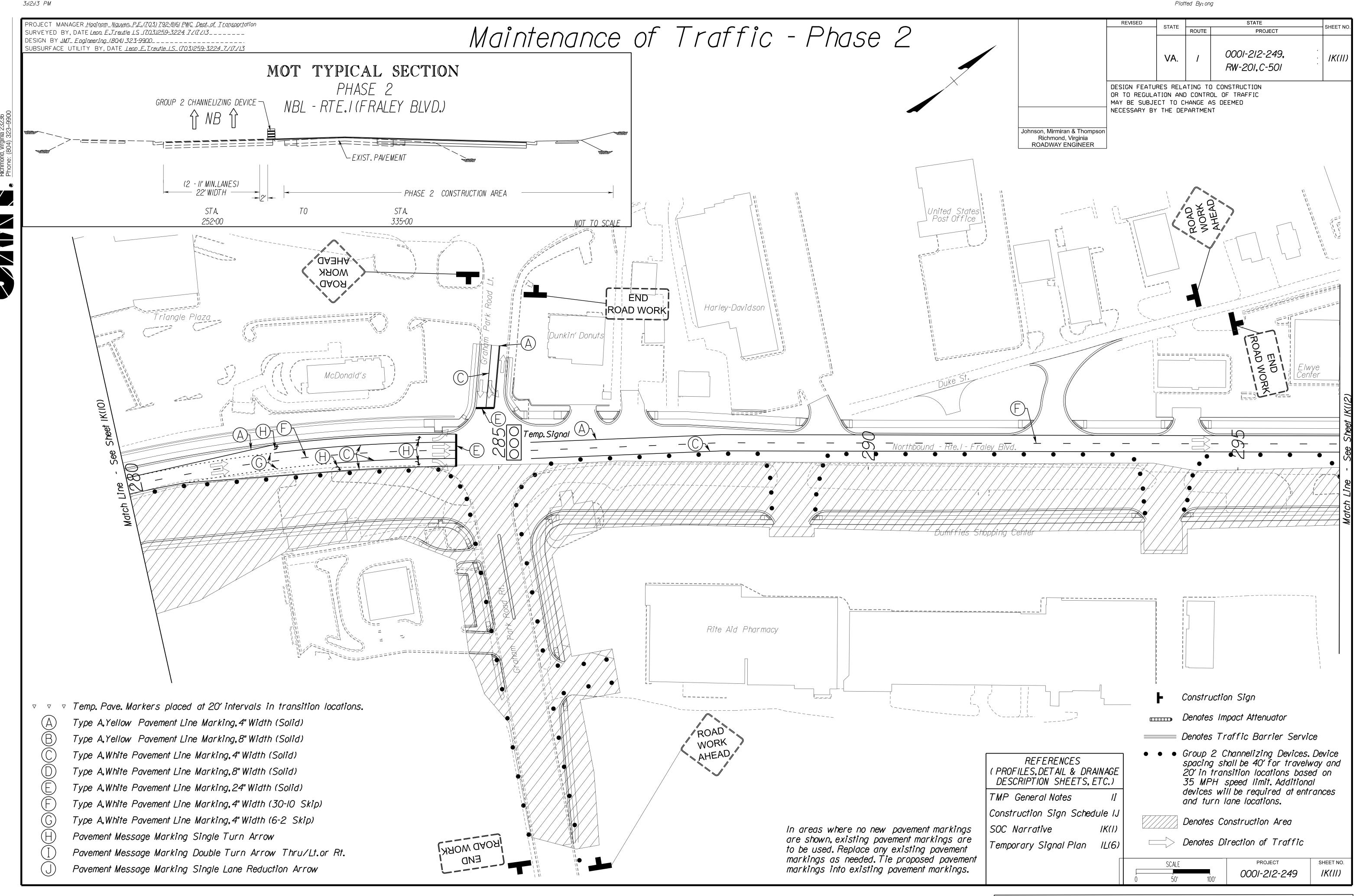


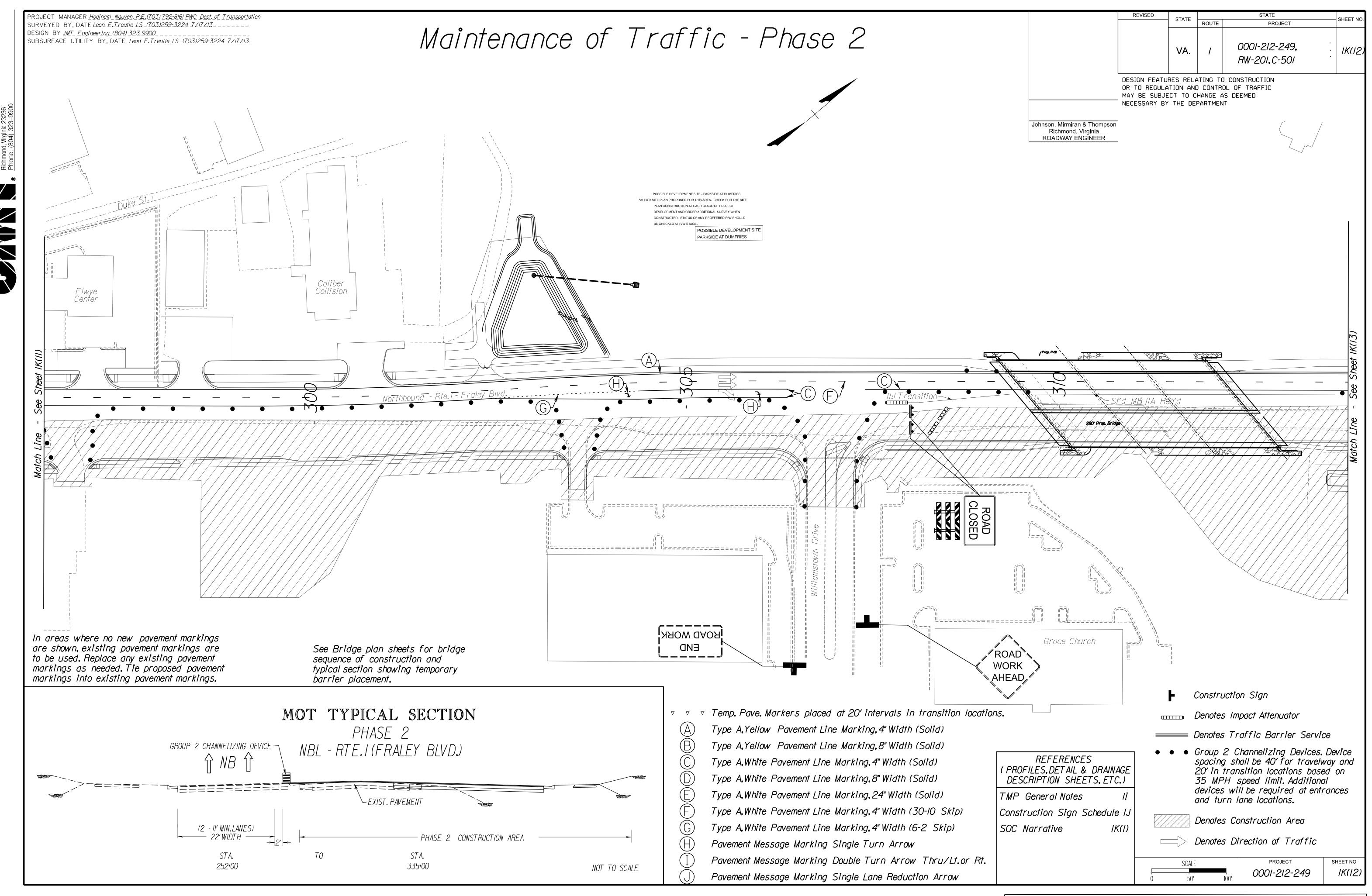


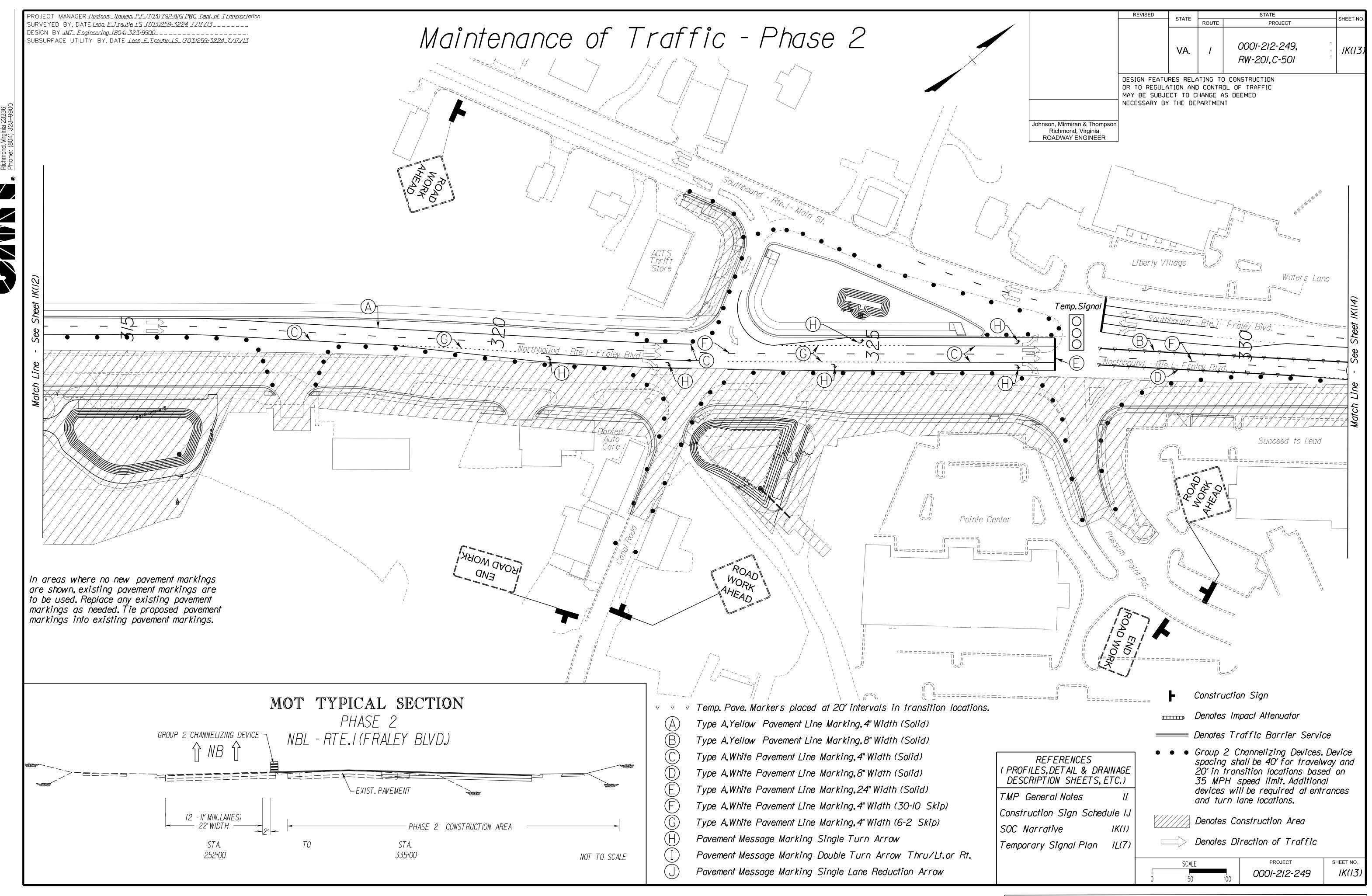


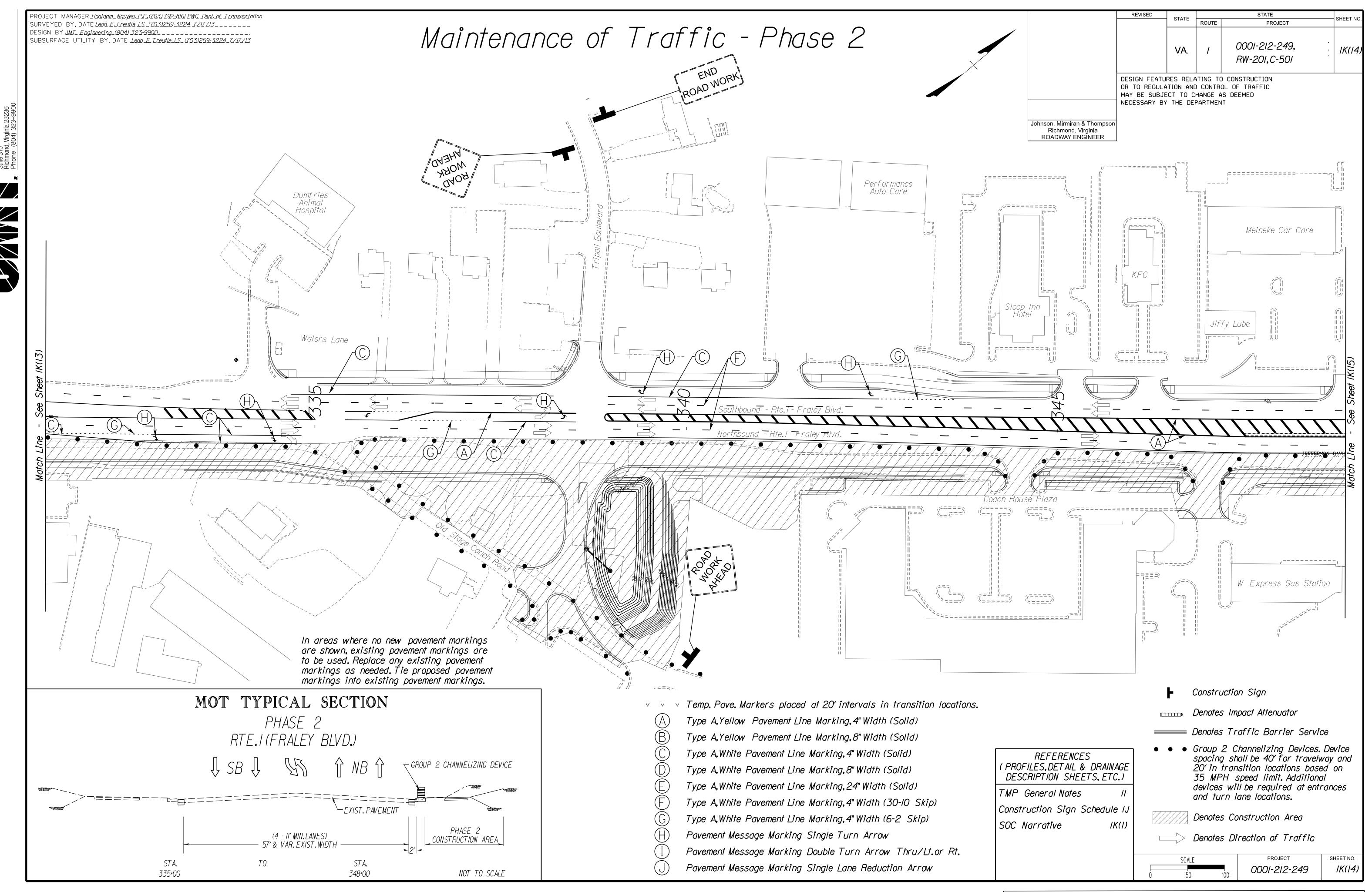


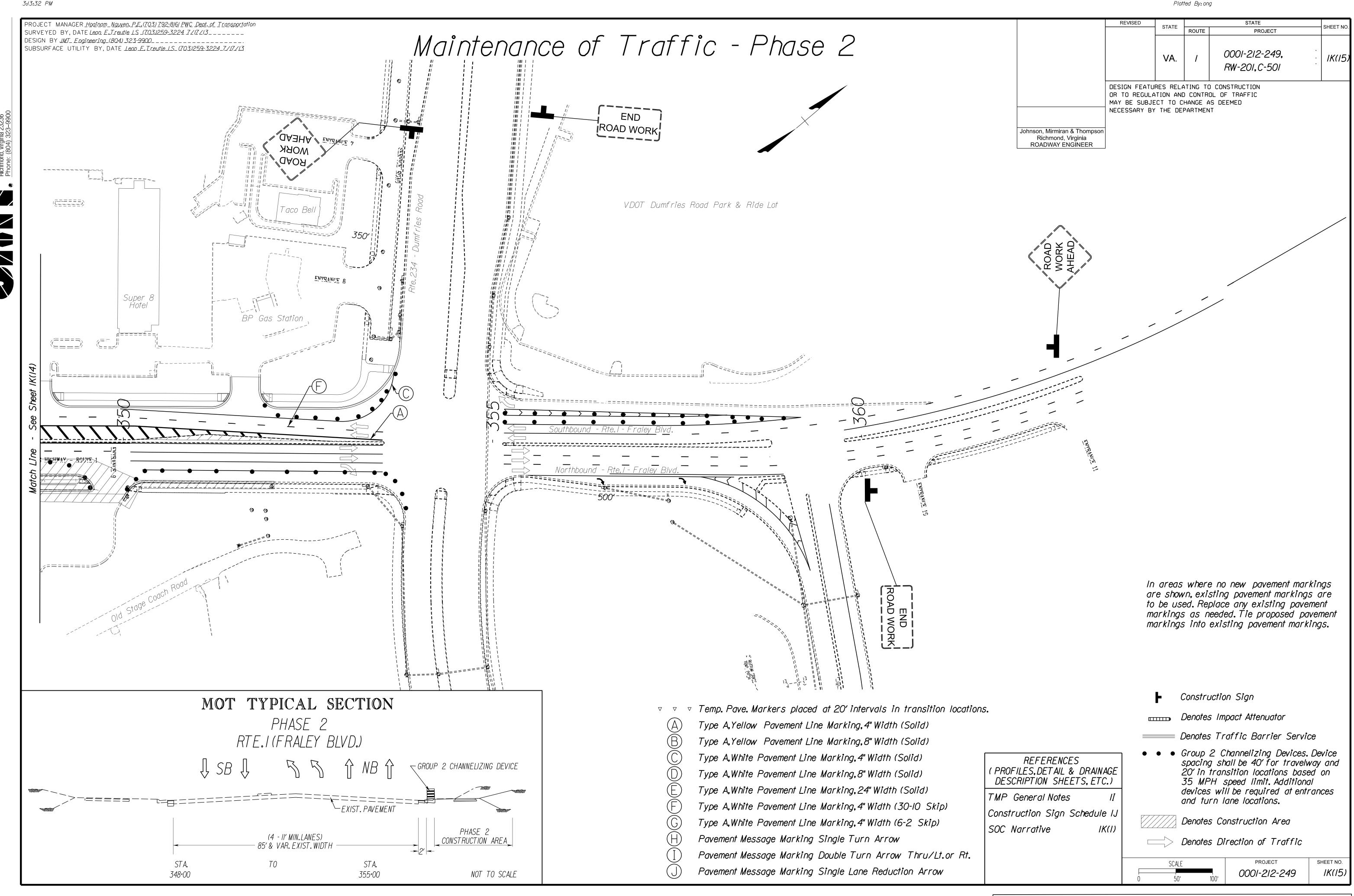


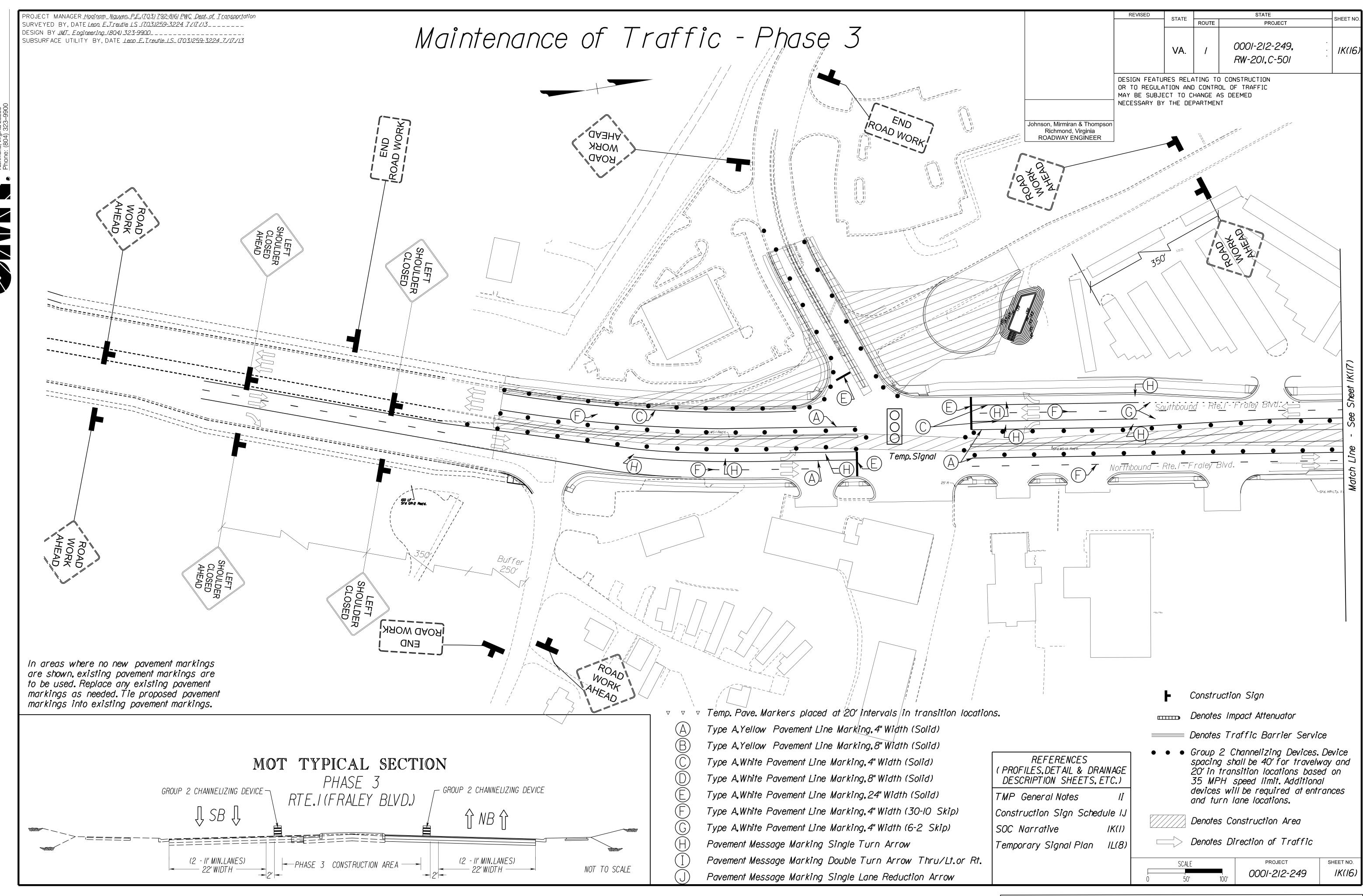


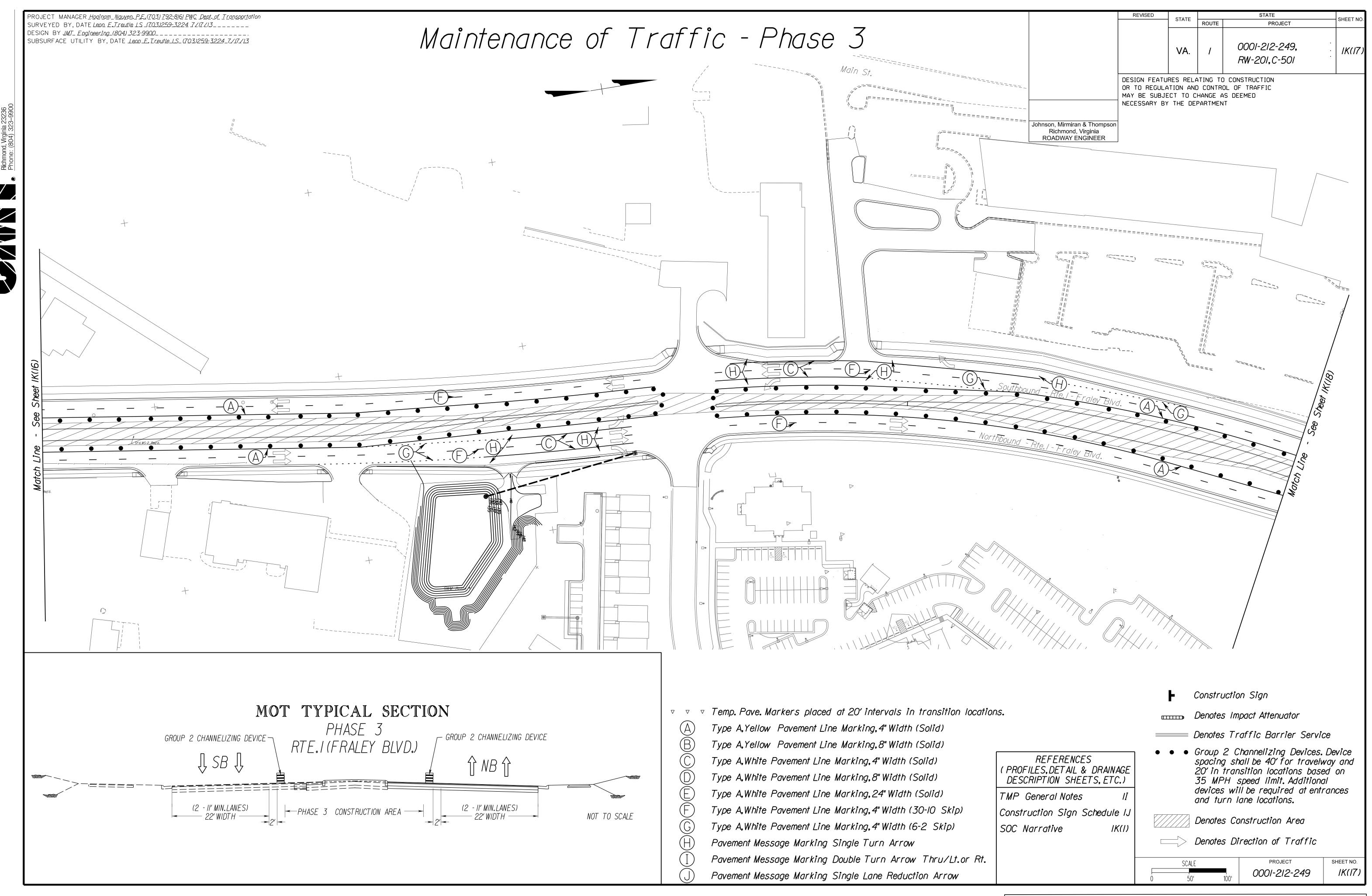


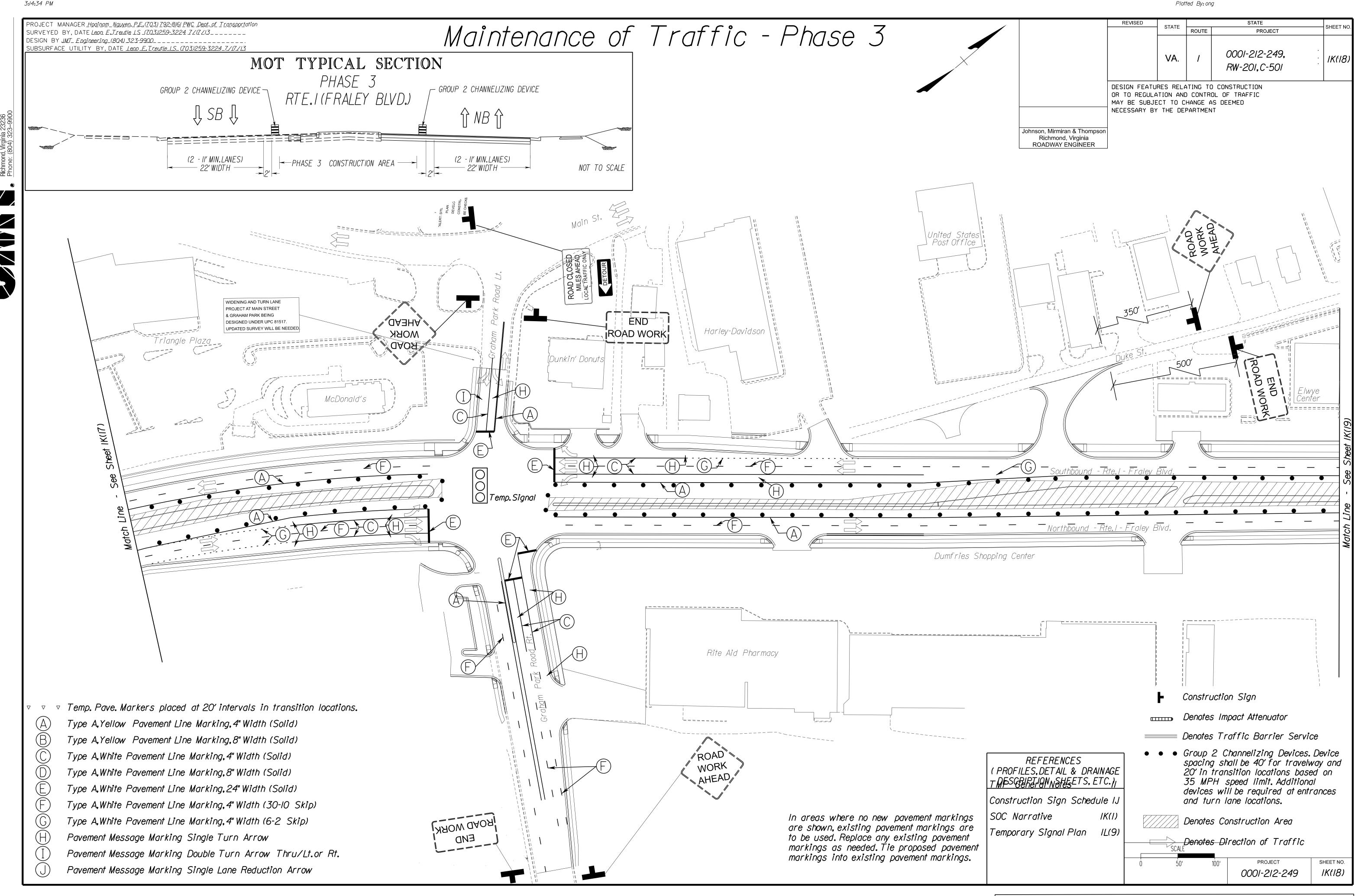


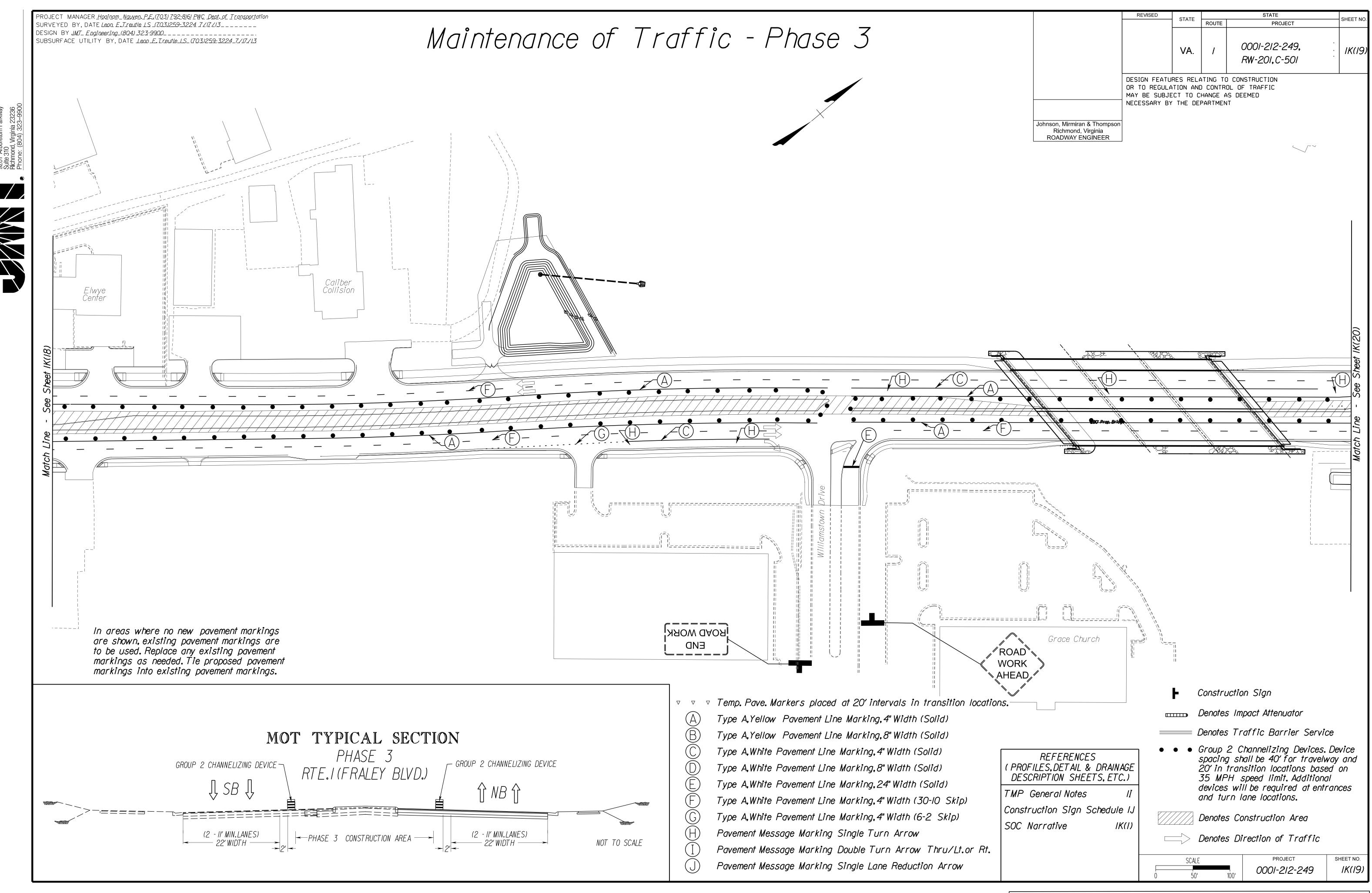


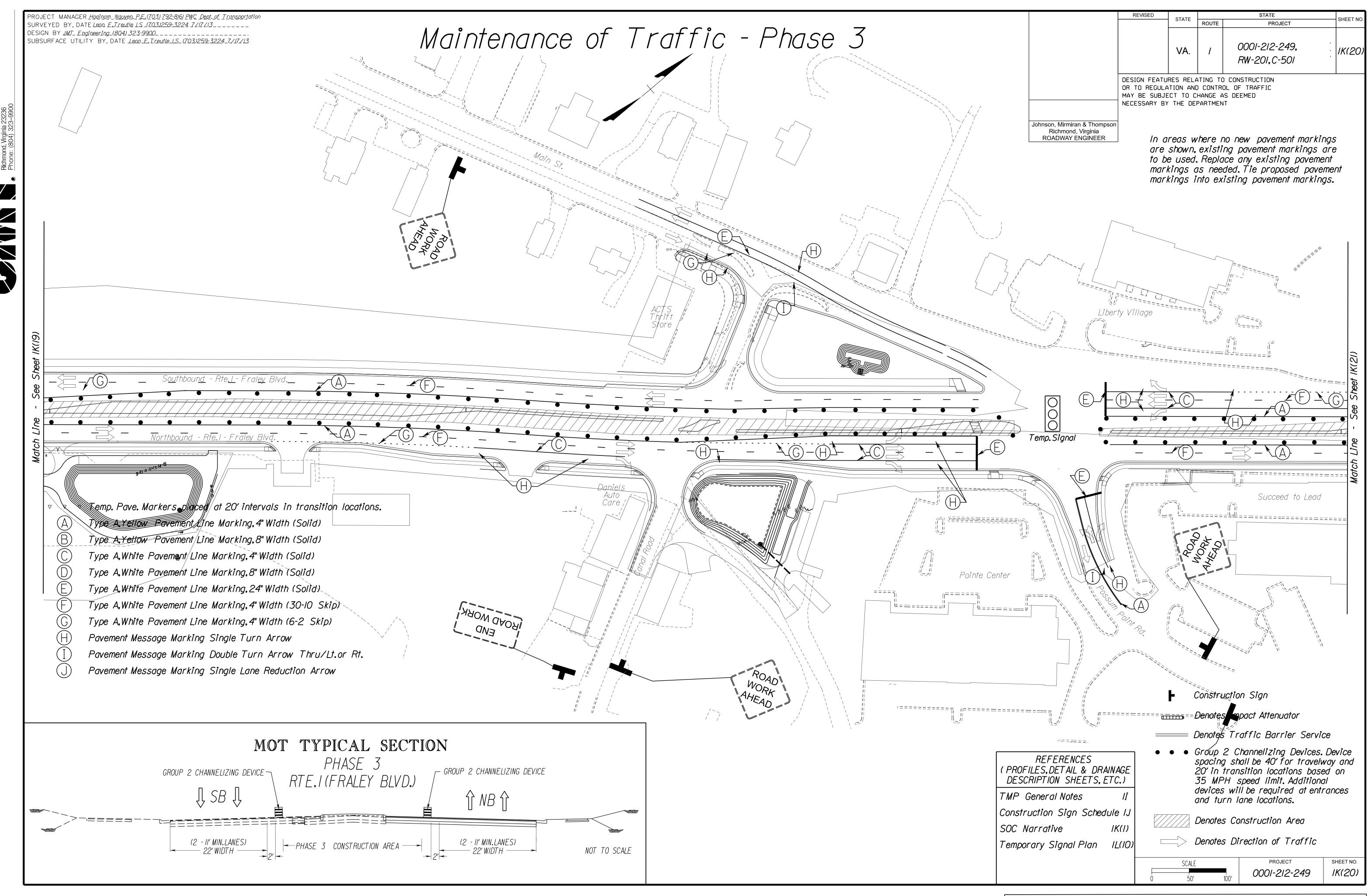


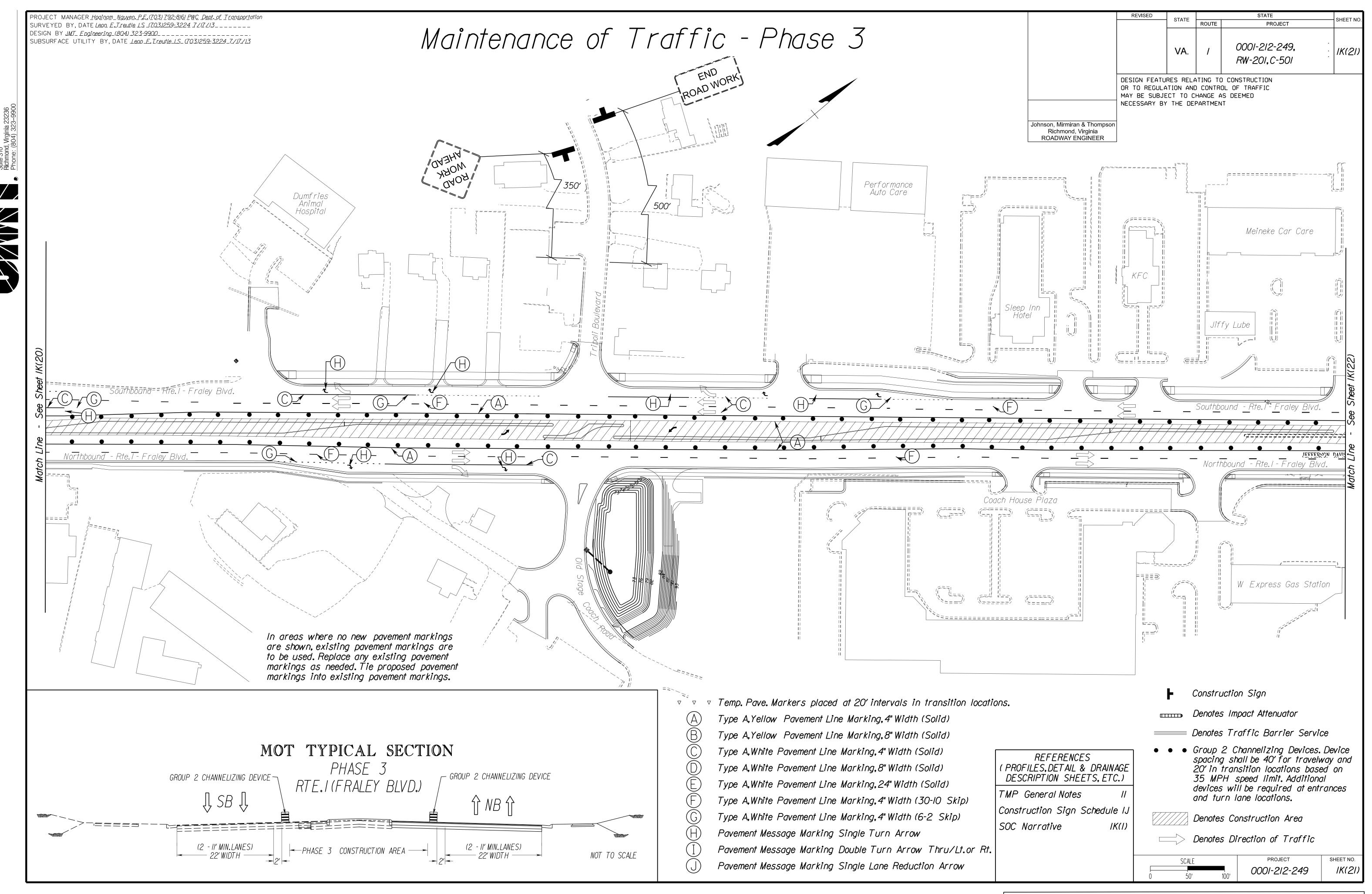


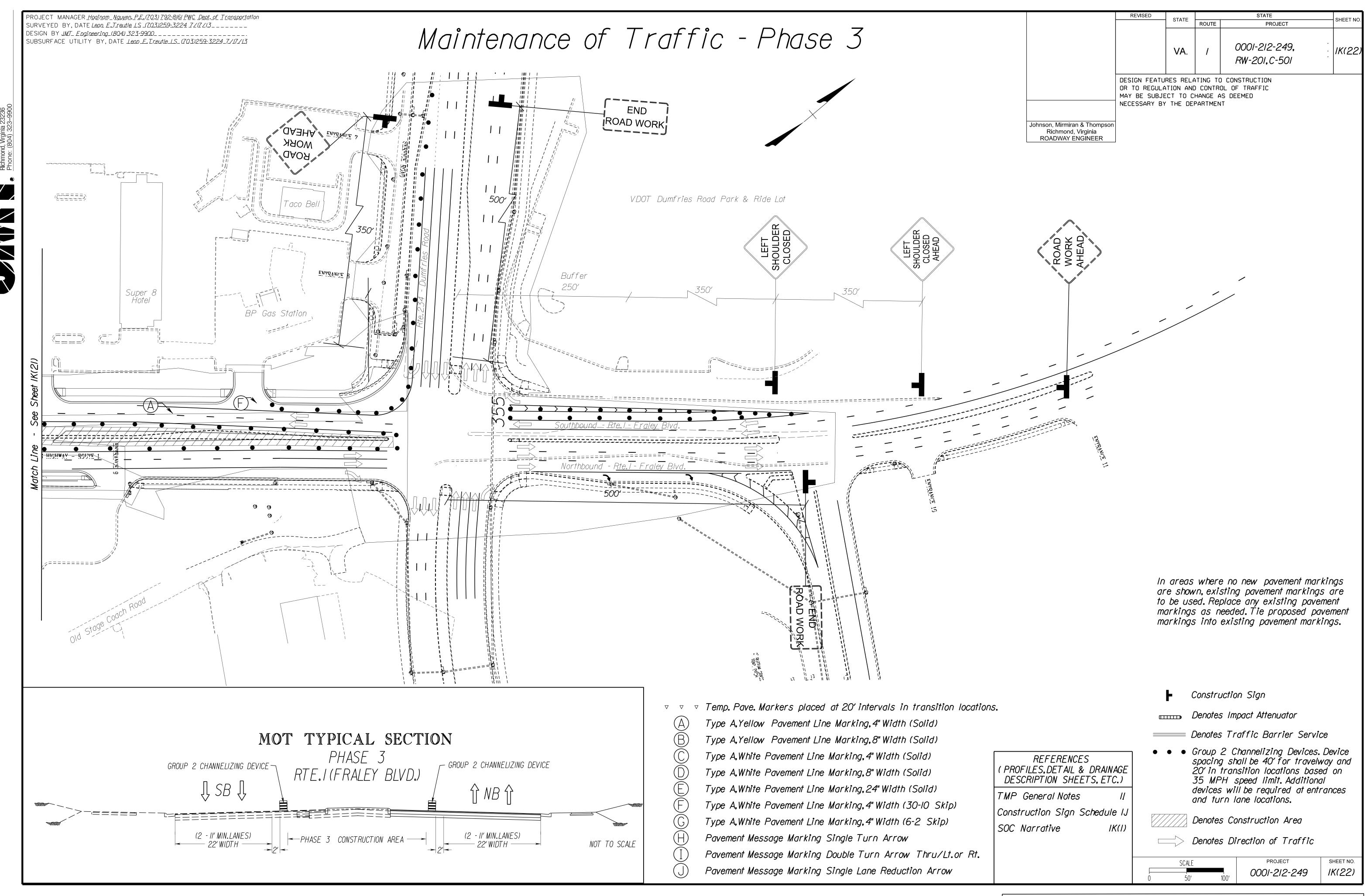












PROJECT MANAGER_____

SURVEYED BY, DATE ______ DESIGN BY ______ SUBSURFACE UTILITY BY, DATE ________

EROSION & SEDIMENT CONTROL NARRATIVE

EROSION & SEDIMENT CONTROL GENERAL NOTES

PROJECT DESCRIPTION

This project is located in Prince William County and approximately 36.30 acres will be disturbed by the proposed construction activity. The Project is in Prince William County Dumfries Virginia along US Route I. The existing roadway consists of two northbound and two south bound lanes which separate as they pass into and through the town of Dumfries. See 'Project Soil Map' in this sheetfor the Project Location. The existing roadway is drained by shoulder ditches with some curb and gutter sections. The proposed Project will combine the separated northbound and southbound lanes and the northbound lanes will carry all US Route I traffic transitioning to six lanes separated by a raised grassy median, (VDOT Standard MS-2). The proposed roadway will be drained by curb and gutter for the entire Project length. The existing US Route I southbound lanes will become a local roadway through the town of Dumfries. The Project will extend from the existing intersection with Quantico Gateway Drive north to the intersection with Dumfries Road (SR 234) for approximately 1.8 miles in length. The Project will also:

I.Construct a IO-ft shared-use path for bicyclists and pedestrians along the southbound side (east side) of Fraley BLVD and a 5-ft sidewalk along the northbound side (west side) of Fraley BLVD. 2.Conruct and widen the Fraley BLVD Bridge over Quantico Creek.

EXISTING SITE CONDITIONS

The project location is highly urbanized with lots of existing storm sewer system. There are small strips of grass and some forest areas in the project site. There are existing BMPs located near the project site aswell. The drainage throughout the project ultimately drains to Quantico Creek. The project topography has good slope for drainage.

ADJACENT PROPERTY

Adjacent to Route I on both sides are primarily commercial developed properties.

OFF-SITE AREAS

There are no anticipated Off-Site borrow areas and/or surplus material disposal areas associated with this project. Therefore offsite-borrow is not covered by this Erosion and Sediment Control Plan. In the event that the above statement is not valid the contractor shall submit a supplementary E&S plan to the owner covering the off-site borrow area which would have to be approved by the authority before any off-site activity commences.

According to the Soil Survey of Prince William County, Virginia, the soils in the project area primarily consist of Urban Land _ Udorthents complex (54B). This designation describes areas where 85 percent or more of the surface layer is covered by asphalt, concrete or other impervious surfaces and areas of variable depth and slope which are well draining to moderately well drained soils. The Udorthents are areas where the existing soils have been altered by excavation or covered by fill. Also included are undisturbed soils and fill area containing material, such as concrete, wood and asphalt.

CRITICAL EROSION AREAS

No areas have been identified as critical areas for erosion.

EROSION AND SEDIMENT CONTROL MEASURES

Unless otherwise indicated, all vegetative and structural erosion and sediment control practices shall be constructed and maintained according to minimum standards and specifications of the Virginia Erosion and Sediment Control Handbook (1992) and the VDOT Road and Bridge Specifications (2007). See sheet IM(I).IM(2) and IM(3) for a list of E&S controls used and General E&S notes.

PERMANENT STABILIZATION

Permanent stabilization shall be done in accordance with the VESCH and VDOT Road & Bridge Specifications (2016). All areas disturbed by construction shall be stabilized with permanent seeding immediately following finish grading. Seeding shall be done in accordance with these plans unless otherwise directed by the engineer.

STORMWATER MANAGEMENT

The project is grandfathered and storm water management was designed based on Part II C criteria of VSMP.Calculation of runoff before and after development indicates that there will be a net increase in peak runoff as a result of the project. Therefore Part II C design criteria has been ap plied to be in compliance with water quantity and quality measures.. See drainage report for more

STRUCTURAL PRACTICES

I. <u>Safety Fence- 3.01</u>

A safety fence shall be installed around all temporary sediment traps in accordance with the VESCH.

2. Temporary Construction Entrance - 3.02

Temporary construction entrances with wash racks shall be installed adjacent to the construction limits. During muddy conditions, drivers of construction vehicles will be required to wash their wheels before leaving the limits of construction. Any sediment tracked into the travel way shall be cleaned at the end of each day, in accordance with minimum standard *17. Water source for wash rack to be provided by fire hydrant. If no hydrant is available, water to be provided by private water truck.

3. <u>Construction Road Stabilization - 3.03</u>
Temporary stabilization with stone of access roads, subdivision streets, parking areas and other traffic areas immediately after grading to reduce erosion caused by vehicles during wet weather, and to prevent having to regrade permanent roadbeds between initial grading and final stabilization.

4. <u>Silt Fence Barrier - 3.05 Super Silt Fence Barrier</u>

Silt fence sediment barriers will be installed down slope of areas with minimal grades to filter sediment-laden runoff from the sheet flow, as indicated on the plans.

5. Storm Drain Inlet Protection - 3.07

All storm sewer inlets shall be protected during construction. Sediment-laden water shall be filtered before entering the storm sewer inlets.

6. <u>Culvert Inlet Protection - 3.08</u> All culvert inlets shall be protected during construction. Sediment-laden water shall be filtered

before entering the culvert inlets. 7. <u>Temporary Diversion Dike - 3.09</u>
Diversion Dikes shall be installed below major graded areas to direct sediment-laden runoff into

the sediment traps. Diversion Dikes shall be installed above major graded areas to divert clean water around the disturbed areas.

8. <u>Temporary Diversion Channel - 3,12</u> Temporary Diversion Channels shall be installed in the locations shown on the plan to divert the

existing channels and allow for the construction of the culverts to be performed in the dry.

9. <u>Temporary Sediment Trap - 3.13</u> A temporary ponding area will be formed by constructing an earthen embankment with a stone weir outlet. The depth and configuration of the trap will be designed to meet minimum standards, and

will be filled in Phase II when all storm sewer utilities are in place and functional. Specific

details of the sediment traps are shown on the plan. Temporary stone dams shall be constructed across the drainage ditches to reduce the velocity of

concentrated stormwater flows, thereby reducing erosion of the swale or ditch.

VEGETATIVE PRACTICES

Temporary Seeding - 3.31

Permanent or temporary soil stabilization shall be applied to denuded areas within seven days after final grade is reached on any portion of the site. Temporary soil stabilization shall be applied with in seven days to denuded areas that may not be at final grade but will remain dormant for longer than 30 days. Permanent stabilization shall be applied to areas that are to be left dormant for more than one year.

Permanent or temporary soil stabilization shall be applied on rough-graded areas that will not be brought to final grade for a year or more or where permanent, long-lived, vegetative cover, is needed on fine-graded areas. Permanent seeding shall consist of perennial vegetative cover and shall be determined by the slopes, soil types, and maintenance requirements.

MANAGEMENT STRATEGIES

The first step in this Erosion and Sediment Control Plan for this multi-phase project is to install all perimeter controls. All perimeter controls will be in place prior to any excavation.

Phase I of the Erosion and Sediment Control Plan shall:

I. Flag limits of clearing and grading and hold pre-construction meeting. 2. Install construction entrances with wash racks in the location shown on the plans. Water for the wash racks to be provided by private water truck if no hydrant is available. 3. Provide minimum grading to allow Phase I measures to be installed. 4. Install perimeter controls as shown to include diversion dikes and silt fence. These sediment trapping measures shall be installed as a first step in grading per the Phase I Erosion and

Sediment Control Plan and will be seeded and mulched immediately following installation. 5. Gradina operations may commence once perimeter controls diversions and trappina measures are installed to the satisfaction of the inspector. 6. Temporary seeding or other stabilization will follow immediately after grading.

7. Once all of Phase I controls are in place, the Contractor is to contact the county inspector for sign-off. Once sign-off is obtained by the county, the Contractor can proceed with general clearing and earthworks activities. 8. Install proposed utilities.

9. Fine arade excavated areas. 10. Lime, fertilize and permanently seed and mulch all areas that will not receive impervious

II. For vegetative stabilization of all denuded areas see erosion control measures and vegetative

practices. 12. Once all areas are stabilized to the satisfaction of the county inspector the control shall remove perimeter controls.

MAINTENANCE STRATEGIES - SEDIMENT & EROSION CONTROL I. It will be the responsibility of the Contractor to ensure that all downstream areas are protected against erosion and sedimentation. In doing so, the Contractor must coordinate with the

county inspector throughout the duration of this project. 2. In general, all erosion and sediment control measures will be checked daily and after each significant rainfall. Refer to the attached erosion and sediment control standard notes for detailed maintenance and revegetation/stabilization requirements.

3. All new seeded and mulch areas will be inspected after each rainfall event to ensure the new seed has not been washed away. If so, the areas shall be re-seeded and mulched immediately. 4. The inspector has the authority to add or delete erosion and sediment controls as needed in the field, as site conditions warrant. The Contractor does have the authority to add additional sediment and erosion control measures as the Contractor deems necessary to prevent erosion and movement of sediment to off-site areas. Additional measures should be authorized by the project

5. All temporary erosion and sediment control measures shall be removed within 30 days after final site stabilization, in accordance with minimum standard *18.

TWO PHASE EROSION & SEDIMENT CONTROL PLAN

Phase I controls shall be placed as indicated on the Erosion & Sediment Plans, prior to any land disturbing activities. Mud and debris will be washed from all construction vehicles and equipment before leaving the site. See land disturbing/construction sequence, this sheet.

Phase II work will not commence until Phase I work has been approved by the county inspector. Phase II includes $\frac{1P(1)-1P(23)}{1P(23)}$ the adjustment of silt fence and perimeter controls, providing the cut and fill areas are near final grade and storm sewer is functional. The utilities, curb and gutter, and roads also should be near final grade. Base stone for the roads and parking area should be completed within seven (7) days after reaching final grade for subgrade. Inlet protection shall be provided for all proposed and existing inlet storm structures. Additionally, any stock piles (location of which will be coordinated in the field with the site inspector) will be provided with perimeter silt fence. Topsoil, stockpiles and all areas to be rough graded during initial phase of construction shall be seeded with fast germinating temporary vegetation immediately following grading. Mixture of seed will depend on the time of year. 3:1 slope areas not adequately stabilized by seeding are to be sodded and pegged at the direction of the inspector. After all construction operations have ended and all disturbed areas have been stabilized, mechanical sediment controls shall be removed and the ground permanently stabilized with vegetation upon the approval of the site inspector. See land disturbing/construction sequence, this sheet.

The implementation of Phase II controls cannot begin until the Phase II controls have been approved by the Prince William County Inspector.

LAND DISTURBING/CONSTRUCTION SEQUENCE PHASE I

A Pre-Construction meeting shall be held prior to commencement of work.

Prior to clearing and grubbing all perimeter controls are to be installed as shown and as necessary. Construct temporary sediment traps at proposed locations. The contractor shall install and maintain all necessary temporary pipes to provide adequate drainage throughout construction. Construct proposed drainage outfalls and channel relocations or improvements as shown on the plans. For all ditches constructed during Phase I, the required check dams shall be installed at the time ditches are constructed. Obtain County Site Inspector's approval of perimeter controls.

LAND DISTURBING/CONSTRUCTION SEQUENCE PHASE II

After the County Site Inspector's approval of Phase IE&S controls, clear and grub remainder of the site as necessary. Construct the proposed drainage system as shown and as necessary, install inlet protection as shown and as needed. All silt fence is to be installed as shown and as necessary, drop inlet silt traps shall be installed as shown and as needed, rock check dams shown shall be installed at the same time the ditch is constructed. All ditches shall be constructed and stabilized according to the plans, once stabilization has been completed direct flow to the ditches and remove temporary diversion dikes. Install all curb & gutter and place base stone pavement except where this would interfere with the temporary sediment traps. Fine grade site and install all landscaping, including permanent seeding and fertilizing as shown in the plan. Install base course asphalt paving and finial paving. Clean site of all trash and debris. Have the County Inspector inspect all areas to determine if they are adequately stabilized.

CHECKLIST

FOR EROSION AND SEDIMENT CONTROL PLANS

1M(2) <u>Minimum Standards</u> - All applicable Minimum Standards must be addressed.

Narrative

<u>1M(1)</u> <u>Project description</u> - briefly describes the nature and purpose of the land-disturbing activity, and the area (acres) to be disturbed.

Existing site conditions - a description of the existing topography, vegetation and drainage.

Adjacent areas - A description of neighboring areas such as streams, lakes, residential areas, road, etc., which might be affected by the land disturbance.

Off-site areas - Describe any off-site land-disturbing activities that will occur(including borrow sites, waste or surplus areas, etc.). Will any other areas be disturbed?

unit, erodibility, permeability, depth, texture and soil structure. <u>Critical areas</u> - A description of areas on the site which have potentially serious erosion problems

(e.g., steep slopes, channels, wet weather/underground springs, etc.). <u>Erosion and sediment control measures</u> - A description of the methods which will be used to control erosion and sedimentation on the site. (Controls should meet the specifications in Chapter 3.)

<u>Soils</u> - a brief description to the soils on the site giving such information as soil name, mapping

Permanent stabilization - A brief description, including specifications, of how the site will be stabilized after construction is completed.

<u>Stormwater runoff considerations</u> - Will the development site cause an increase in peak runoff rates? Will the increase in run off cause flooding or channel degradation down stream? Describe the strategy to control stormwater runoff.

SITE PLAN

1A <u>Vicinity map</u> - A small map locating the site in relation to the surrounding area. Include any landmarks which might assist in locating the site.

<u>Indicate north</u> - The direction of north in relation to the site.

1N<u>(1)-1N(2</u>3) <u>Limits of clearing and grading</u> - Areas which are to be cleared and graded.

1N(1)-1N(23) Existing contours - the existing contours of the site.

1P(3)-1P(14) Final contours - Changes to the existing contours, including final drainage patterns.

1N(3)-1N(23) Existing vegetation - The existing tree lines, grassed areas, or unique vegetation.

1M(1) Soils - The boundaries of different soil types.

1N<u>(1)-1N(2</u>3) <u>Existing drainage patterns</u> - The dividing lines and the direction of flow for the different drainage areas. Include the size (acreage) of each drainage area.

<u>Critical erosion areas</u> - Areas with potentially serous erosion problems. (See Chapter 6 for

Site Development - Show all improvements such as buildings, parking lots, access roads, utility

<u>Location of practices</u> - The locations of erosions and sediment controls and stormwater management practices used on the site. Use the standard symbols and abbreviations in Chapter 3 of the

Off-site areas - Identify any off-site land-disturbing activities (e.g., borrow sites, waste areas, etc.). Show locations of erosion controls. (Is there sufficient information to assure adequate protection and stabilization?)

<u>Detail drawings</u> - Any structural practices used that are not referenced to the E&S hand book or local handbooks should be explained and illustrated with detail drawings.

Maintenance - A schedule of regular inspections and repair of erosion and sediment control structures should be set forth.

TABLE 3.31B ACCEPTABLE TEMPORARY SEEDING PLANT MATERIALS "QUICK REFERENCE FOR ALL REGIONS" Rate <u>Planting Dates</u> <u>Species</u> lbs./acre Sept. 1 - Feb. 15 50/50 Mix of Annual Ryegrass (Loium multi-florum) 50 - 100 & Cereal 9 Winter Rye (Secale cereale) 60 - 100 Feb. 16 - Apr. 30 Annual Ryegrass (Lolium multi-florum) 50 German Millet May 1 - Aug. 31 (Setaria italica) Source: Va. DSWC

REVISED ROUTE PROJECT 0001-212-249, IM(I) VA. RW-201, C-501

DESIGN FEATURES RELATING TO CONSTRUCTION OR TO REGULATION AND CONTROL OF TRAFFIC MAY BE SUBJECT TO CHANGE AS DEEMED NECESSARY BY THE DEPARTMENT

Johnson, Mirmiran & Thompson Richmond, Virginia HYDRAULIC ENGINEER

> Project Soils Map Prince William County, Virginia

> > End Project



18C Dumfries sandy loam,7 to 15 percent slopes 18D Dumfries sandy loam, 15 to 25 percent slopes

27A Hatboro-Codorus complex, 0 to 2 percent slopes Lunt loam,7 to 15 percent slopes

Urban land-Udorthents complex, 0 to 7 percent slopes

TABLE 3.32-D SITE SPECIFIC SEEDING MIXTURES FOR COASTAL PLAIN AREA Total Lbs. Per Acre Minimum Care Lawn - Commercial or Residential - Kentucky 31 or Turf-Type Tall Fescue 175-200 lbs. Common Bermudagrass ** 75 lbs. High-Maintenance Lawn Kentucky 31 or Turf-Type Tall Fescue 200-250 lbs. Hybrid Bermudagrass (seed) ** 40 lbs. (unhulled) 30 lbs. (hulled) Hybrid Bermudagrass (by other vegetative establishment method, see Std. & Spec. 3.34) General Slope (3:1 or less) - Kentucky 31 Fescue 128 lbs. - Red Top Grass 2 lbs. Seasonal Nurse Crop * 20 lbs. 150 lbs. Low Maintenance Slope (Steeper than 3:1) - Kentucky 31 Tall Fescue 93-108 lbs. Common Bermudagrass ** 0-15 lbs. Red Top Grass 2 lbs. Seasonal Nurse Crop * 20 lbs. Sericea Lespedeza * 20 lbs. * Use seasonal nurse crop in accordance with seeding dates as stated below: February, March through April Annual Rye May 1st through August Foxtail Millet September, October through November 15th Annual Rye November 16th through January Winter Rye ** May through October, use hulled seed. All other seeding periods, use unhulled seed. Weeping Lovegrass may be added to any slope or lowmaintenance mix during warmer seeding periods; add 10-20 lbs./acre in mixes.

SHEET NO. **PROJECT** Source: Va. DSWC 0001-212-249

PLANS

PROJECT MANAGER_____ SURVEYED BY, DATE ______ DESIGN BY _____ SUBSURFACE UTILITY BY, DATE ______

EROSION & SEDIMENT CONTROL GENERAL NOTES

4VAC50-30-40 MINIMUM STANDARDS. (MS-19)

AN EROSION AND SEDIMENT CONTROL PROGRAM ADOPTED BY A DISTRICT OR LOCALITY MUST BE CONSISTENT WITH THE FOLLOWING CRITERIA, TECHNIQUES AND METHODS:

- I. PERMANENT OR TEMPORARY SOIL STABILIZATION SHALL BE APPLIED TO DENUDED AREAS WITHIN SEVEN DAYS AFTER FINAL GRADE IS REACHED ON ANY PORTION OF THE SITE. TEMPORARY SOIL STABILIZATION SHALL BE APPLIED WITHIN SEVEN DAYS TO DENUDED AREAS THAT MAY NOT BE AT FINAL GRADE BUT WILL REMAIN DORMANT FOR LONGER THAN 30 DAYS. PERMANENT STABILIZATION SHALL BE APPLIED TO AREAS THAT ARE TO BE LEFT DORMANT FOR MORE THAN ONE YEAR.
- 2. DURING CONSTRUCTION OF THE PROJECT. SOIL STOCKPILES AND BORROW AREAS SHALL BE STABILIZED OR PROTECTED WITH SEDIMENT TRAPPING MEASURES. THE APPLICANT IS RESPONSIBLE FOR THE TEMPORARY PROTECTION AND PERMANENT STABILIZATION OF ALL SOILS STOCKPILES ON SITE AS WELL AS BORROW AREAS AND SOIL INTENTIONALLY TRANSPORTED FROM THE PROJECT SITE.
- 3. A PERMANENT VEGETATIVE COVER SHALL BE ESTABLISHED ON DENUDED AREAS NOT OTHERWISE PERMANENTLY STABILIZED. PERMANENT VEGETATION SHALL NOT BE CONSIDERED ESTABLISHED UNTIL A GROUND COVER IS ACHIEVED THAT, IS UNIFORM, MATURE ENOUGH TO SURVIVE AND WILL INHIBIT EROSION.
- 4. SEDIMENT BASINS AND TRAPS, PERIMETER DIKES, SEDIMENT BARRIERS AND OTHER MEASURES INTENDED TO TRAP SEDIMENT SHALL BE CONSTRUCTED AS A FIRST STEP IN ANY LAND-DISTURBING ACTIVITY AND SHALL BE MADE FUNCTIONAL BEFORE UP SLOPE LAND DISTURBANCE TAKES PLACE.
- 5. STABILIZATION MEASURES SHALL BE APPLIED TO EARTHEN STRUCTURES SUCH AS DAMS,DIKES AND DIVERSIONS IMMEDIATELY AFTER INSTALLATION.
- 6. SEDIMENT TRAPS AND SEDIMENT BASINS SHALL BE DESIGNED AND CONSTRUCTED BASED UPON THE TOTAL DRAINAGE AREA TO BE SERVED BY THE TRAP OR BASIN.
- a. THE MINIMUM STORAGE CAPACITY OF A SEDIMENT TRAP SHALL BE 134 CUBIC YARDS PER ACRE OF DRAINAGE AND THE TRAP SHALL ONLY CONTROL DRAINAGE AREAS LESS THAN THREE ACRES.
- b. SURFACE RUNOFF FROM DISTURBED AREAS THAT IS COMPRISED OF FLOW FROM DRAINAGE AREAS GREATER THAN OR EQUAL TO THREE ACRES SHALL BE CONTROLLED BY A SEDIMENT BASIN. THE MINIMUM STORAGE CAPACITY OF A SEDIMENT BASIN SHALL BE 134 CUBIC YARDS PER ACRE OF DRAINAGE AREATHE OUTFALL SYSTEM SHALL,AT A MINIMUM,MAINTAIN THE STRUCTURAL INTEGRITY OF THE BASIN DURING A TWENTY-FIVE YEAR STORM OF 24-HOUR DURATION.RUNOFF COEFFICIENTS USED IN RUNOFF CALCULATIONS SHALL CORRESPOND TO A BARE EARTH CONDITION OR THOSE CONDITIONS EXPECTED TO EXIST WHILE THE SEDIMENT BASIN IS UTILIZED.
- 7. CUT AND FILL SLOPES SHALL BE DESIGNED AND CONSTRUCTED IN A MANNER THAT WILL MINIMIZE EROSION. SLOPES THAT ARE FOUND TO BE ERODING EXCESSIVELY WITHIN ONE YEAR OF PERMANENT STABILIZATION SHALL BE PROVIDED WITH ADDITIONAL SLOPE STABILIZING MEASURES UNTIL THE PROBLEM IS CORRECTED.
- 8. CONCENTRATED RUNOFF SHALL NOT FLOW DOWN CUT OR FILL SLOPES UNLESS CONTAINED WITHIN AN ADEQUATE TEMPORARY OR PERMANENT CHANNEL, FLUME OR SLOPE DRAIN STRUCTURE.
- 9. WHENEVER WATER SEEPS FROM A SLOPE FACE, ADEQUATE DRAINAGE OR OTHER PROTECTION SHALL BE PROVIDED.
- IO. ALL STORM SEWER INLETS THAT ARE MADE OPERABLE DURING CONSTRUCTION SHALL BE PROTECTED SO THAT SEDIMENT-LADEN WATER CANNOT ENTER THE CONVEYANCE SYSTEM WITHOUT FIRST BEING FILTERED OR OTHERWISE TREATED TO REMOVE SEDIMENT.
- II. BEFORE NEWLY CONSTRUCTED STORMWATER CONVEYANCE CHANNELS OR PIPES ARE MADE OPERATIONAL, ADEQUATE OUTLET PROTECTION AND ANY REQUIRED TEMPORARY OR PERMANENT CHANNEL LINING SHALL BE INSTALLED IN BOTH THE CONVEYANCE CHANNEL AND RECEIVING CHANNEL.
- I2. WHEN WORK IN A LIVE WATERCOURSE IS PERFORMED, PRECAUTIONS SHALL BE TAKEN TO MINIMIZE ENCROACHMENT, CONTROL SEDIMENT TRANSPORT AND STABILIZE THE WORK AREA TO THE GREATEST EXTENT POSSIBLE DURING CONSTRUCTION. NON ERODIBLE MATERIAL SHALL BE USED FOR THE CONSTRUCTION OF CAUSEWAYS AND COFFERDAMS, EARTHEN FILL MAY BE USED FOR THESE STRUCTURES IF ARMORED BY NON ERODIBLE COVER MATERIALS.
- 13. WHEN A LIVE WATERCOURSE MUST BE CROSSED BY CONSTRUCTION VEHICLES MORE THAN TWICE IN ANY SIX-MONTH PERIOD, A TEMPORARY VEHICULAR STREAM CROSSING CONSTRUCTED OF NON ERODIBLE MATERIAL SHALL BE PROVIDED.
- 14. ALL APPLICABLE FEDERAL STATE AND LOCAL REGULATIONS PERTAINING TO WORKING IN OR CROSSING LIVE WATERCOURSES SHALL BE MET.
- *IS. THE BED AND BANKS OF A WATERCOURSE SHALL BE STABILIZED IMMEDIATELY AFTER WORK IN THE WATERCOURSE IS COMPLETED.*
- I6. UNDERGROUND UTILITY LINES SHALL BE INSTALLED IN ACCORDANCE WITH THE FOLLOWING STANDARDS IN ADDITION TO OTHER APPLICABLE CRITERIA:
- a. NO MORE THAN 500 LINEAR FEET OF TRENCH MAY BE OPENED AT ONE TIME. b. EXCAVATED MATERIAL SHALL BE PLACED ON UPHILL SIDE OF TRENCHES.
- c. EFFLUENT FROM DEWATERING OPERATIONS SHALL BE FILTERED OR PASSED THROUGH AN APPROVED SEDIMENT TRAPPING DEVICE,OR BOTH, AND DISCHARGED IN A MANNER THAT DOES NOT ADVERSELY AFFECT FLOWING STREAMS OR OFF-SITE PROPERTY.
- d. MATERIAL USED FOR BACK FILLING TRENCHES SHALL BE PROPERLY COMPACTED IN ORDER TO MINIMIZE EROSION AND PROMOTE STABILIZATION. e. RESTABILIZATION SHALL BE ACCOMPLISHED IN ACCORDANCE WITH THESE REGULATIONS.
- f. APPLICABLE SAFETY REGULATIONS SHALL BE COMPLIED WITH.
- IT. WHERE CONSTRUCTION VEHICLE ACCESS ROUTES INTERSECT PAVED OR PUBLIC ROADS, PROVISIONS SHALL BE MADE TO MINIMIZE THE TRANSPORT OF SEDIMENT BY VEHICULAR TRACKING ONTO THE PAVED SURFACE.WHERE SEDIMENT IS TRANSPORTED ONTO A PAVED OR PUBLIC ROAD SURFACE, THE ROAD SURFACE SHALL BE CLEANED THOROUGHLY AT THE END OF EACH DAY. SEDIMENT SHALL BE REMOVED FROM THE ROADS BY SHOVELING OR SWEEPING AND TRANSPORTED TO A SEDIMENT CONTROL DISPOSAL AREA, STREET WASHING SHALL BE ALLOWED ONLY AFTER SEDIMENT IS REMOVED IN THIS MANNER. THIS PROVISION SHALL APPLY TO INDIVIDUAL DEVELOPMENT LOTS AS WELL AS TO LARGER LAND-DISTURBING ACTIVITIES.
- 18. ALL TEMPORARY EROSION AND SEDIMENT CONTROL MEASURES SHALL BE REMOVED WITHIN 30 DAYS AFTER FINAL SITE STABILIZATION OR AFTER THE TEMPORARY MEASURES ARE NO LONGER NEEDED,UNLESS OTHERWISE AUTHORIZED BY THE LOCAL PROGRAM AUTHORITY. TRAPPED MEASURES SHALL BE PERMANENTLY STABILIZED TO PREVENT FURTHER EROSION AND SEDIMENTATION.
- 19. PROPERTIES AND WATERWAYS DOWNSTREAM FROM DEVELOPMENT SITE SHALL BE PROTECTED FROM SEDIMENT DEPOSITION, EROSION AND DAMAGE DUE TO INCREASE IN VOLUME, VELOCITY AND PEAK FLOW RATE OF STORMWATER RUNOFF FOR THE STATED FREQUENCY STORM OF 24-HOUR DURATION IN ACCORDANCE WITH THE FOLLOWING STANDARDS AND CRITERIA:
- a. CONCENTRATED STORMWATER RUNOFF LEAVING A DEVELOPMENT SITE SHALL BE DISCHARGED DIRECTLY INTO AN ADEQUATE NATURAL OR MAN-MADE RECEIVING CHANNEL,PIPE OR STORM SEWER SYSTEM. FOR THOSE SITES WHERE RUNOFF IS DISCHARGED INTO A PIPE OR PIPE SYSTEM, DOWNSTREAM STABILITY ANALYSES AT THE OUTFALL OF THE PIPE OR PIPE SYSTEM SHALL BE PERFORMED.
- b. ADEQUACY OF ALL CHANNELS AND PIPES SHALL BE VERIFIED IN THE FOLLOWING MANNER: (I) THE APPLICANT SHALL DEMONSTRATE THAT THE TOTAL DRAINAGE AREA TO THE POINT OF ANALYSIS WITHIN THE CHANNEL IS ONE
- HUNDRED TIMES GREATER THAN THE CONTRIBUTING DRAINAGE AREA OF THE PROJECT IN QUESTION; OR
- (a) NATURAL CHANNELS SHALL BE ANALYZED BY THE USE OF A TWO-YEAR STORM TO VERIFY THAT STORMWATER WILL NOT OVERTOP CHANNEL BANKS NOR CAUSE EROSION OF CHANNEL BED AND BANKS; AND (b) ALL PREVIOUSLY CONSTRUCTED MAN-MADE CHANNELS SHALL BE ANALYZED BY THE USE OF A TEN-YEAR STORM TO VERIFY THAT
- STORMWATER WILL NOT OVERTOP ITS BANKS AND BY THE USE OF A TWO-YEAR STORM TO DEMONSTRATE THAT STORMWATER WILL NOT CAUSE EROSION OF CHANNEL BED OR BANKS; AND (c) PIPES AND STORM SEWER SYSTEMS SHALL BE ANALYZED BY THE USE OF A TEN-YEAR STORM TO VERIFY THAT STORMWATER WILL BE CONTAINED WITHIN THE PIPE OR SYSTEM. IF EXISTING NATURAL RECEIVING CHANNELS OR PREVIOUSLY CONSTRUCTED MAN-MADE CHANNELS OR PIPES ARE NOT ADEQUATE, THE APPLICANT SHALL:
- (I) IMPROVE THE CHANNEL TO A CONDITION WHERE A TEN-YEAR STORM WILL NOT OVERTOP THE BANKS AND A TWO-YEAR STORM WILL NOT CAUSE EROSION TO THE CHANNEL BED OR BANKS; OR
- (2) IMPROVE THE PIPE OR PIPE SYSTEM TO A CONDITION WHERE THE TEN-YEAR STORM IS CONTAINED WITHIN THE APPURTENANCES;OR (3) DEVELOP A SITE DESIGN THAT WILL NOT CAUSE THE PRE-DEVELOPMENT PEAK RUNOFF RATE FROM A TWO-YEAR STORM TO INCREASE 4. A COPY OF THE APPROVED EROSION AND SEDIMENT CONTROL PLAN AND PERMIT SHALL BE KEPT ON THE SITE AT ALL TIMES. WHEN RUNOFF OUTFALLS INTO A NATURAL CHANNEL OR WILL NOT CAUSE THE PRE-DEVELOPMENT PEAK RUNOFF RATE FROM A TEN-YEAR STORM TO INCREASE WHEN RUNOFF OUTFALLS INTO A MAN-MADE CHANNEL; OR

- (4) PROVIDE A COMBINATION OF CHANNEL IMPROVEMENT, STORMWATER DETENTION OR OTHER MEASURES WHICH IS SATISFACTORY TO THE PLAN-APPROVING AUTHORITY TO PREVENT DOWNSTREAM EROSION.
- d. THE APPLICANT SHALL PROVIDE EVIDENCE OF PERMISSION TO MAKE THE IMPROVEMENTS. e. ALL HYDROLOGIC ANALYSES SHALL BE BASED ON EXISTING WATERSHED CHARACTERISTICS AND THE ULTIMATE DEVELOPMENT OF THE SUBJECT PROJECT.
- f. IF THE APPLICANT CHOOSES AN OPTION THAT INCLUDES STORMWATER DETENTION HE SHALL OBTAIN APPROVAL FROM THE LOCALITY OF PLAN FOR MAINTENANCE OF THE DETENTION FACILITIES. THE PLAN SHALL SET FORTH THE MAINTENANCE REQUIREMENTS OF THE FACILITY AND THE PERSON RESPONSIBLE FOR PERFORMING THE MAINTENANCE.
- g. OUTFALL FROM A DETENTION FACILITY SHALL BE DISCHARGED TO A RECEIVING CHANNEL, AND ENERGY DISSIPATERS SHALL BE PLACED AT THE OUTFALL OF ALL DETENTION FACILITIES AS NECESSARY TO PROVIDE A STABILIZED TRANSITION FROM THE FACILITY TO THE RECEIVING CHANNEL.
- h. ALL ON-SITE CHANNELS MUST BE VERIFIED TO BE ADEQUATE. i. INCREASED VOLUMES OF SHEET FLOWS THAT MAY CAUSE EROSION OR SEDIMENTATION ON ADJACENT PROPERTY SHALL BE DIVERTED TO A STABLE OUTLET, ADEQUATE CHANNEL, PIPE OR PIPE SYSTEM, OR TO A DETENTION FACILITY.
- Î. IN APPLYING THESE STORMWATER RUNOFF CRITERIA,INDIVIDUAL LOTS OR PARCELS IN A RESIDENTIAL,COMMERCIAL,OR INDUSTRIAL DEVELOPMENT SHALL NOT BE CONSIDERED TO BE SEPARATE DEVELOPMENT PROJECTS. INSTEAD, THE DEVELOPMENT, AS A WHOLE, SHALL BE CONSIDERED TO BE A SINGLE DEVELOPMENT PROJECT. HYDROLOGIC PARAMETERS THAT REFLECT THE ULTIMATE DEVELOPMENT CONDITION SHALL BE USED IN ALL ENGINEERING CALCULATIONS.
- k. ALL MEASURES USED TO PROTECT PROPERTIES AND WATERWAYS SHALL BE EMPLOYED IN A MANNER WHICH MINIMIZES IMPACTS ON PHYSICAL, CHEMICAL AND BIOLOGICAL INTEGRITY OF RIVERS, STREAMS AND OTHER WATERS OF THE STATE.

<u>MINIMUM STANDARDS (MS-19) NARRATIVE</u>

- I. INSTRUCTION FOR TEMPORARY SOIL STABILIZATION REQUIREMENTS ARE PROVIDED ON THE PLANS, SEE THE E&S NARRATIVE VEGETATIVE PRACTICES *I ON SHEET *IP(I) AND SEED MIXTURES AND APPLICATION RATES IN TABLE 3.3I-B ON SHEET *IP(I). INSTRUCTION FOR PERMANENT STABILIZATION REQUIREMENTS ARE PROVIDED ON THE PLANS, SEE THE E&S NARRATIVE - VEGETATIVE PRACTICES *2 ON SHEET *IP(I).
- 2. DURING CONSTRUCTION OF THE PROJECT, ANY SOIL STOCKPILES AND BORROW AREAS SHALL BE STABILIZED OR PROTECTED WITH SEDIMENT TRAPPING MEASURES. THE CONTRACTOR IS RESPONSIBLE FOR THE TEMPORARY PROTECTION AND PERMANENT STABILIZATION OF ALL SOILS STOCKPILES ON SITE AS WELL AS BORROW AREAS AND SOIL INTENTIONALLY TRANSPORTED FROM THE
- 3. A NOTE STATING THE DEFINITION OF PERMANENT STABILIZATION IS PROVIDED ON THE PLANS, SEE THE E&S NARRATIVE -VEGETATIVE PRACTICES *2 ON SHEET *IP(I) AND TABLE 3.32-D ON SHEET *IP(I).
- 4. ALL SEDIMENT TRAPPING MEASURES SHALL BE CONSTRUCTED AS A FIRST STEP PRIOR TO UP SLOPE LAND DISTURBANCE.
- 5. STABILIZATION MEASURES ARE PROVIDED FOR THE EARTHEN STRUCTURES, SEE THE E&S NARRATE MANAGEMENT STRATEGIES, TASK 4 AND SEED MIXTURES AND APPLICATION RATES IN TABLE 3.31-B ON SHEET IP(1).
- 6a. THE SEDIMENT TRAPS ARE CONTROLLING LESS THAN THREE ACRES AND ARE DESIGNED WITH A MINIMUM STORAGE CAPACITY OF 134 CUBIC YARDS PER ACRE, SEE STRUCTURAL PRACTICES ON SHEET IP(1).

- 7. CUT AND FILL SLOPES SHOWN ON THIS PLAN HAVE BEEN EVALUATED BY A GEOTECHNICAL ENGINEER AND DESIGNED TO PREVENT EROSION. INSTRUCTION FOR PERMANENT STABILIZATION REQUIREMENTS ARE PROVIDED ON THE PLANS, SEE THE E&S NARRATIVE -VEGETATIVE PRACTICES *2 ON SHEET *IJ.PROVIDE SURFACE ROUGHENING OR CRIMPING TO ENHANCE SEED GERMINATION.
- 8. ANY CONCENTRATED RUNOFF FROM THIS SITE IS OUTLET INTO AN ADEQUATE OUTFALL CHANNEL CHANNEL ADEQUACY WILL BE VERIFIED WITH THE SUBMISSION OF THE ROADWAY CONSTRUCTION PLANS AND DRAINAGE CALCULATIONS.
- 9. IT IS NOT KNOWN AT THIS TIME IF THERE WILL BE ANY SEEPAGE OF WATER FROM UNDERGROUND, WHENEVER WATER SEEPS FROM A SLOPE FACE, ADEQUATE DRAINAGE OR OTHER PROTECTION SHALL BE PROVIDED.
- IO. INLET PROTECTION HAS BEEN PROVIDE FOR ALL STORM SEWER INLETS DOWN SLOPE OF THE DISTURBANCE ACTIVITIES, SEE PLAN SHEETS IQ(3) THRU IR(14).
- II. OUTLET PROTECTIONS ARE PROVIDED ON THE PLAN AT THE APPROPRIATE LOCATIONS, SEE EROSION AND SEDIMENT CONTROL SHEETS IQ(3) THRU IR(14).
- 12. ENCROACHMENT IN THE WATERCOURSE SHALL BE LIMITED TO THE LIMITS OF DISTURBANCE, WHICH ARE AT A MINIMUM TO CONSTRUCT THE ROAD. SILT FENCE AND DIVERSION DIKES ARE PROVIDED TO CONTROL SEDIMENT TRANSPORT.
- 13. WHERE IT IS NECESSARY TO CROSS A LIVE WATERCOURSE, A TEMPORARY OR PERMANENT VEHICULAR STREAM CROSSING, IN ACCORDANCE WITH VESCH 3.24, SHALL BE PROVIDED.
- 14. ALL APPLICABLE FEDERAL, STATE AND LOCAL REGULATIONS PERTAINING TO WORKING IN OR CROSSING LIVE WATERCOURSES SHALL BE
- 15. THE BED AND BANKS OF THE DISTURBED WATERCOURSES ARE TO BE IMMEDIATELY STABILIZED AFTER WORK IS COMPLETED.
- 16 UNDERGROUND UTILITIES a. NO MORE THAN 500 LINEAR FEET OF TRENCH MAY BE OPENED AT ONE TIME.
- b. EXCAVATED MATERIAL SHALL BE PLACED ON UPHILL SIDE OF TRENCHES.
- c. EFFLUENT FROM DEWATERING OPERATION SHALL BE FILTERED OR PASSED THROUGH AN APPROVED SEDIMENT TRAPPING DEVICE, OR BOTH, AND DISCHARGED IN A MANNER THAT DOES ONT ADVERSELY AFFECT FLOWING STREAMS OR OFF-SITE PROPERTY. d. MATERIAL USED FOR BACK FILLING TRENCHES SHALL BE PROPERLY COMPACTED IN ORDER TO MINIMIZE EROSION AND PROMOTE
- STABILIZATION. e. TEMPORARY AND PERMANENT SEEDING SCHEDULES ARE PROVIDED ON SHEET *IP(I), SEE TABLES 3.3IB & 3.32D.
- 17. A NOTE REQUIRING THE CLEANING OF ADJACENT TRAVELWAYS IS SHOWN ON THE PLAN, SEE STRUCTURAL PRACTICES TEMPORARY CONSTRUCTION ENTRANCE 3.02 SEE SHEET *IP(I).
- I8. A NOTE REQUIRING THE REMOVAL OF ALL TEMPORARY EROSION AND SEDIMENT CONTROL MEASURES SHALL BE REMOVED WITHIN 30 DAYS AFTER FINAL SITE STABILIZATION IS PROVIDED ON THE PLAN, SEE MAINTENANCE STRATEGIES-SEDIMENT & EROSION CONTROL SEE SHEET *IP(I).
- 19. STORMWATER OUTFALL ADEQUACY WILL BE VERIFIED WITH THE SUBMISSION OF THE ROADWAY CONSTRUCTION PLANS AND DRAINAGE CALCULATIONS.

FROSION & SFDIMENT CONTROL STANDARD NOTES:

- I. THE OWNER/DEVELOPER MUST NOTIFY THE DEPARTMENT OF PUBLIC WORKS AT 792-7070 AT LEAST 24 HOURS PRIOR TO THE START OF CONSTRUCTION IN ACCORDANCE WITH APPLICABLE COUNTY ORDINANCES AND POLICIES.
- 2. THE OWNER/DEVELOPER GRANTS THE RIGHT-OF-ENTRY ON TO THIS PROPERTY TO THE DESIGNATED PRINCE WILLIAM COUNTY PERSONNEL FOR THE PURPOSE OF INSPECTING AND MONITORING FOR COMPLIANCE WITH TITLE 10.01, CHAPTER 5, ARTICLE 4 OF THE CODE OF VIRGINIA, EROSION AND SEDIMENT CONTROL LAW AND THE DESIGN AND CONSTRUCTION STANDARDS MANUAL SECTION 750.04
- 3. ALL EROSION CONTROL MEASURES SHOWN ON THE APPROVED PLAN MUST BE IN PLACE AND INSPECTED AND APPROVED BY THE DEPARTMENT OF PUBLIC WORKS PRIOR TO CLEARING, STRIPPING OF TOPSOIL OR GRADING.

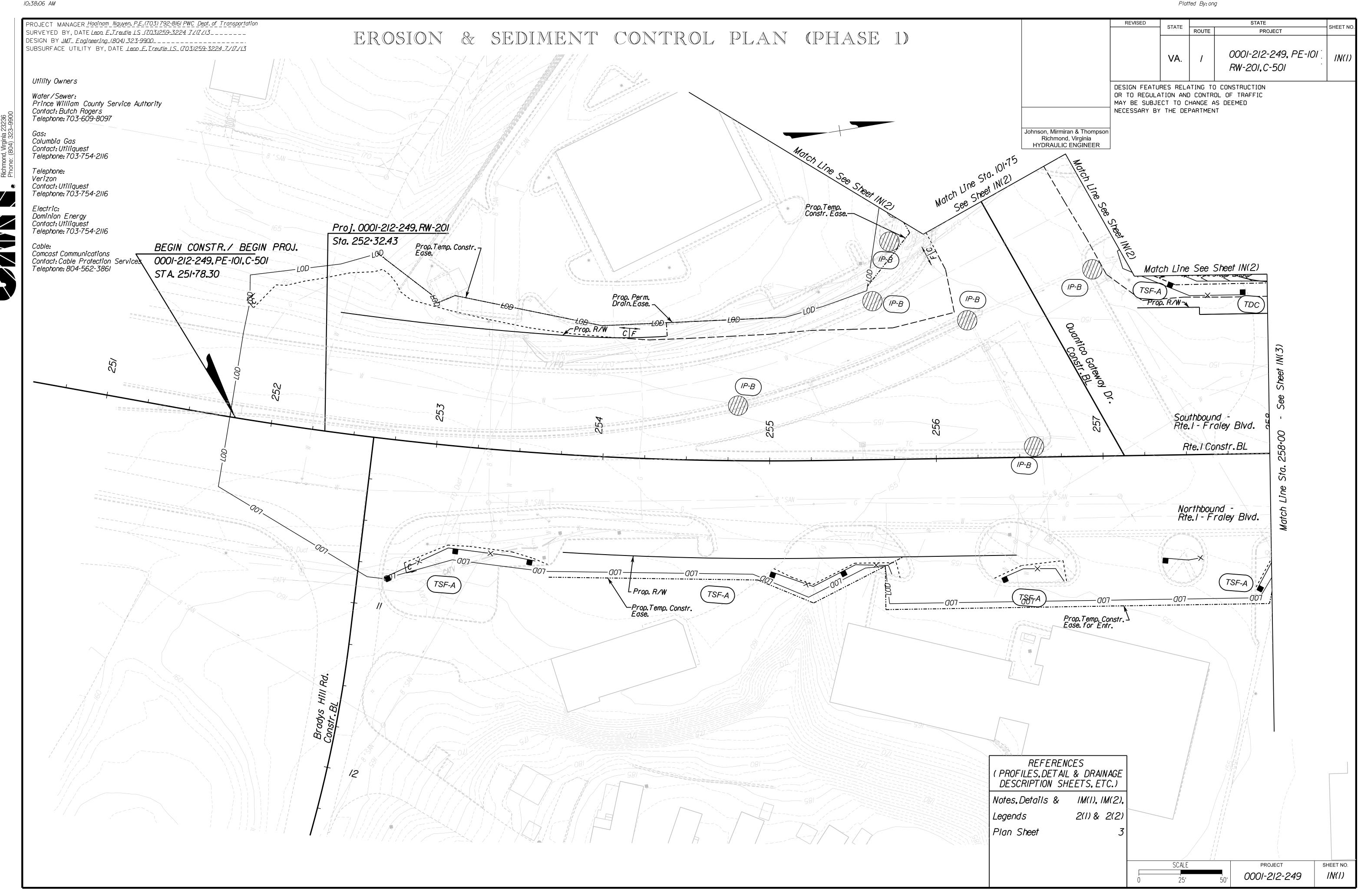
REVISED STATE SHEET N ROUTE PROJECT 0001-212-249, IM(2) VA. RW-201, C-501

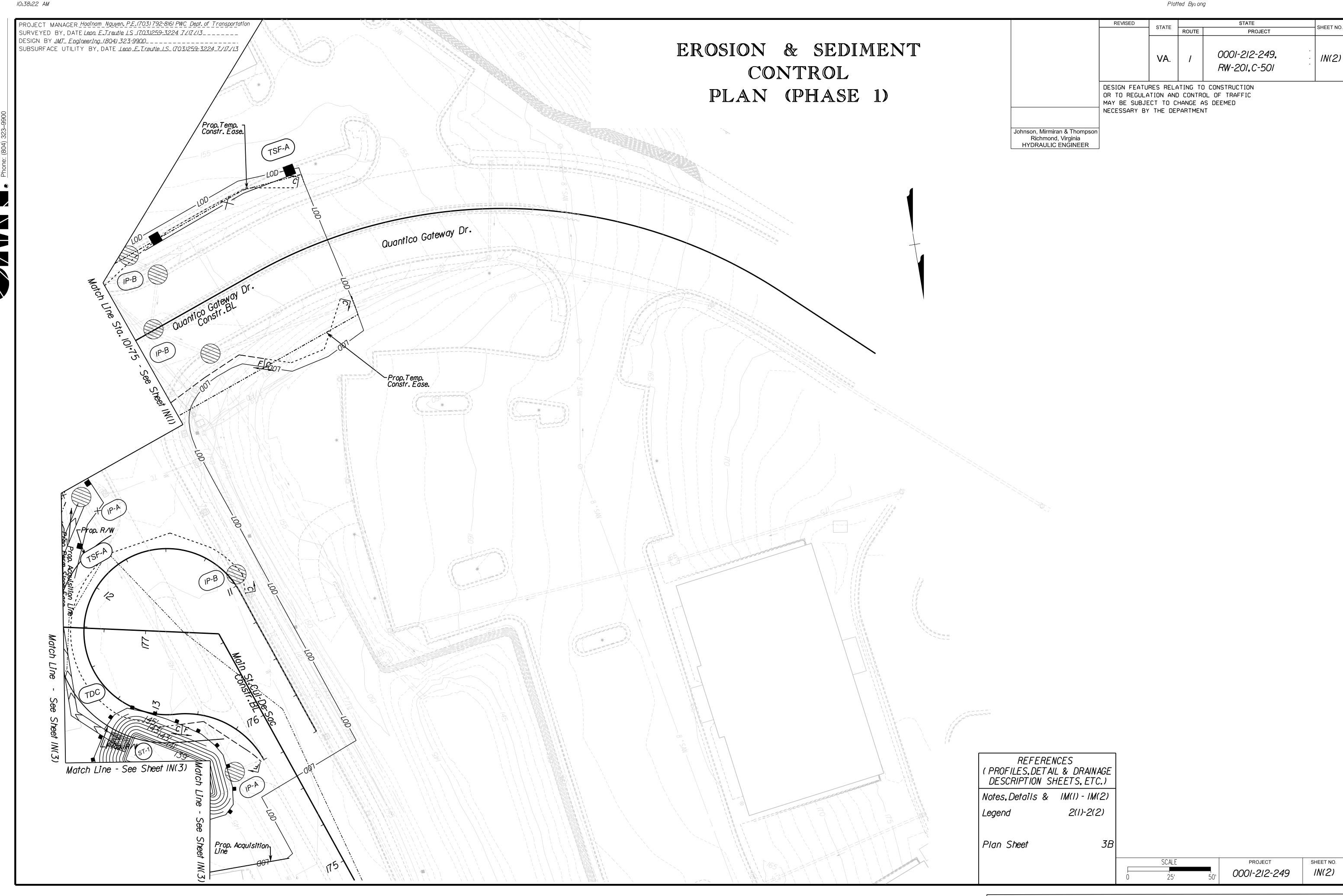
DESIGN FEATURES RELATING TO CONSTRUCTION OR TO REGULATION AND CONTROL OF TRAFFIC MAY BE SUBJECT TO CHANGE AS DEEMED NECESSARY BY THE DEPARTMENT

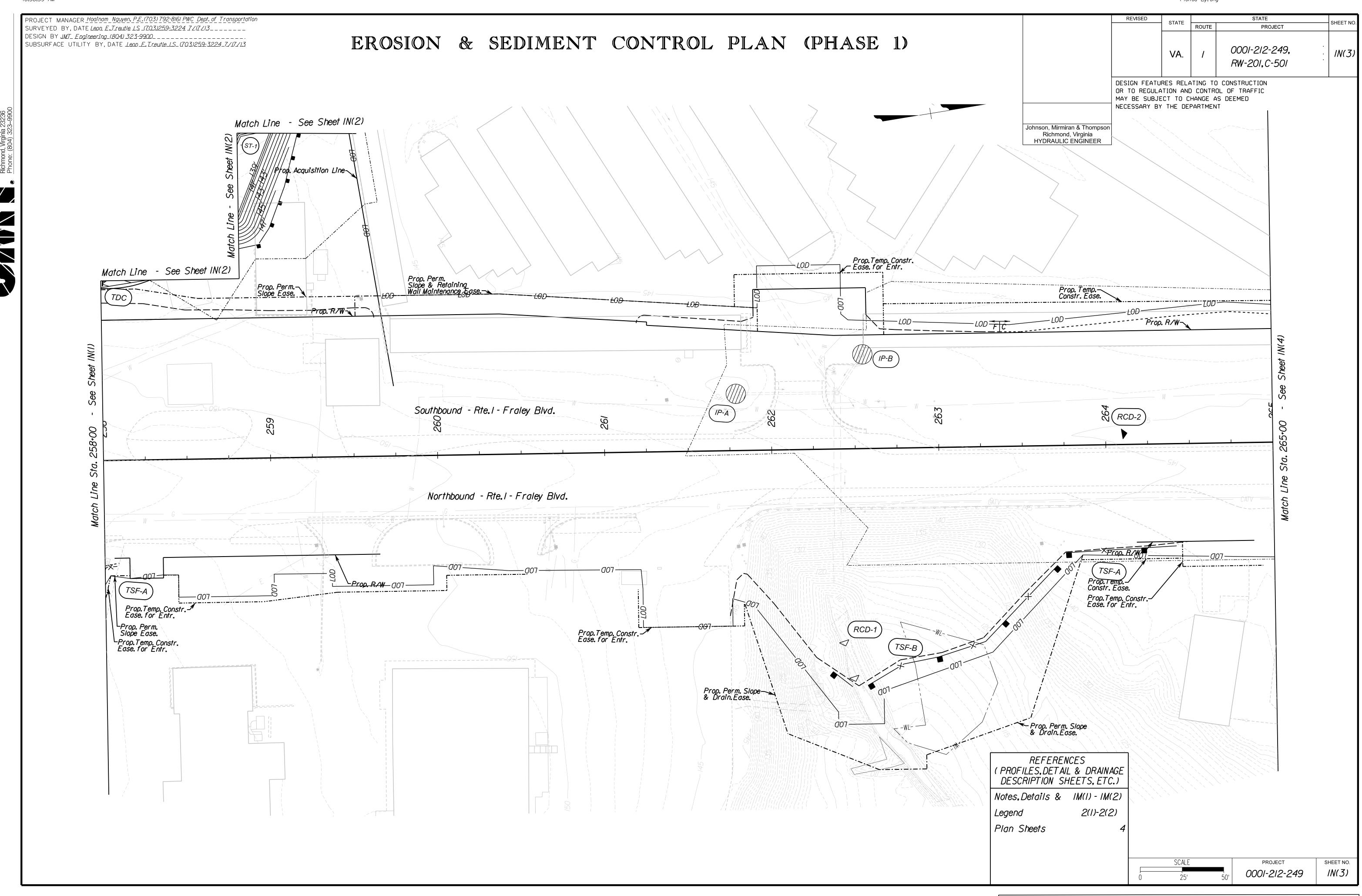
Johnson, Mirmiran & Thompson Richmond, Virginia HYDRAULIC ENGINEER

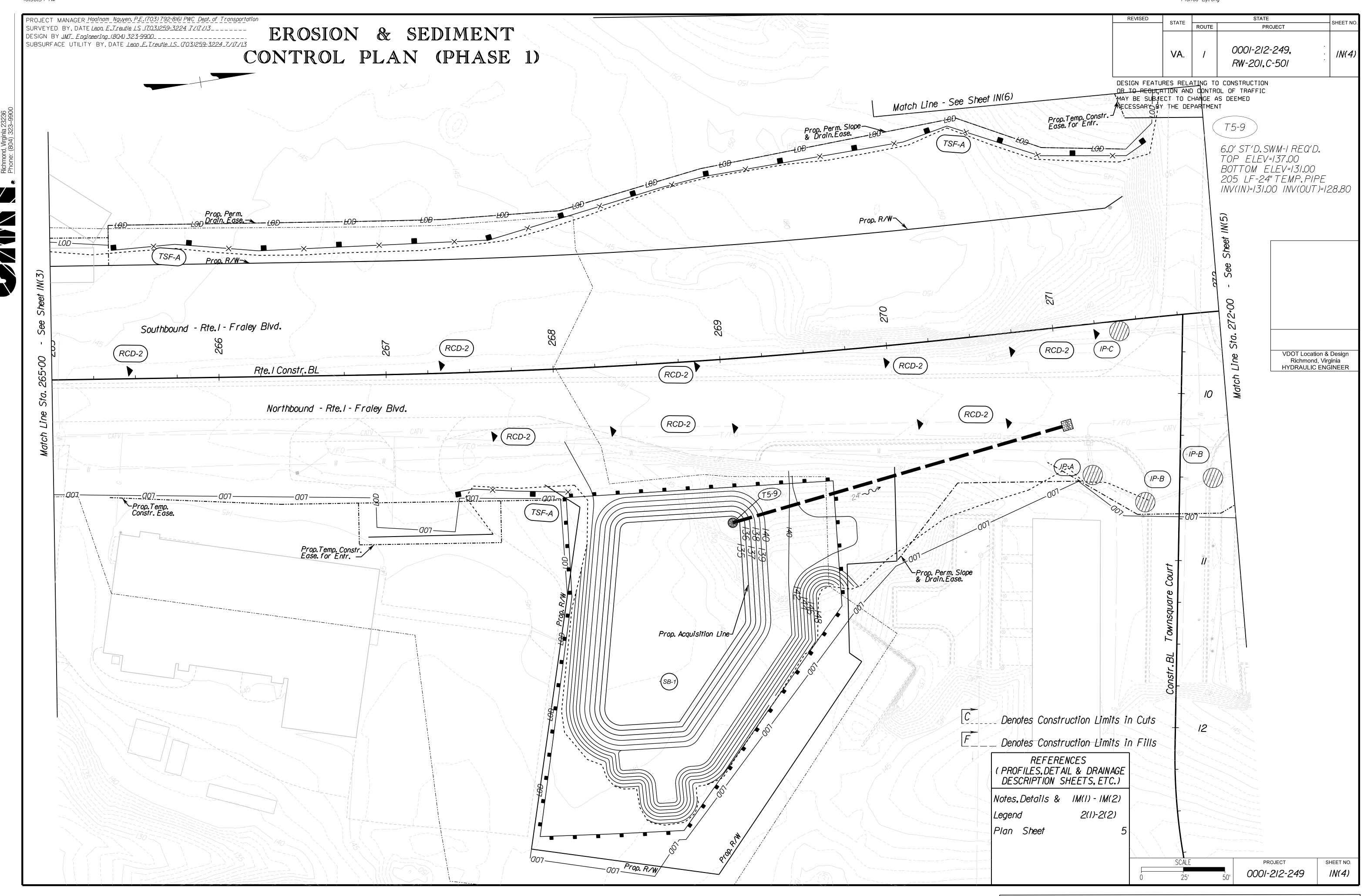
- 5. THE DEVELOPER/DEVELOPER'S REPRESENTATIVE IS RESPONSIBLE FOR THE INSTALLATION OF ANY ADDITIONAL EROSION CONTROL MEASURES NECESSARY TO PREVENT EROSION AND SEDIMENTATION AS DETERMINED BY PRINCE WILLIAM COUNTY.
- 6. ALL DISTURBED AREAS ARE TO DRAIN TO APPROVED SEDIMENT CONTROL MEASURES AT ALL TIMES DURING LAND DISTURBING ACTIVITIES AND DURING SITE DEVELOPMENT UNTIL COMPLETE AND ADEQUATE STABILIZATION IS ACHIEVED.
- 7. WATER MUST BE PUMPED INTO AN APPROVED FILTERING DEVICE DURING DEWATERING OPERATIONS.
- 8. ALL EROSION AND SEDIMENT CONTROL PRACTICES MUST BE CONSTRUCTED AND MAINTAINED ACCORDING TO THE MINIMUM STANDARDS AND SPECIFICATIONS OF THE VIRGINIA EROSION AND SEDIMENT CONTROL HANDBOOK AND THE VIRGINIA REGULATIONS VR 625-02-00 EROSION AND SEDIMENT CONTROL REGULATIONS AND TO THE PRINCE WILLIAM COUNTY DESIGN AND CONSTRUCTION STANDARDS MANUAL.
- 9. THE DEVELOPER/DEVELOPER'S REPRESENTATIVE WILL BE RESPONSIBLE FOR THE INSTALLATION AND MAINTENANCE OF ALL EROSION AND SEDIMENT CONTROL PRACTICES AT ALL TIMES.
- THE DEVELOPER/DEVELOPER'S REPRESENTATIVE SHALL INSPECT ALL EROSION AND SEDIMENT CONTROL MEASURES DAILY AND AFTER EACH SIGNIFICANT RAINFALL. THE FOLLOWING ITEMS WILL BE CHECKED IN PARTICULAR:
- A. SEDIMENT BASINS WILL BE CLEANED OUT WHEN THE LEVEL OF SEDIMENT BUILDUP REACHES THE CLEANOUT ELEVATION INDICATED ON THE RISER PIPE. SEDIMENT SHALL BE DISPOSED IN SUITABLE AREAS AND IN SUCH A MANNER THAT WILL NOT ERODE OR CAUSE SEDIMENTATION PROBLEMS. THE BASIN EMBANKMENT SHOULD BE CHECKED REGULARLY TO ENSURE THAT IT STRUCTURALLY SOUND AND HAS NOT BEEN DAMAGED BY EROSION OR CONSTRUCTION EQUIPMENT. EMERGENCY SPILLWAYS SHOULD BE CHECKED REGULARLY TO ENSURE THAT ITS LINING IS WELL ESTABLISHED AND EROSION RESISTANT.
- B. SEDIMENT TRAPS WILL BE CHECKED REGULARLY FOR SEDIMENT CLEANOUT.SEDIMENT SHALL BE REMOVED AND THE TRAP RESTORED TO ITS ORIGINAL DIMENSIONS WHEN THE SEDIMENT HAS ACCUMULATED TO ONE HALF THE DESIGN VOLUME OF THE WET STORAGE.SEDIMENT REMOVED FROM THE TRAP SHALL BE DEPOSITED IN A SUITABLE AREA AND IN SUCH A MANNER THAT IT WILL NOT ERODE AND CAUSE SEDIMENTATION PROBLEMS.
- C. GRAVEL OUTLETS WILL BE CHECKED REGULARLY FOR SEDIMENT BUILDUP WHICH WILL PREVENT DRAINAGE. IF THE GRAVEL IS CLOGGED BY SEDIMENT,IT SHALL BE REMOVED AND CLEANED OR REPLACED.
- D. SILT FENCE BARRIERS WILL BE CHECKED REGULARLY FOR UNDERMINING OR DETERIORATION OF THE FABRIC. SEDIMENT SHALL BE REMOVED WHEN THE LEVEL OF SEDIMENT DEPOSITION REACHES HALF WAY TO THE TOP OF THE
- E. SEEDED AREAS WILL BE CHECKED REGULARLY TO ENSURE THAT A GOOD STAND IS MAINTAINED, AREAS SHOULD BE FERTILIZED AND RESEEDED AS NEEDED.
- F. STREAM DIVERSION AND STORM CONVEYANCE CHANNELS SHALL BE INSPECTED DAILY AND AFTER EACH RAIN TO ENSURE THEY'RE FUNCTIONING PROPERLY AND THAT THE INTEGRITY OF THE LININGS ARE NOT IMPAIRED. ANY NECESSARY REPAIRS OR CLEANUP TO MAINTAIN THE EFFECTIVENESS OF THE EROSION CONTROL DEVISES MUST BE MADE IMMEDIATELY AFTER THE INSPECTION.
- IO. SEDIMENT TRAPPING MEASURES WILL BE INSTALLED AS A FIRST STEP IN GRADING AND WILL BE SEEDED AND MULCHED IMMEDIATELY FOLLOWING INSTALLATION.
- II. PERMANENT SOIL STABILIZATION SHALL BE APPLIED TO DENUDED AREAS WITHIN SEVEN (7) DAYS AFTER FINAL GRADE IS REACHED ON ANY PORTION OF THE SITE. TEMPORARY SOIL STABILIZATION SHALL BE APPLIED WITHIN SEVEN (7) DAYS TO DENUDED AREAS THAT MAY NOT BE AT FINAL GRADE BUT WILL REMAIN UNDISTURBED FOR LONGER THAN FOURTEEN (14) DAYS. SEEDING AND SELECTION OF THE SEED MIXTURE SHALL BE IN ACCORDANCE WITH THE VIRGINIA EROSION AND SEDIMENT CONTROL HANDBOOK STANDARD AND SPECIFICATION 3.32. ROADS AND PARKING AREAS SHALL BE STABILIZED WITHIN SEVEN (7) DAYS AFTER FINAL GRADE IS REACHED.
- I2. ALL TEMPORARY EROSION AND SEDIMENT CONTROL MEASURES WILL BE REMOVED WITHIN 30 DAYS AFTER ADEQUATE SITE STABILIZATION AND AFTER THE TEMPORARY MEASURES ARE NO LONGER NEEDED, AS AUTHORIZED BY THE PRINCE WILLIAM COUNTY INSPECTORS.TRAPPED SEDIMENT AND THE DISTURBED SOIL AREAS RESULTING FROM THE DISPOSITION OF TEMPORARY MEASURES WILL BE PERMANENTLY STABILIZED TO PREVENT FURTHER EROSION AND SEDIMENTATION
- 13. WHEN SEDIMENT IS TRANSPORTED ONTO A PAVED ROAD SURFACE, THE ROAD WILL BE CLEANED THOROUGHLY AT THE END OF EACH DAY. SEDIMENT WILL BE REMOVED FROM THE ROADS BY SHOVELING OR SWEEPING AND TRANSPORTED TO A SEDIMENT CONTROL DISPOSAL AREA. STREET WASHING WILL BE ALLOWED ONLY AFTER SEDIMENT IS REMOVED IN THIS MANNER.
- 14. AREAS WHICH ARE NOT TO BE DISTURBED WILL BE CLEARLY MARKED BY FLAGS, SIGNS, ETC.
- 15. RPA AND FLOODPLAIN LIMITS SHALL BE CLEARLY MARKED IN THE FIELD BY FLAGS, SIGNS, ETC.
- 16. TREE SAVE AREAS SHALL BE CLEARLY MARKED IN THE FIELD BY ORANGE SAFETY FENCE.
- I7. ORANGE SAFETY FENCE MUST BE INSTALLED AROUND ALL SILT TRAPS AND SEDIMENT BASINS.

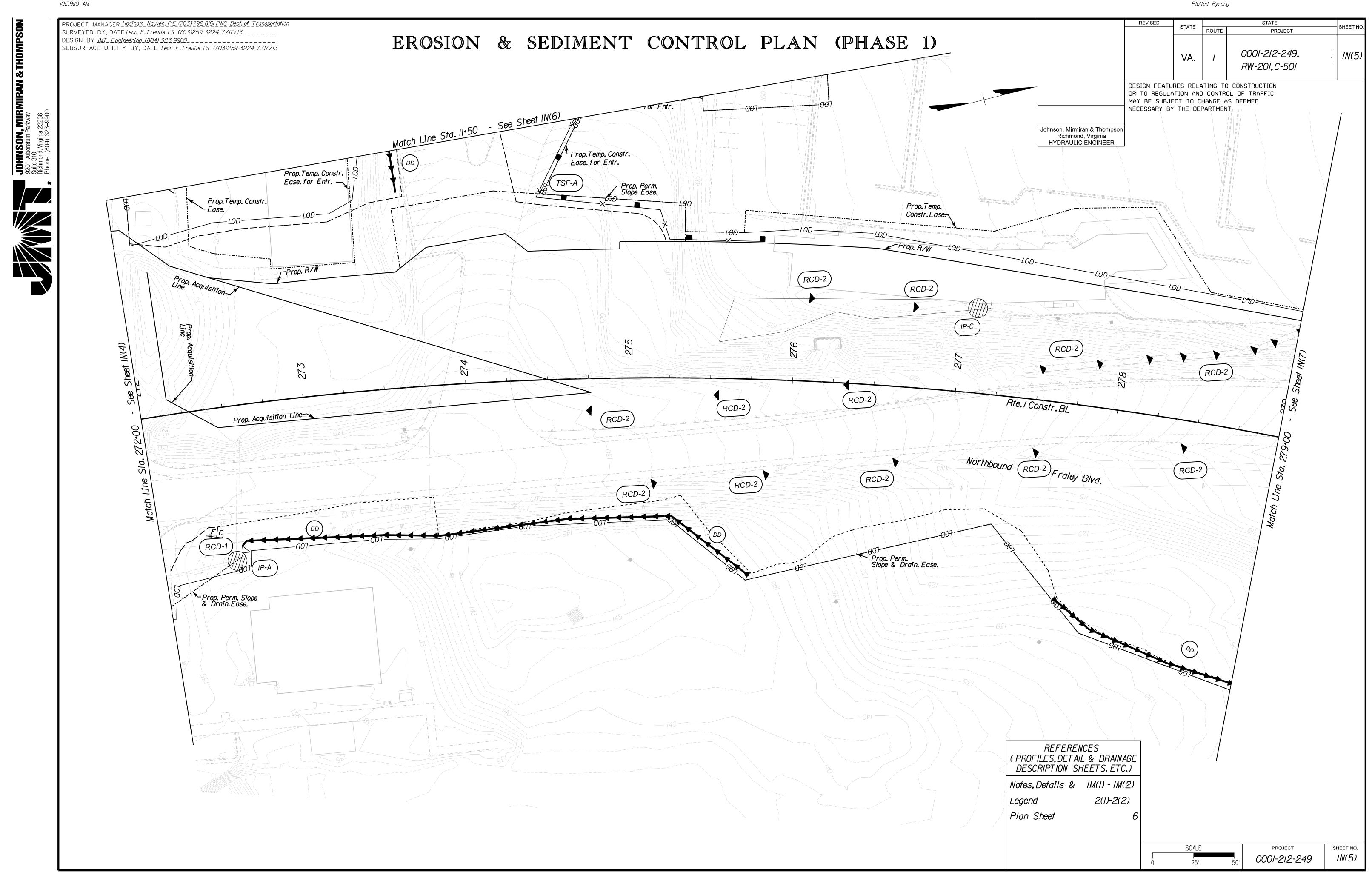
PROJECT SHEET NO. 0001-212-249

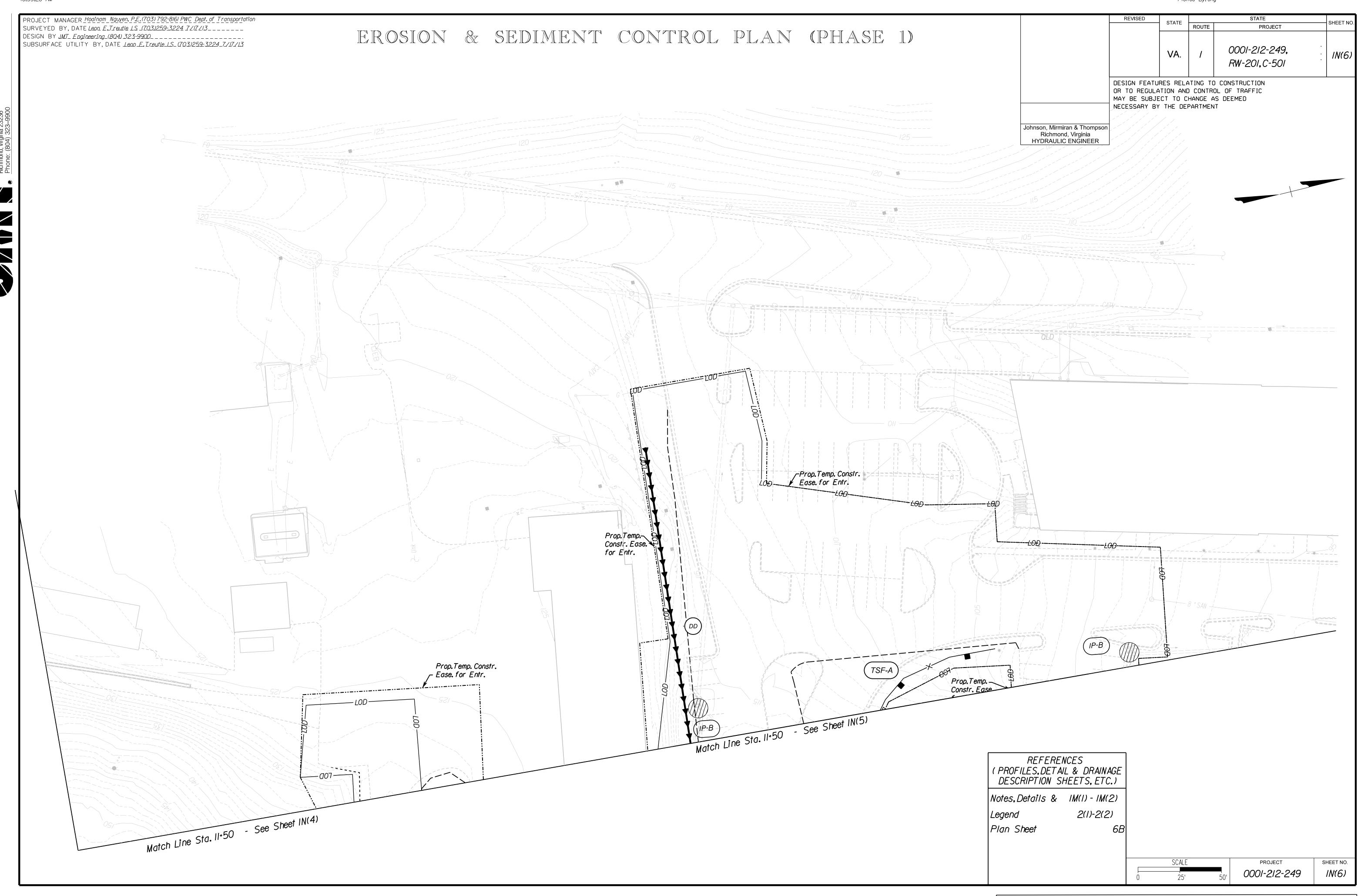


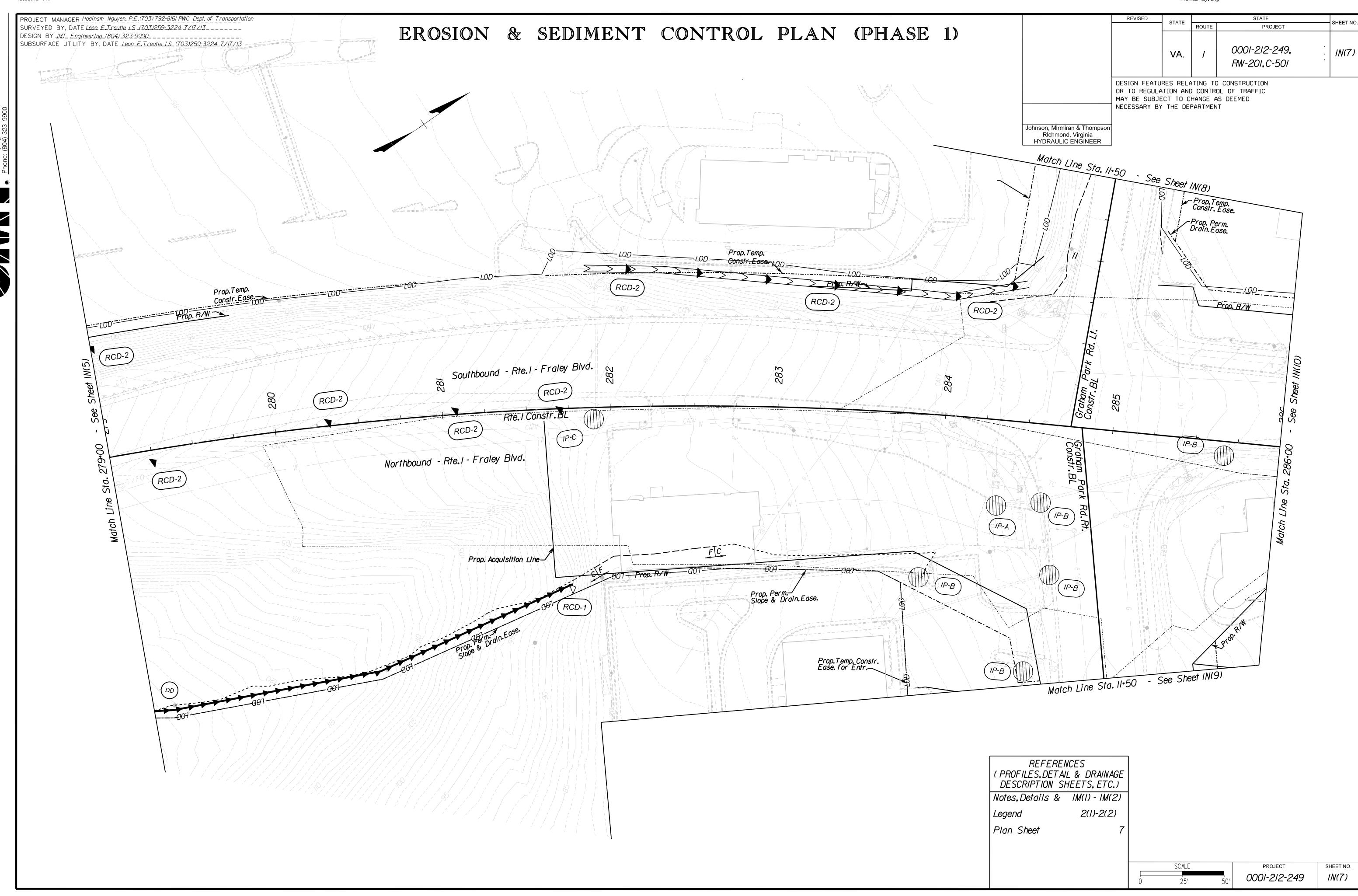


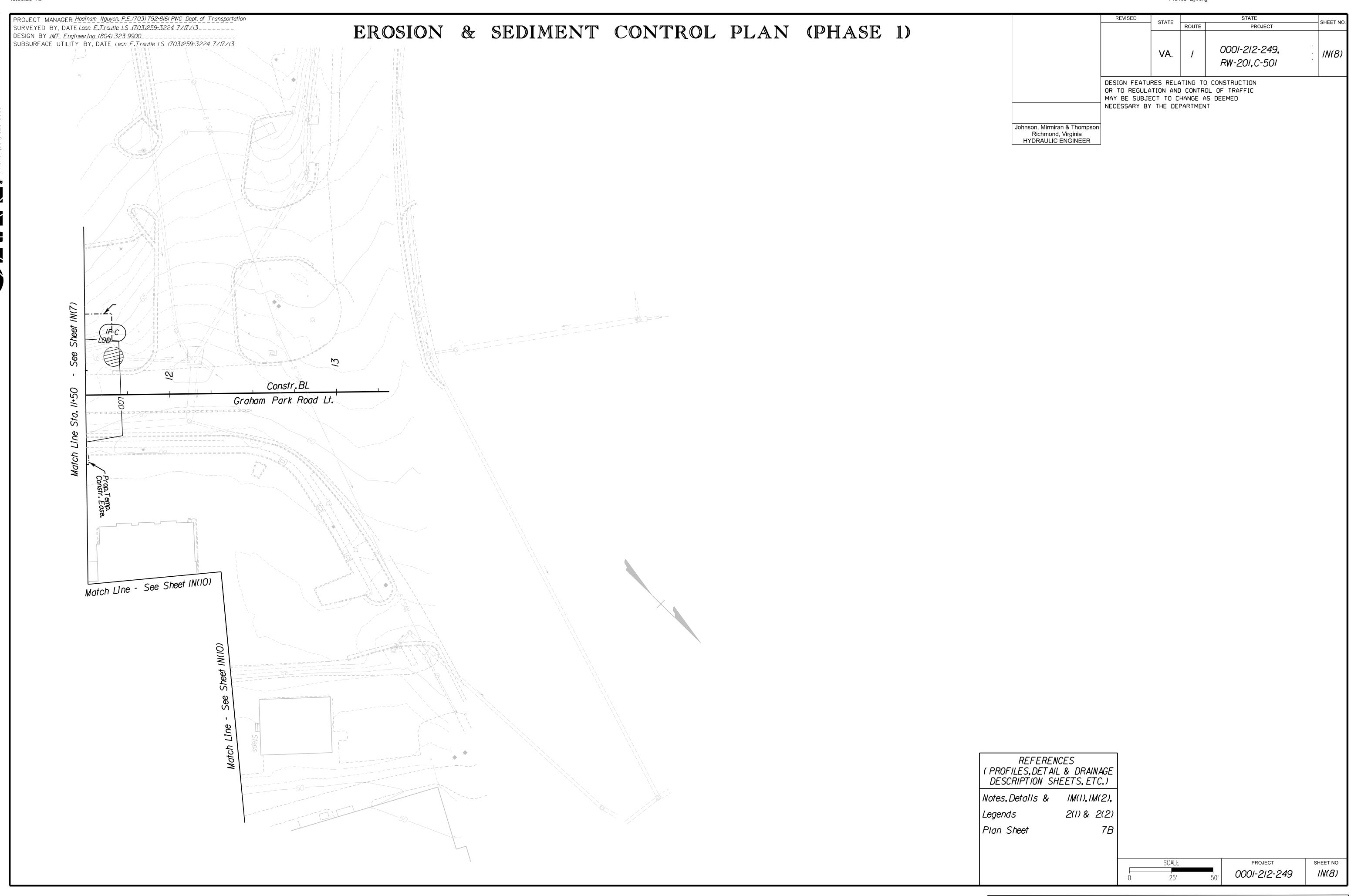


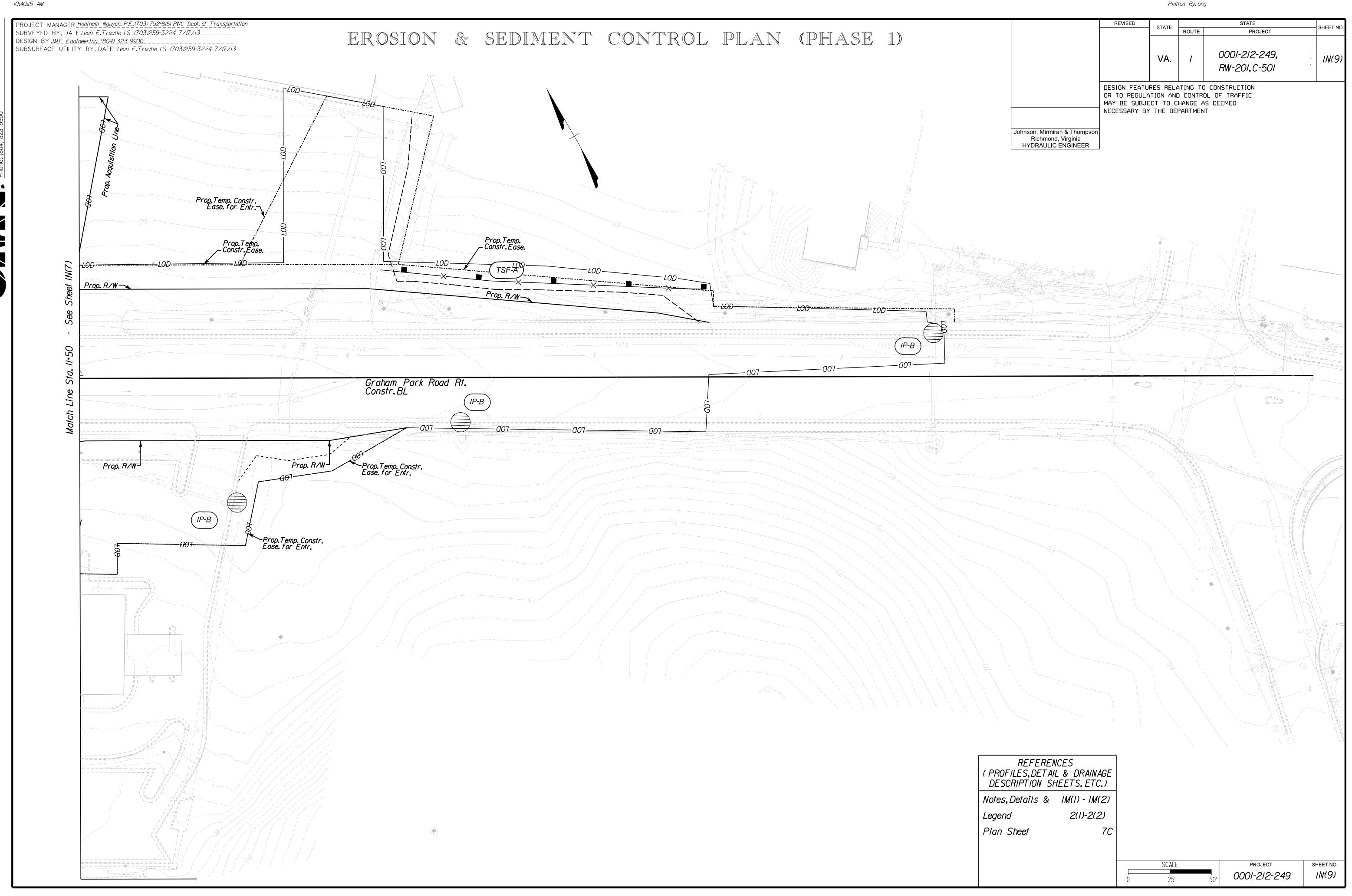


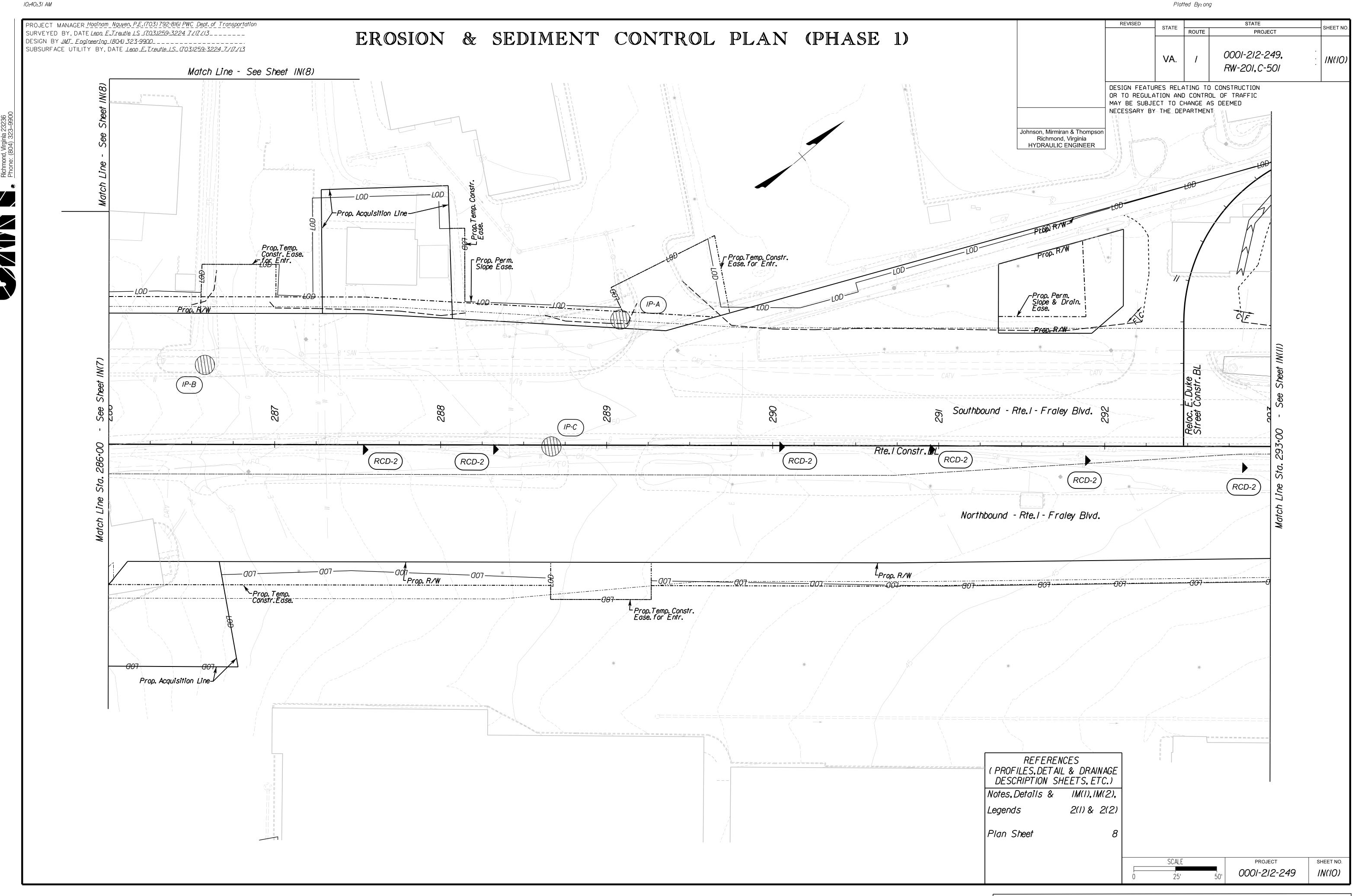


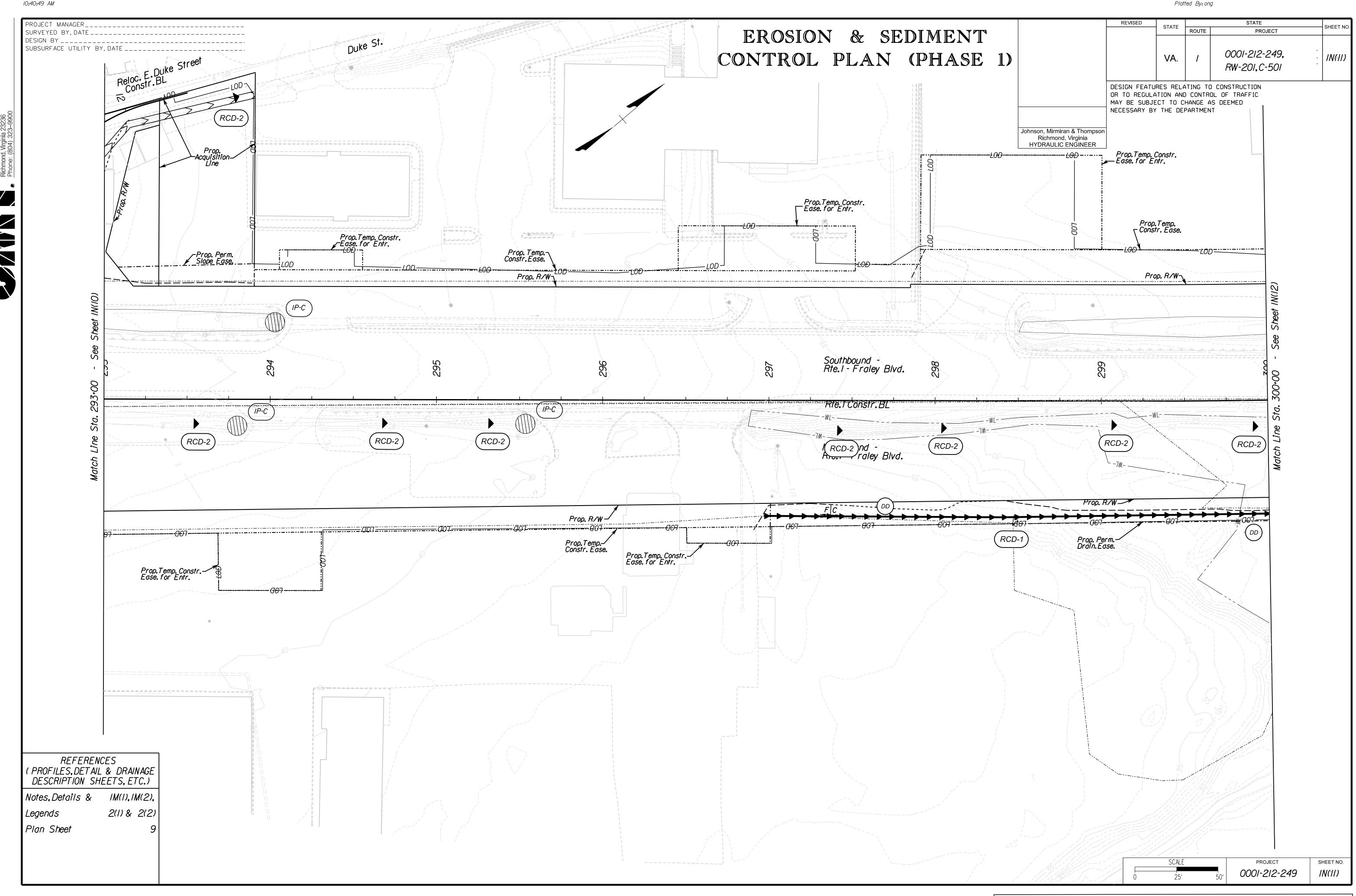




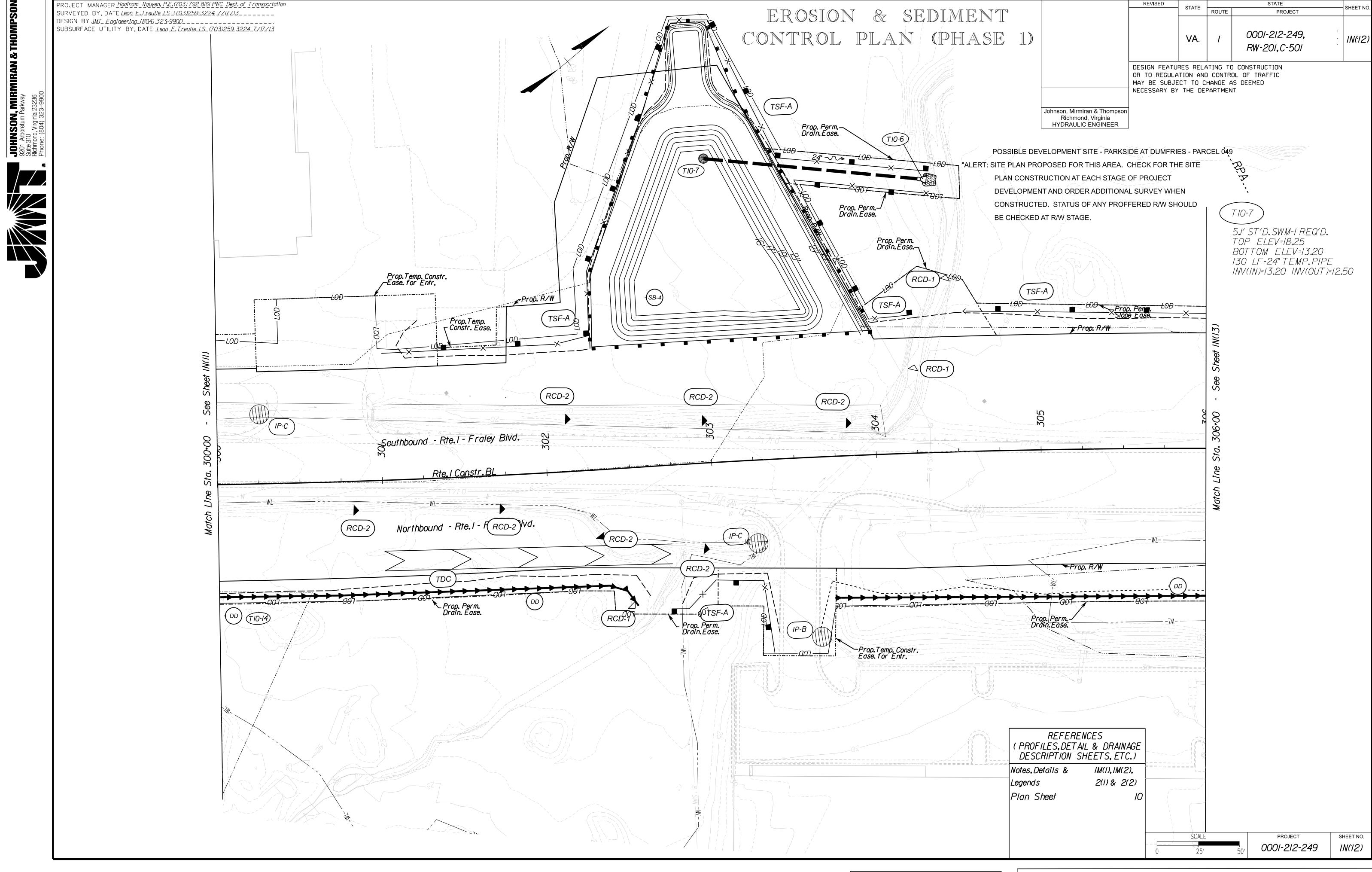




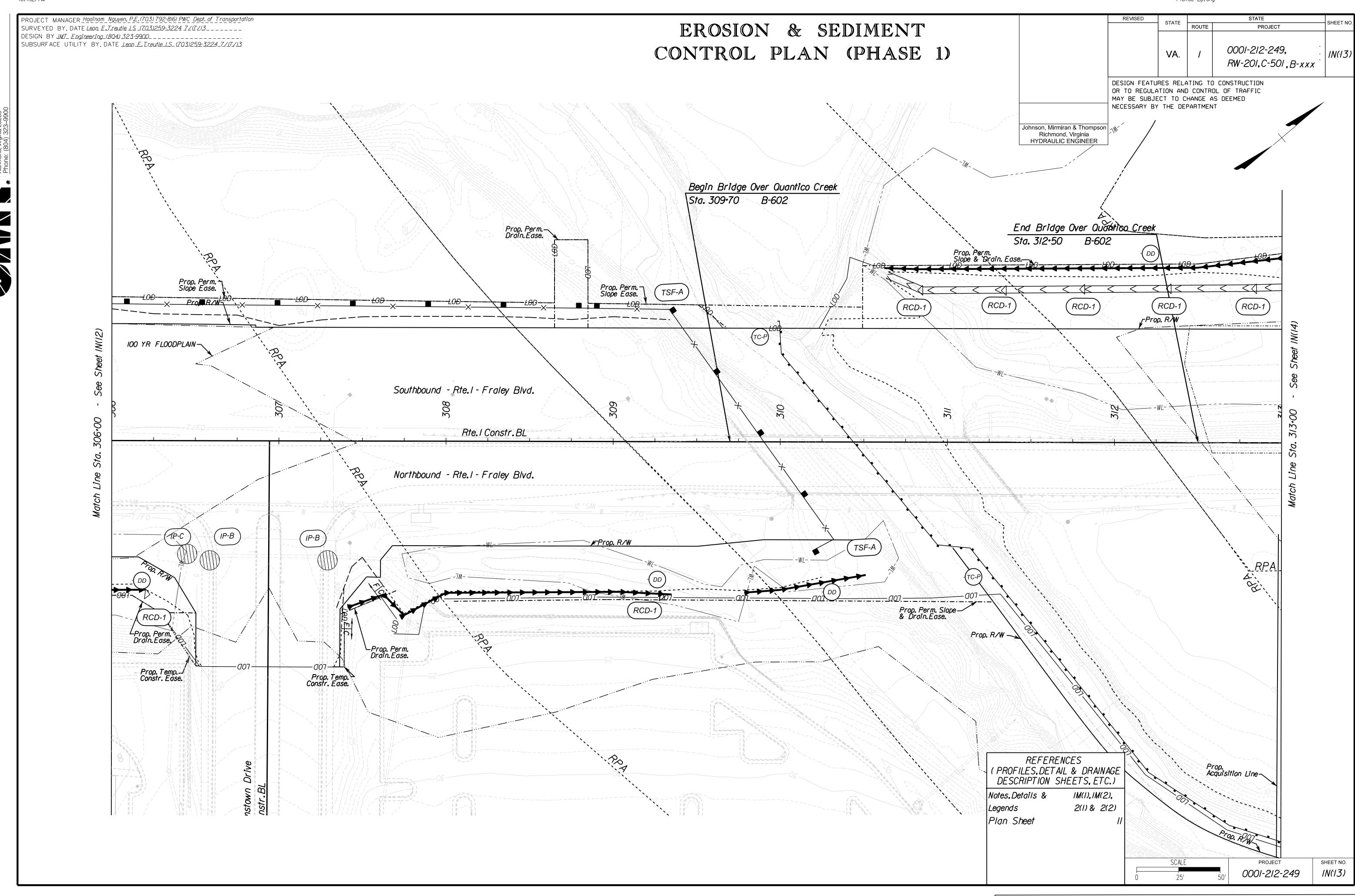


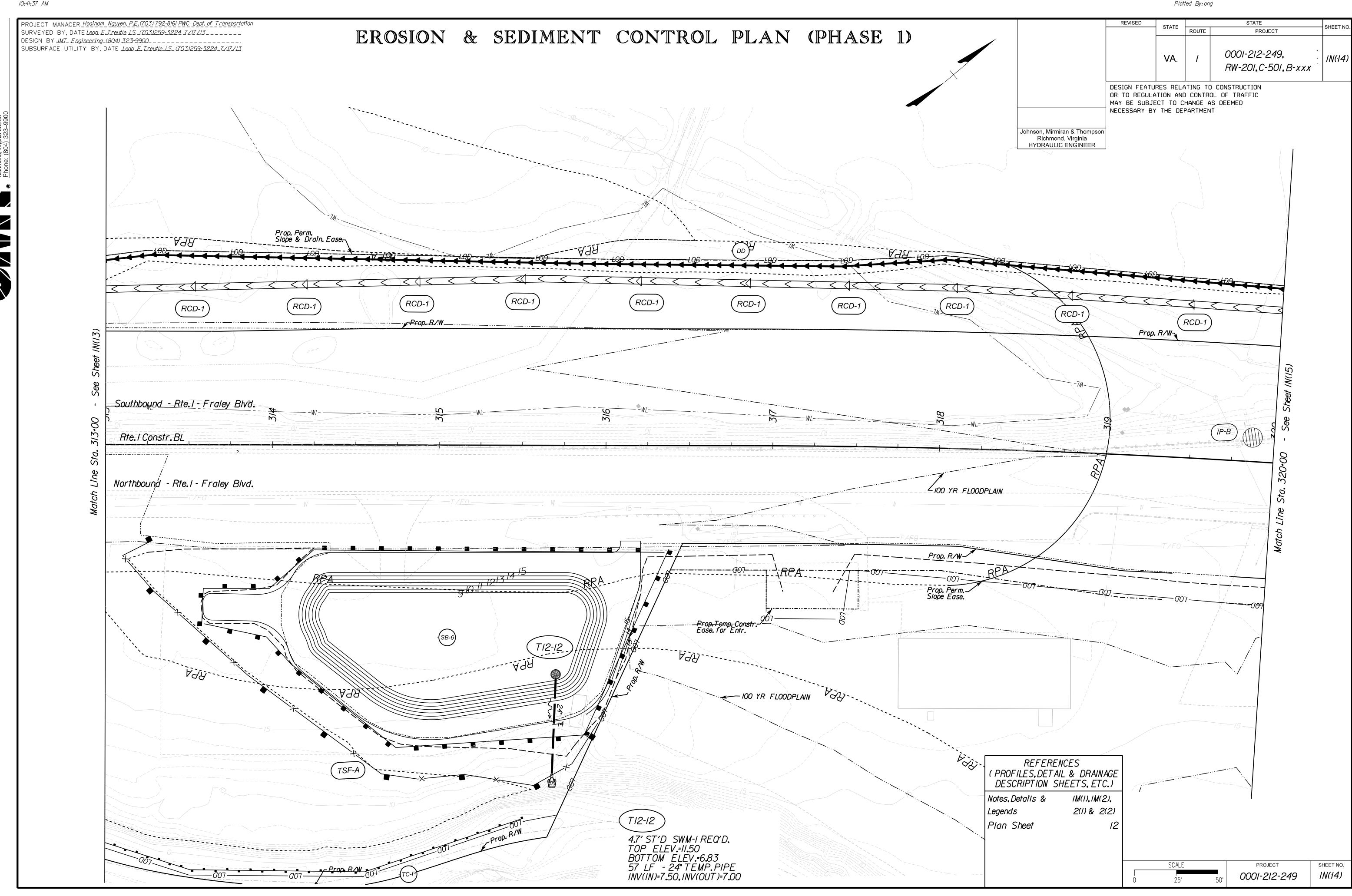


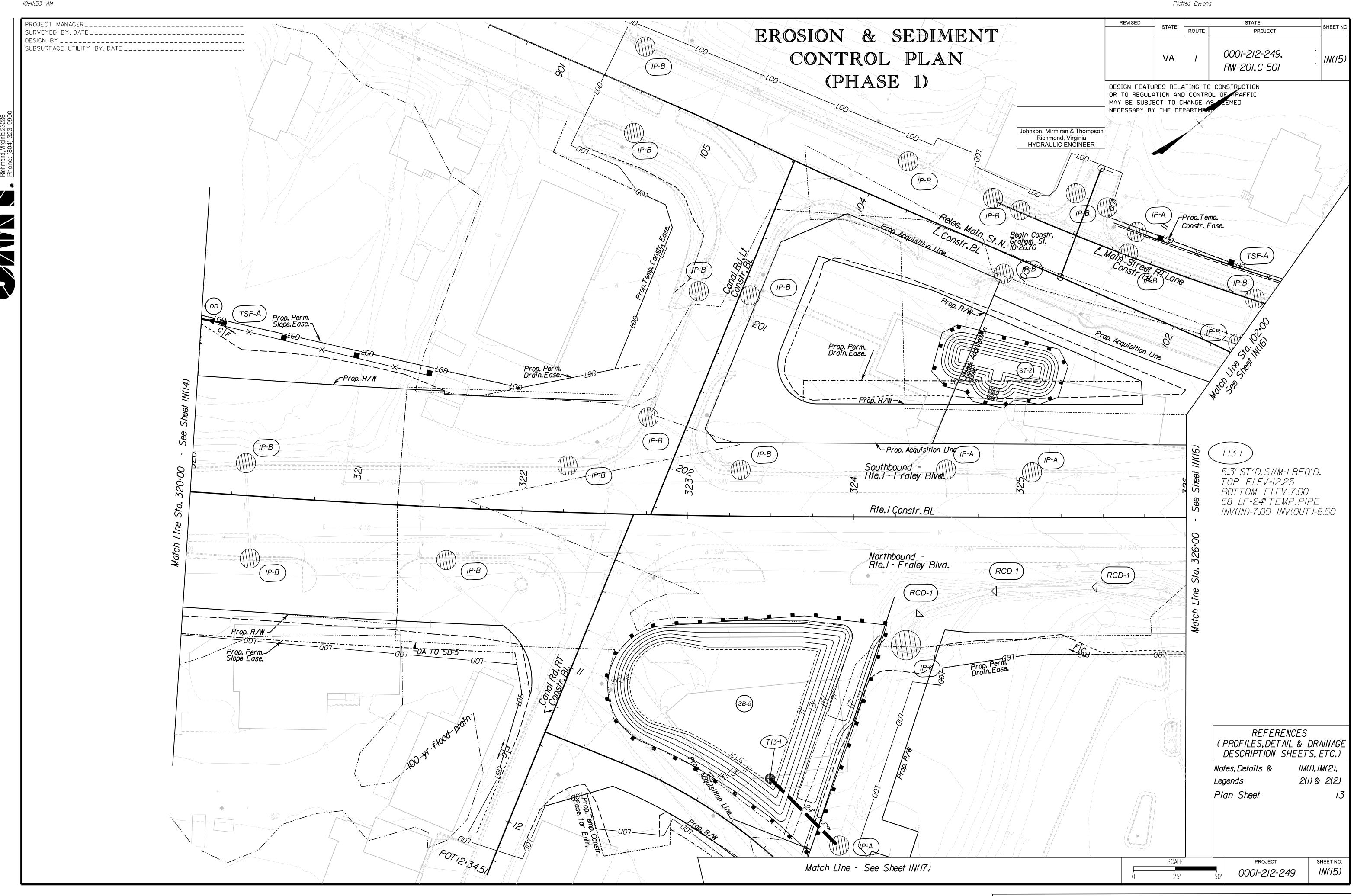
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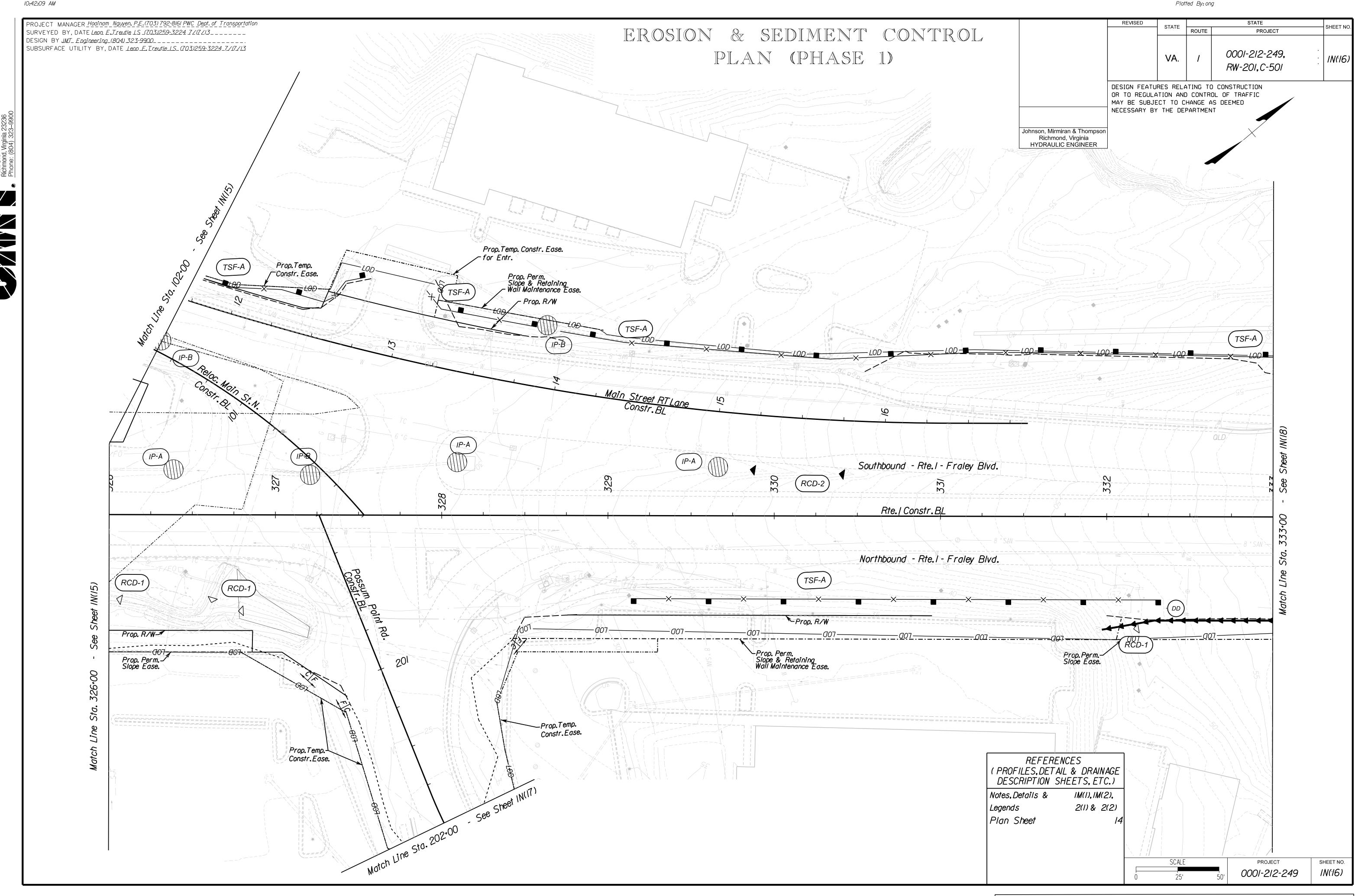


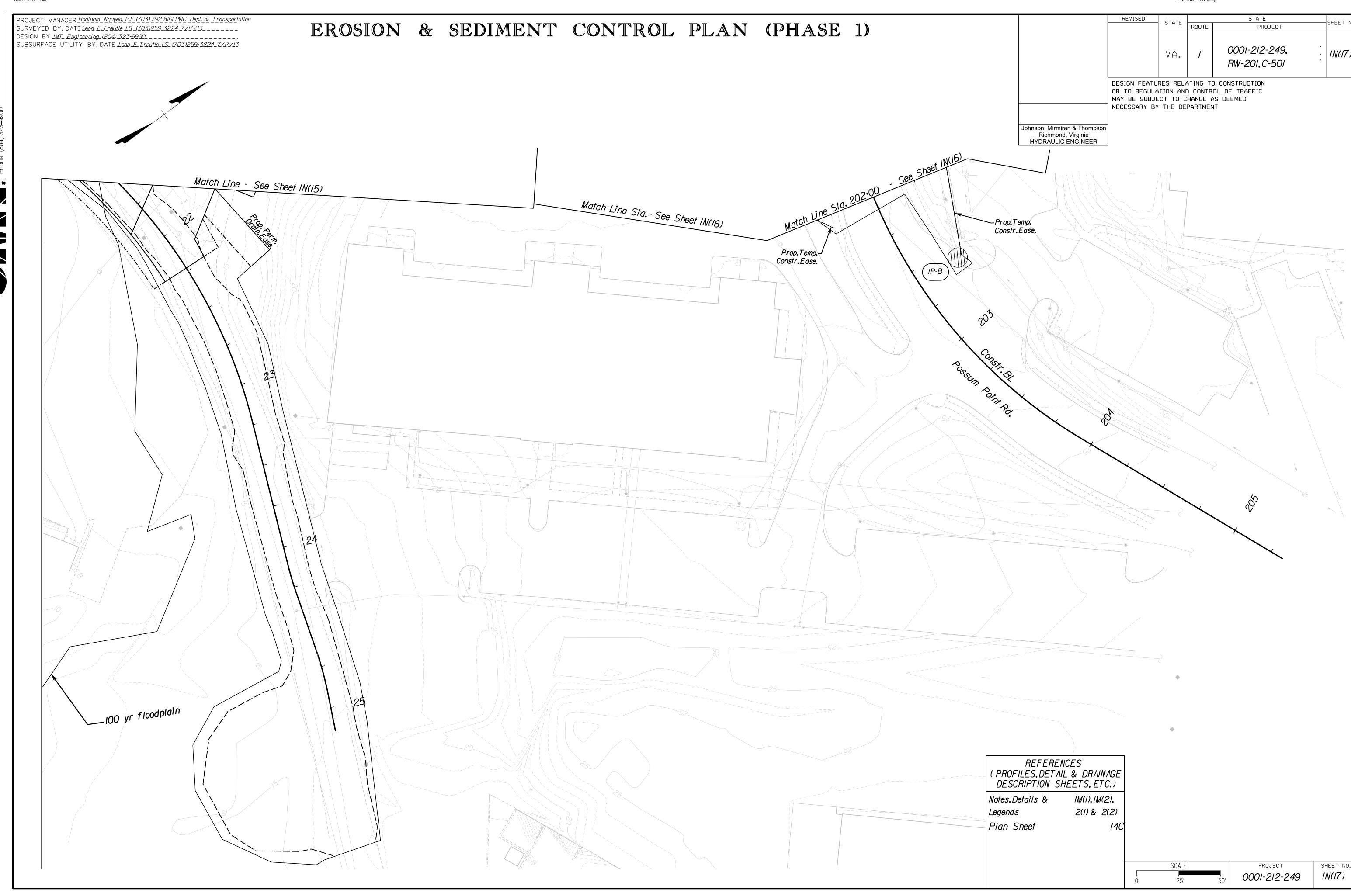
R/W PLANS

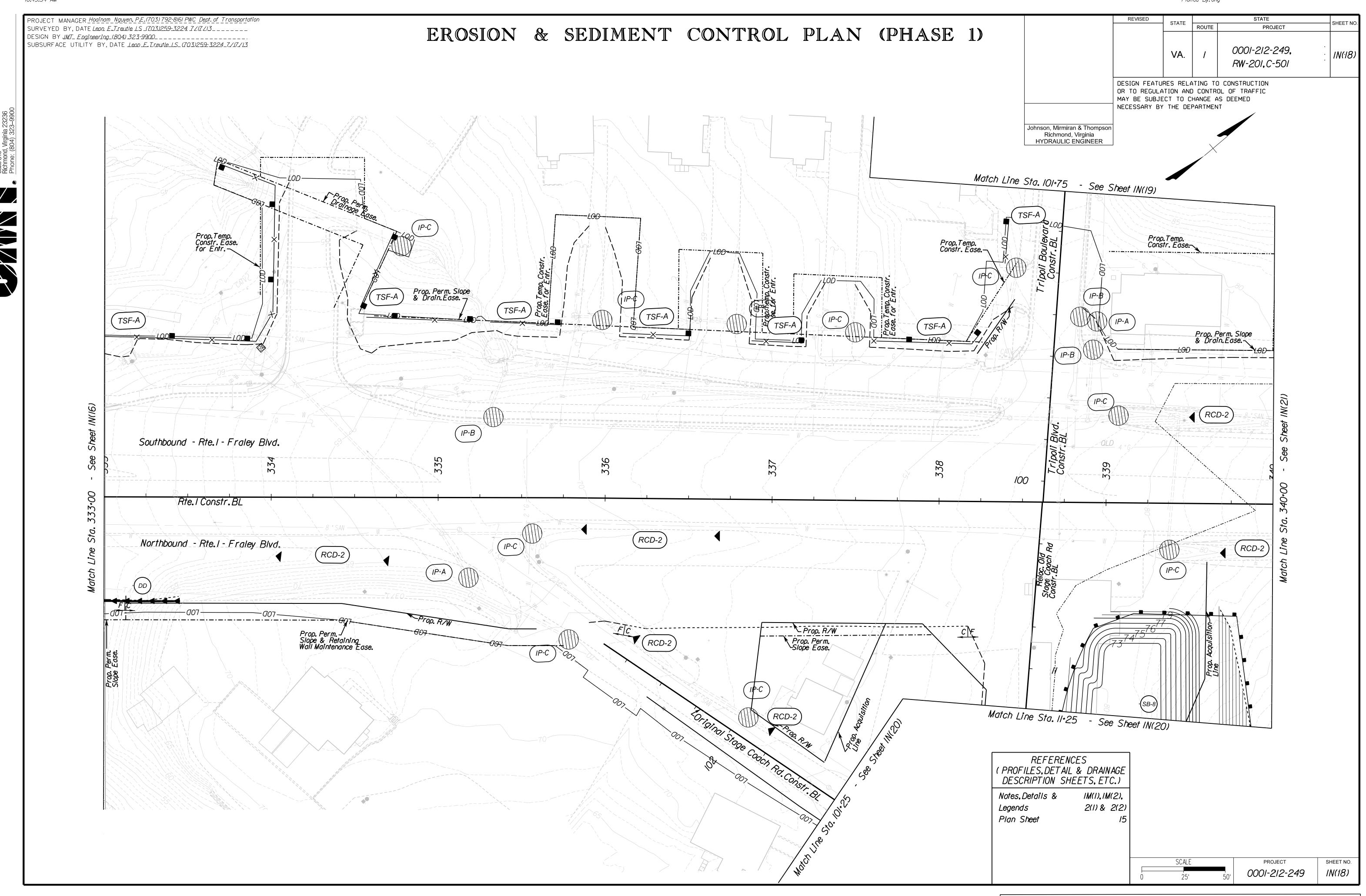


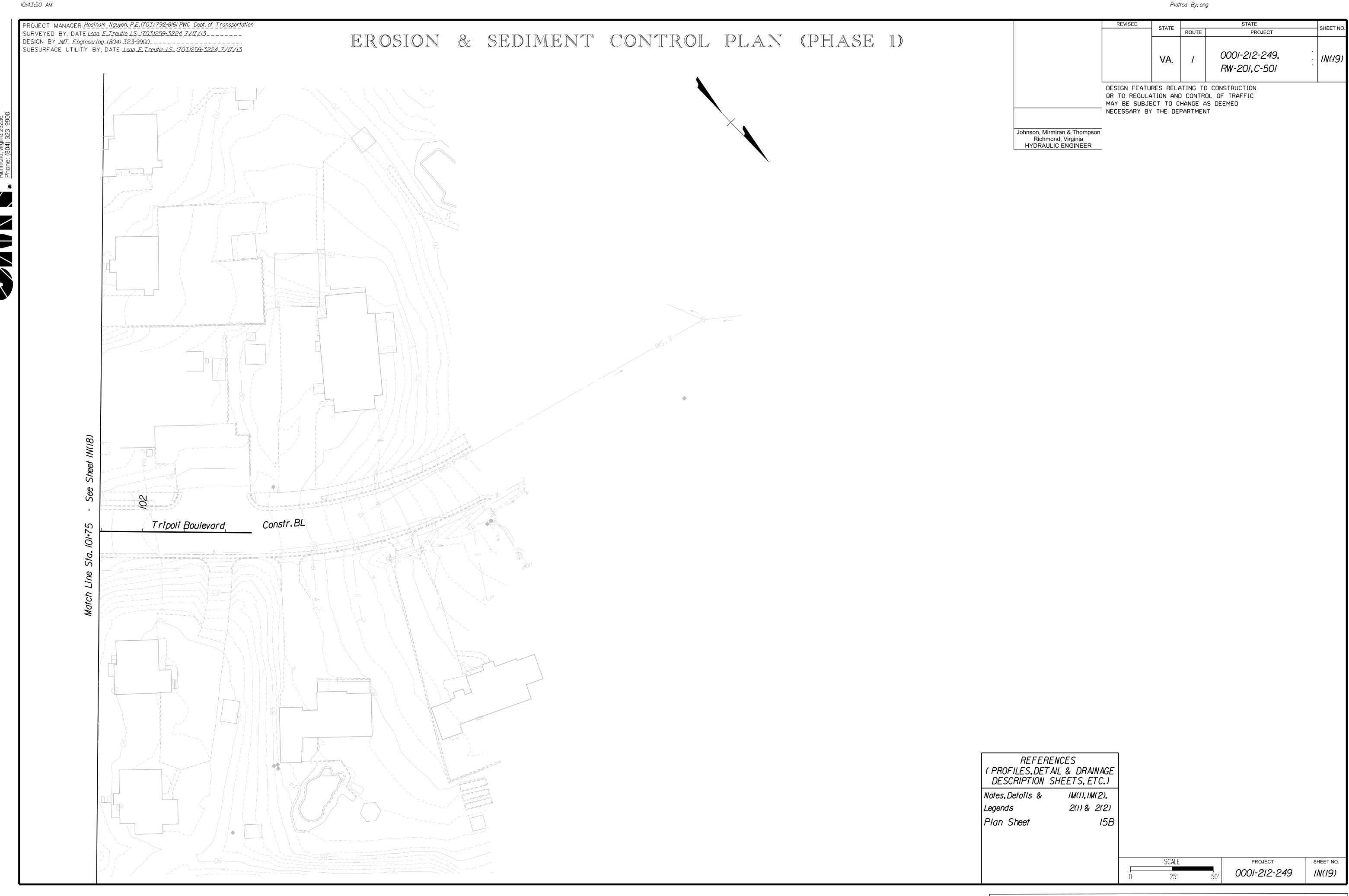


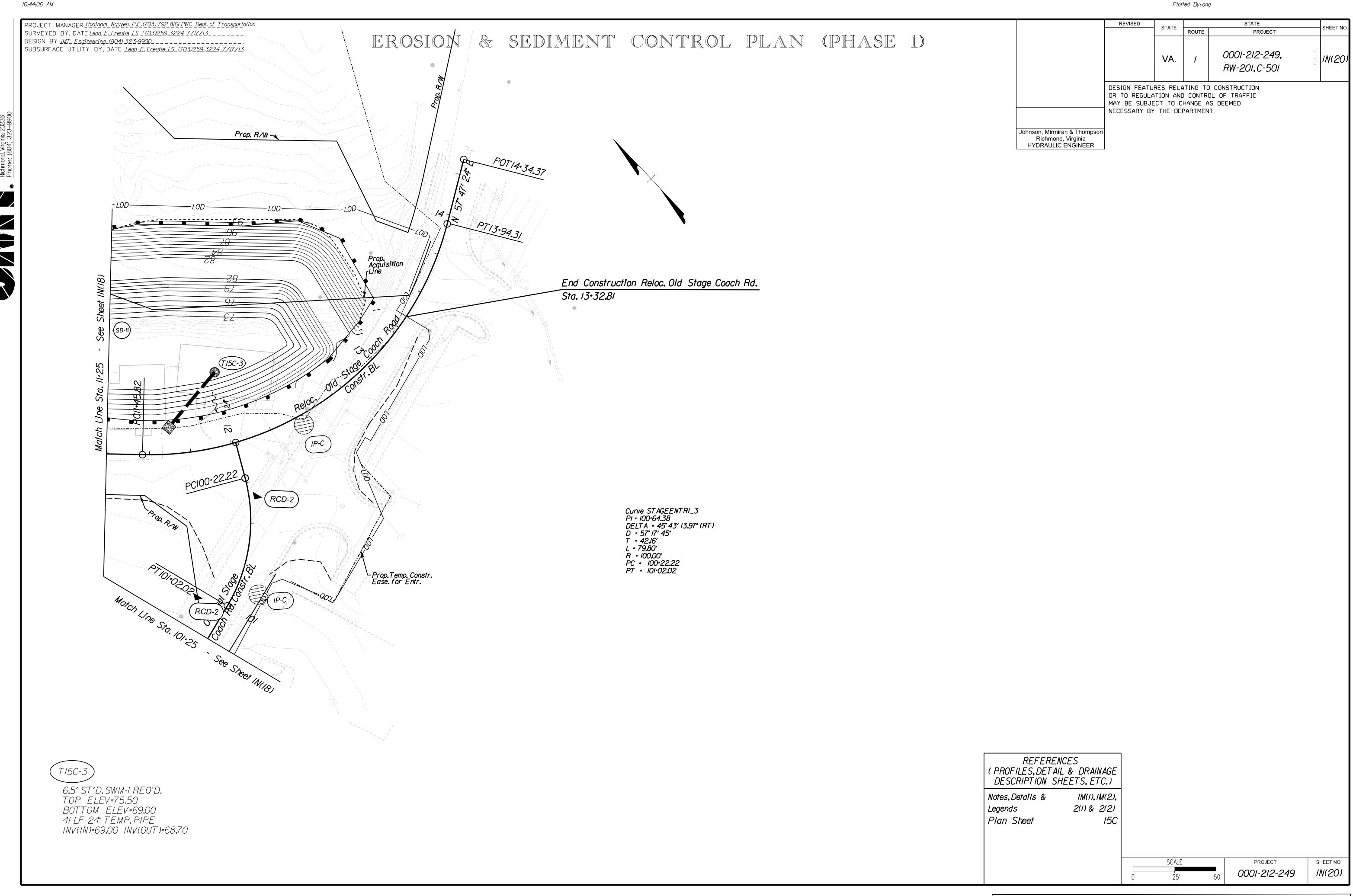


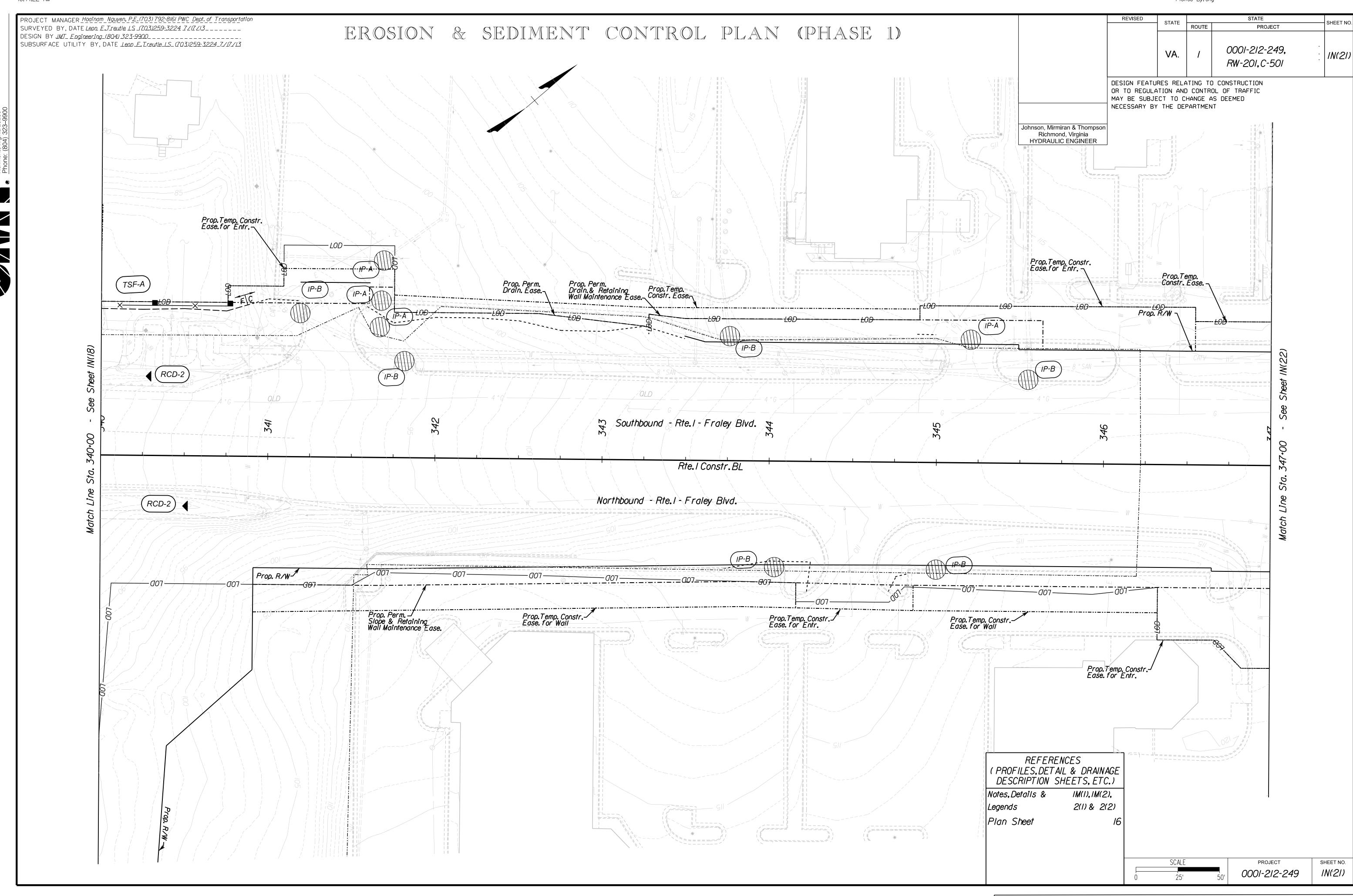


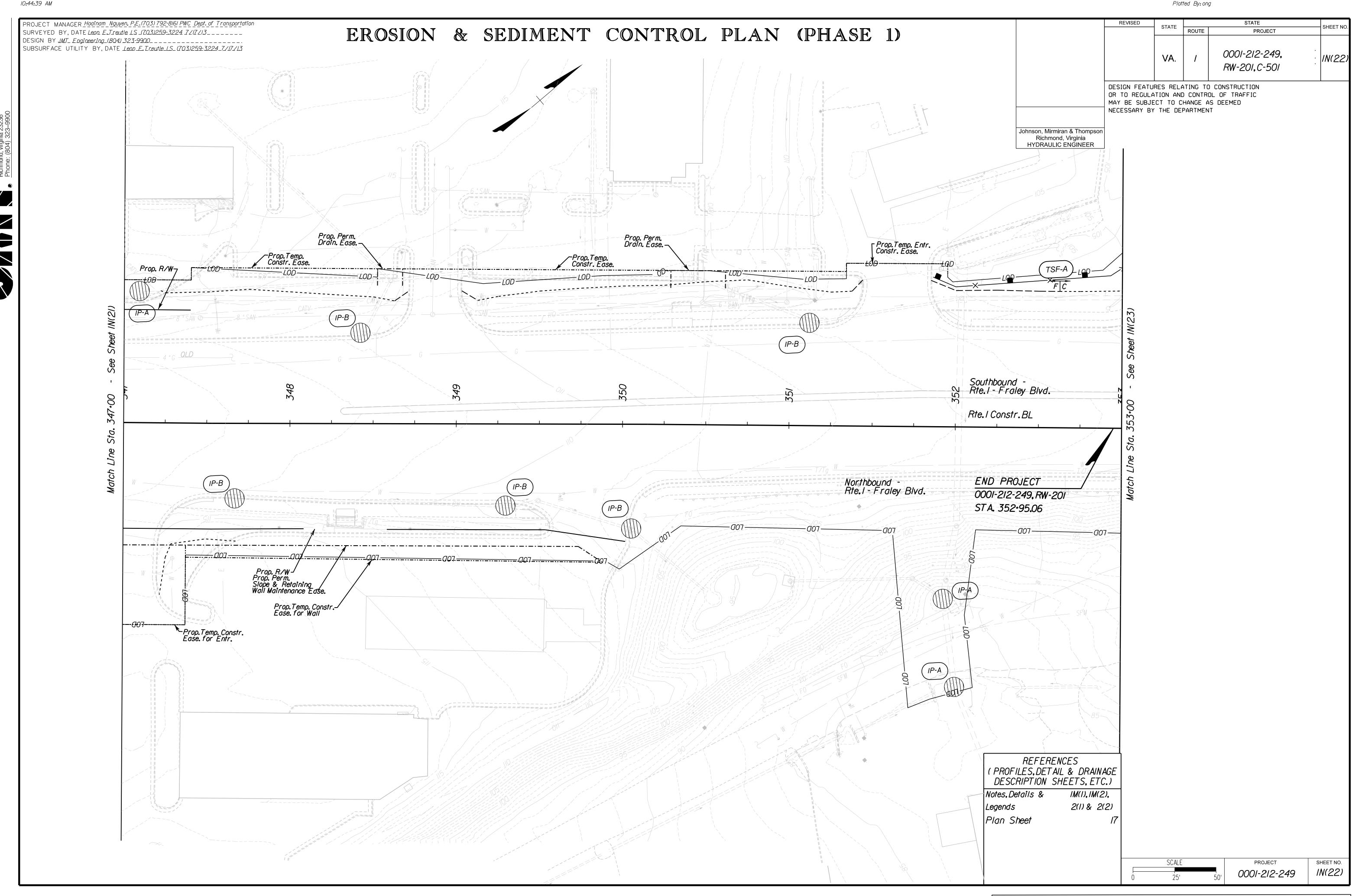


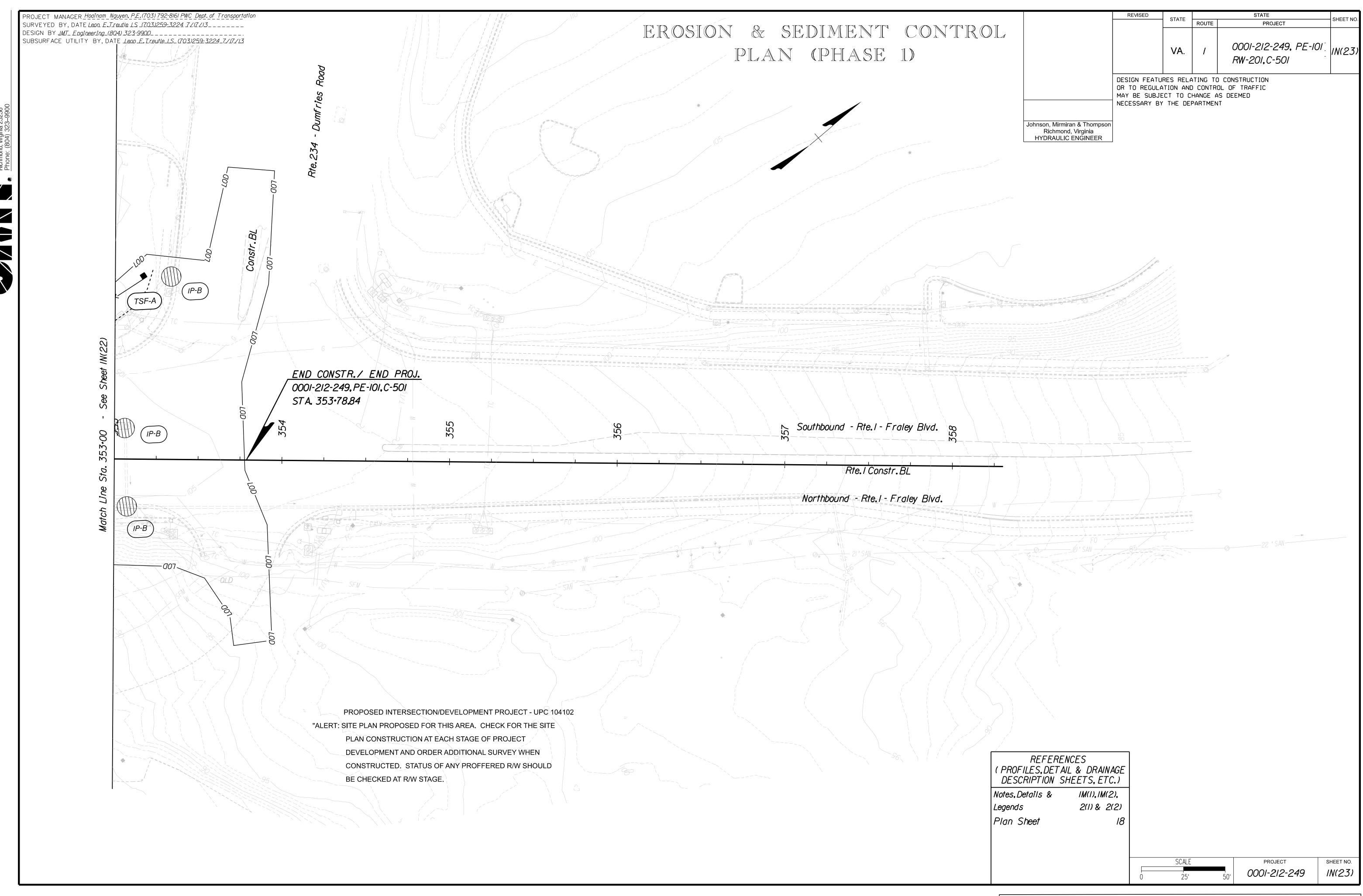












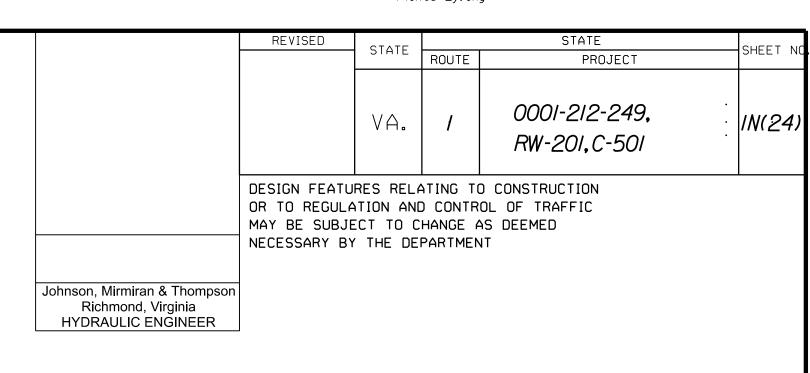
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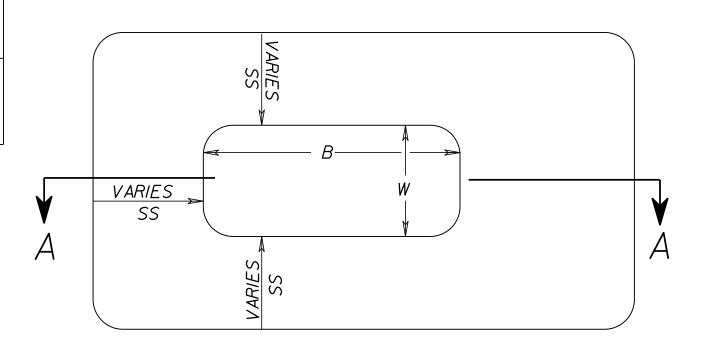
PROJECT MANAGER Hodinam Nguyen, P.E. (703) 792-8161 PWC Dept. of Transportation SURVEYED BY, DATE Leon E.Treutle LS_(703)259-3224 7/17/13______

DESIGN BY JMT_Engineering (804) 323-9900______
SUBSURFACE UTILITY BY, DATE Leon F. Treutle LS_(703)259-3224_7/17/13

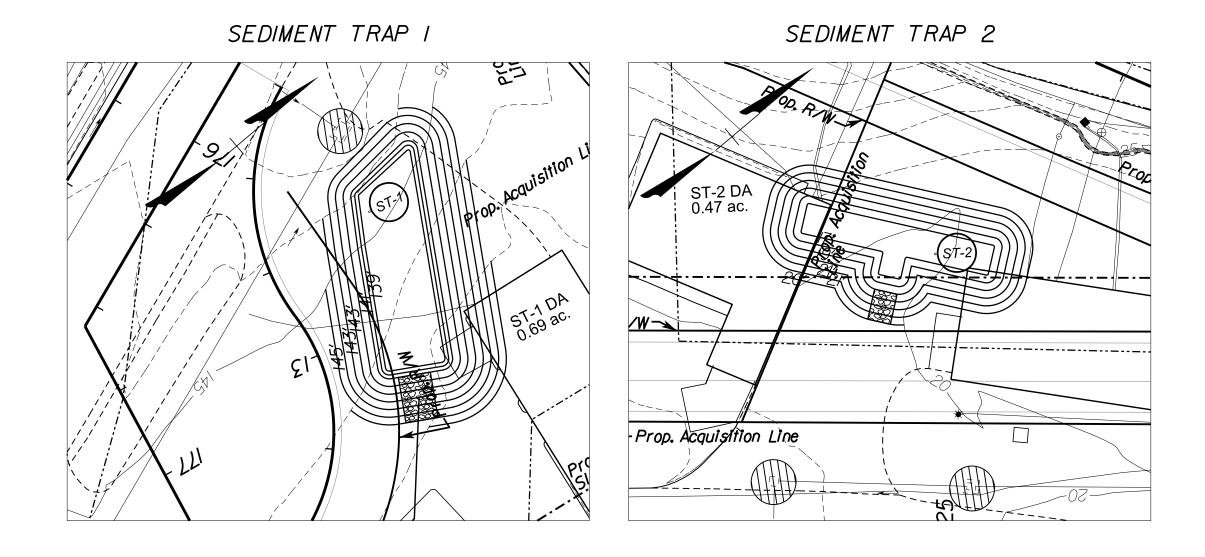
TEMPORARY SEDIMENT TRAP DETAIL SHEET

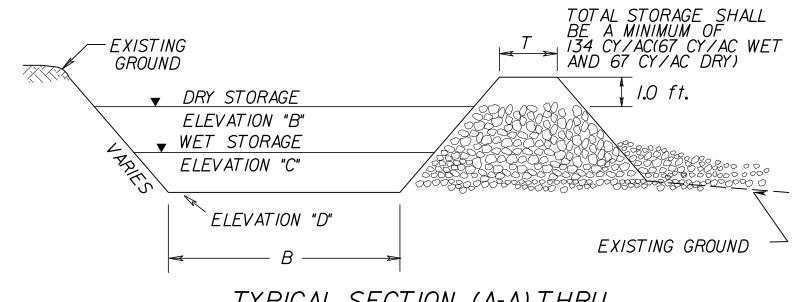
| | | | | TEMF | PORARY | SEDIME | NT TR | AP TAE | BLE | | | | |
|--------------------------------------|------------------------------|---|------------------------|--------------|--------|----------|--------------|----------|----------|------------------------------|----------------------------|--------------------|------------|
| | SEDIMENT TRAP INFORMATION | SECTION THRU TEMPORARY SEDIMENT TRAP | | WET STORAGE | | | DRY STORAGE | | | ESTIMATED QUANTITES | | | |
| | | | SIDE SLOPES | ELEV. (C) | REQ'D. | PROVIDED | ELEV. (B) | REQ'D. | PROVIDED | SEDIMENT BASIN EXCAVATION | SILT.CONTROL EXCAVATION | VDOT *I STONE F | REMARKS |
| | | | (SS) | | CU.YA | CU.YARDS | | CU.YARDS | | CU.YARDS | CU.YARDS | TONS | |
| SHEET NO. STATION NO. TRAP NO. | 3B & 4 259+00,LT ST-I | B= <u>50 FT</u> W= <u>15 FT</u> ELEV.D= <u>139.0</u> T= 2 FT | I:I (WET) 2:I (DRY) | 141.0 | 46 | 70 | 142.0 | 46 | 46 | 116 | <i>3</i> 5 | 3 | DA =0.7 AC |
| SHEET NO. STATION NO. TRAP NO. | 13 325+00,LT ST-2 | B= 50 FT W= 8 - 14 FT ELEV.D= 17.0 T= 2 FT | 2:/ | 19.0 | 3/ | 52 | 20.0 | 31 | 42 | 94 | 26 | 3 | DA =0.5 AC |



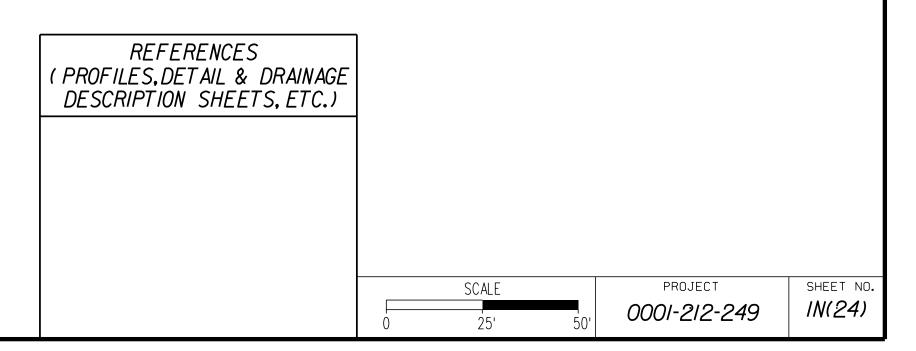


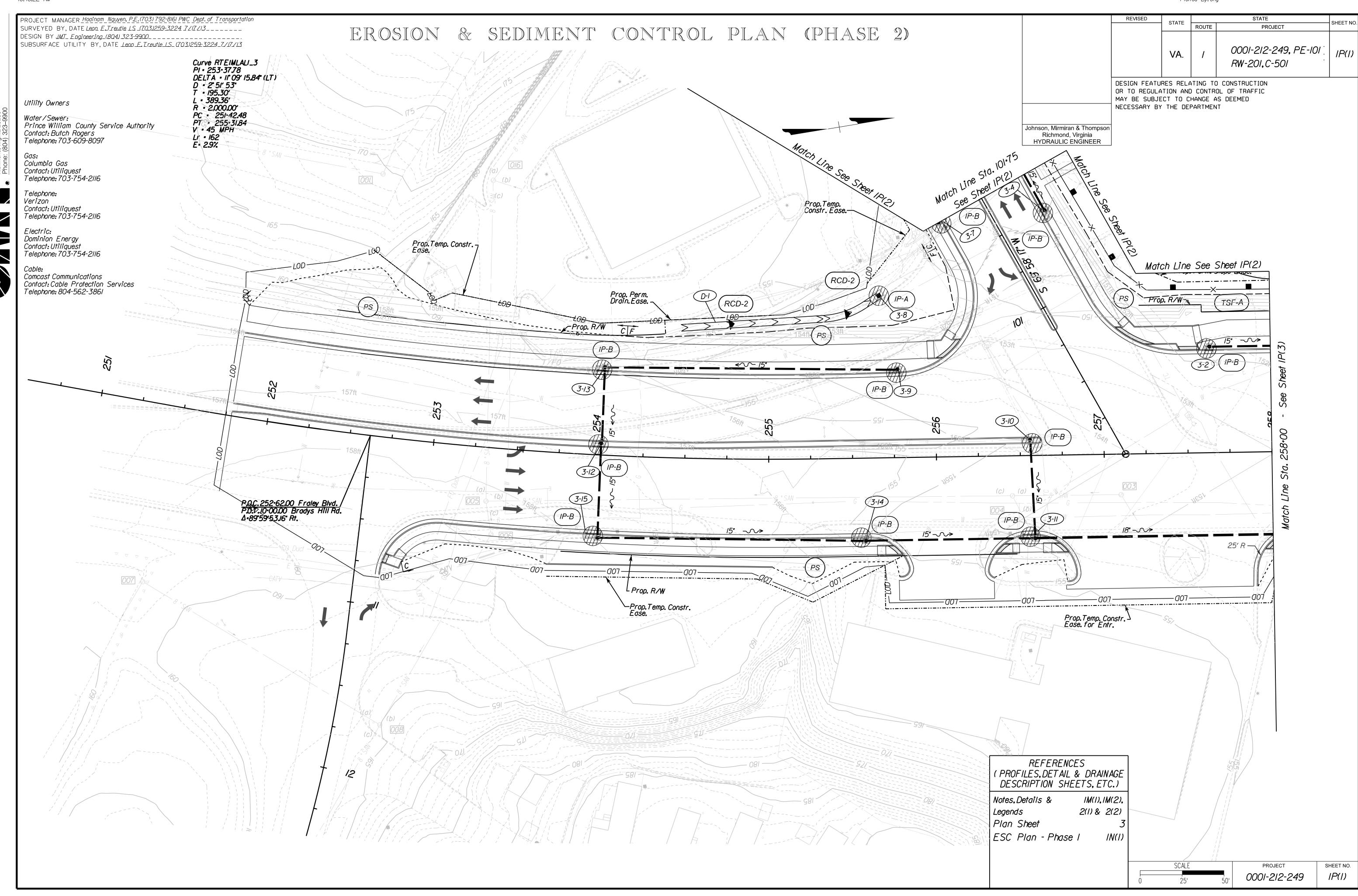
PLAN VIEW OF TEMPORARY SEDIMENT TRAP

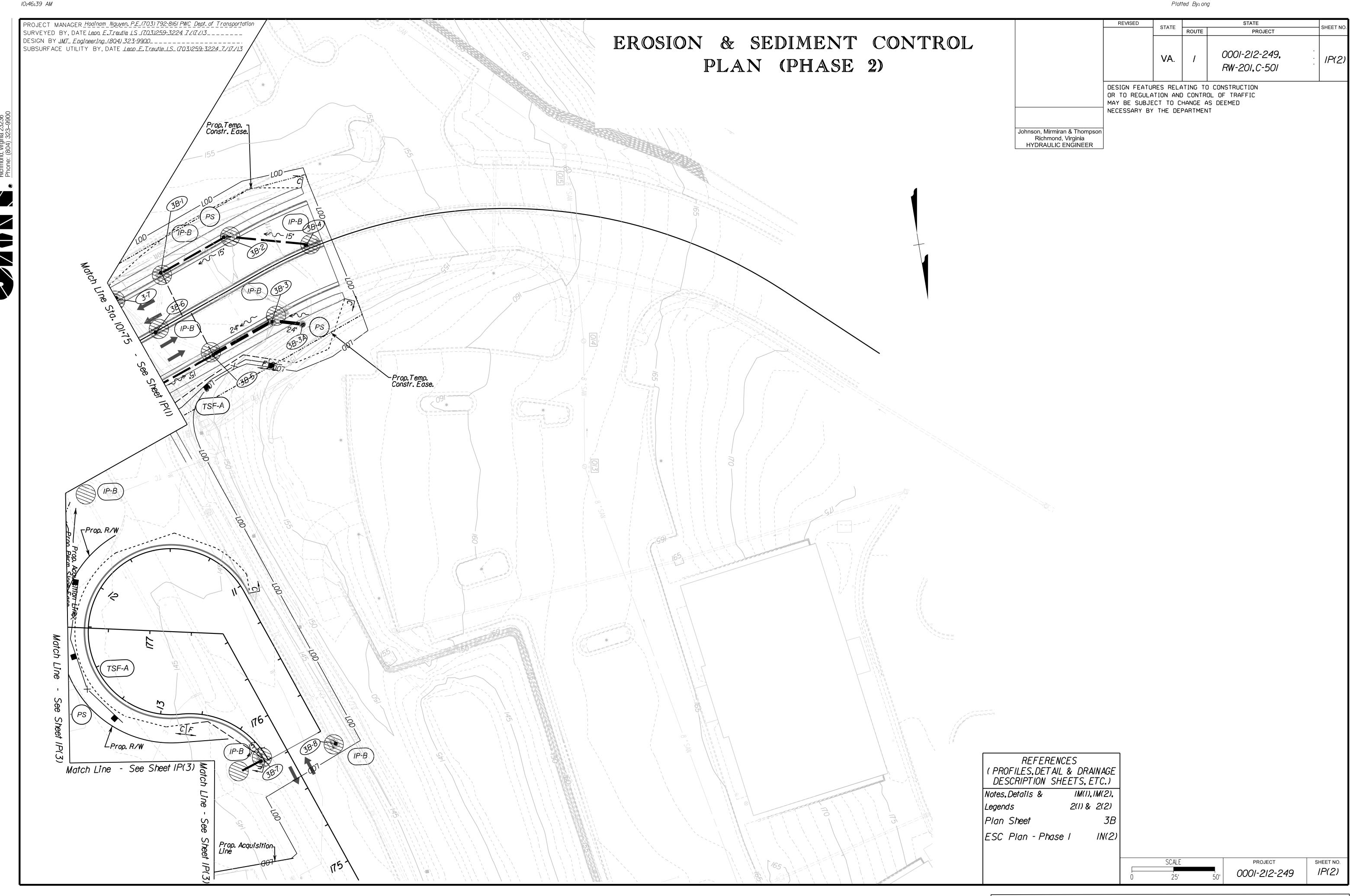


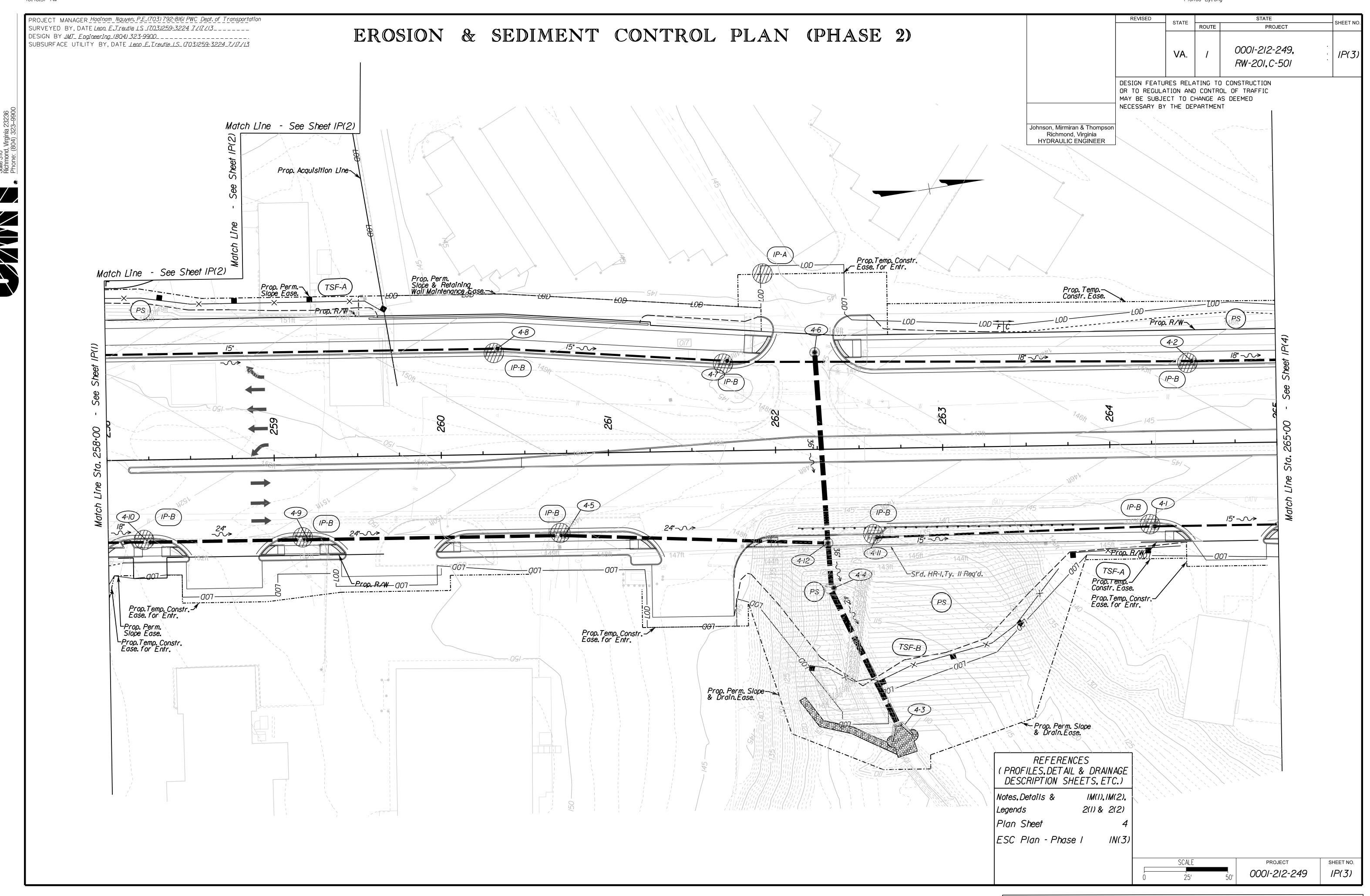


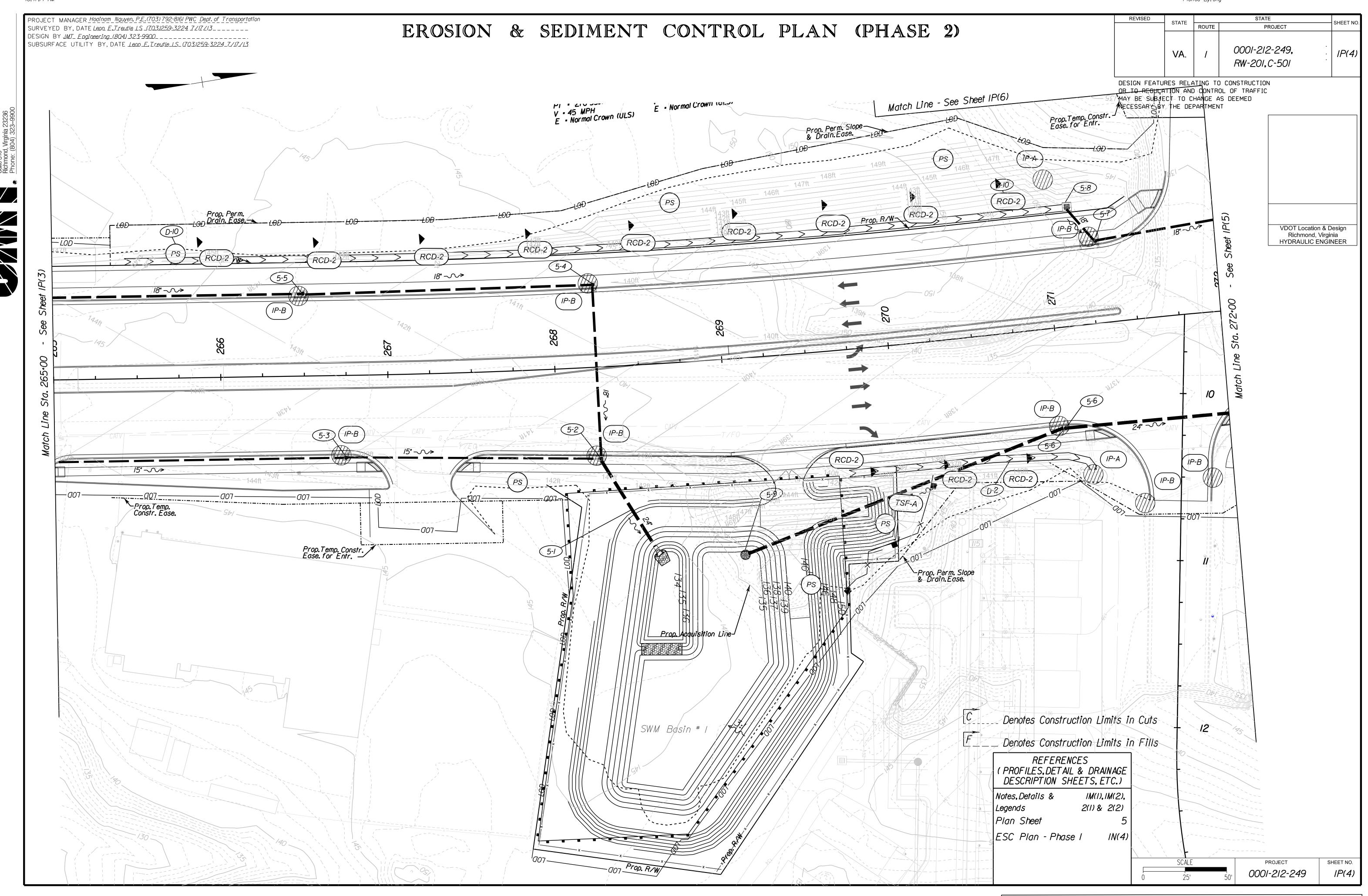
TYPICAL SECTION (A-A)THRU TEMPORARY SEDIMENT TRAP

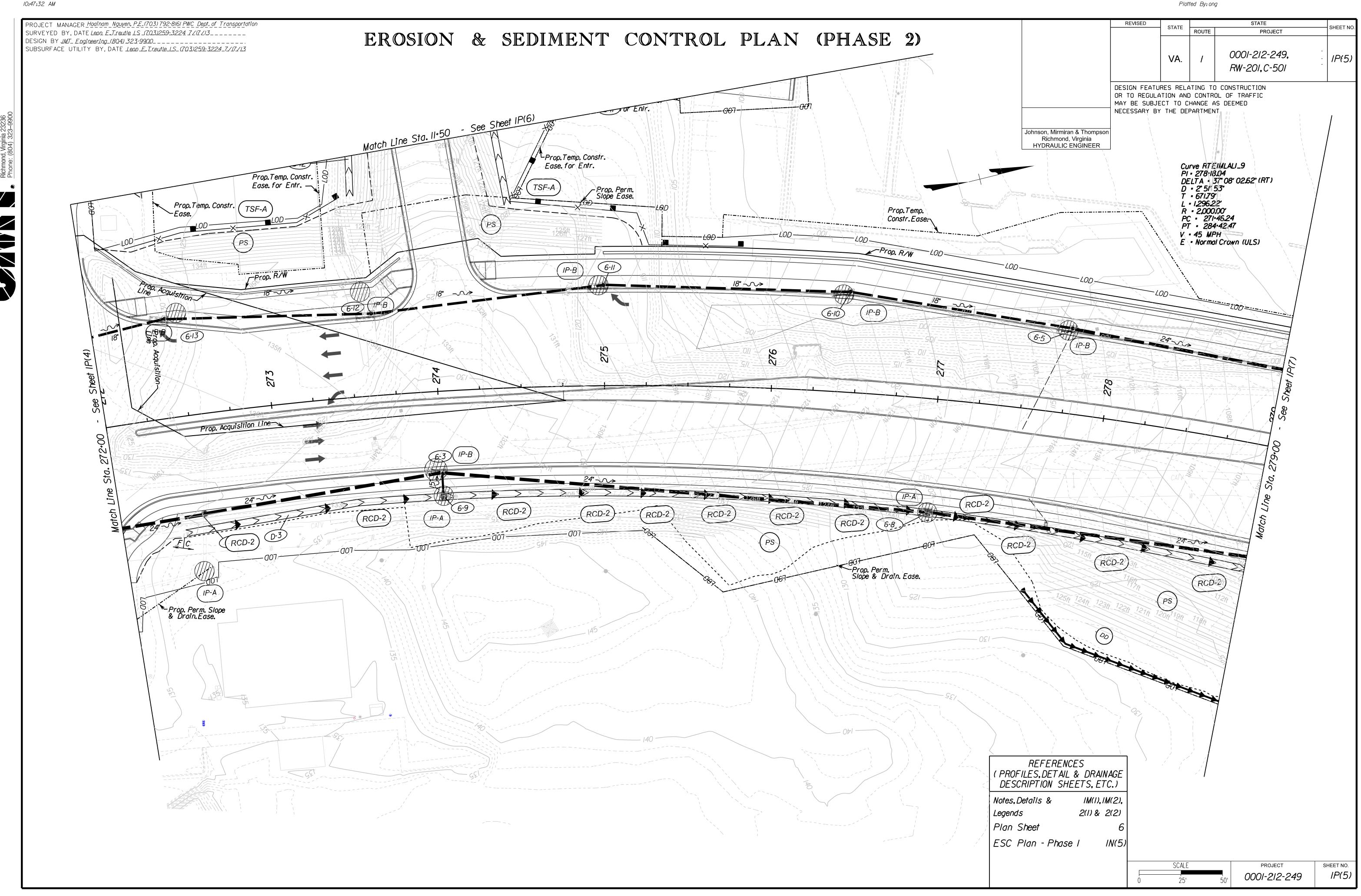


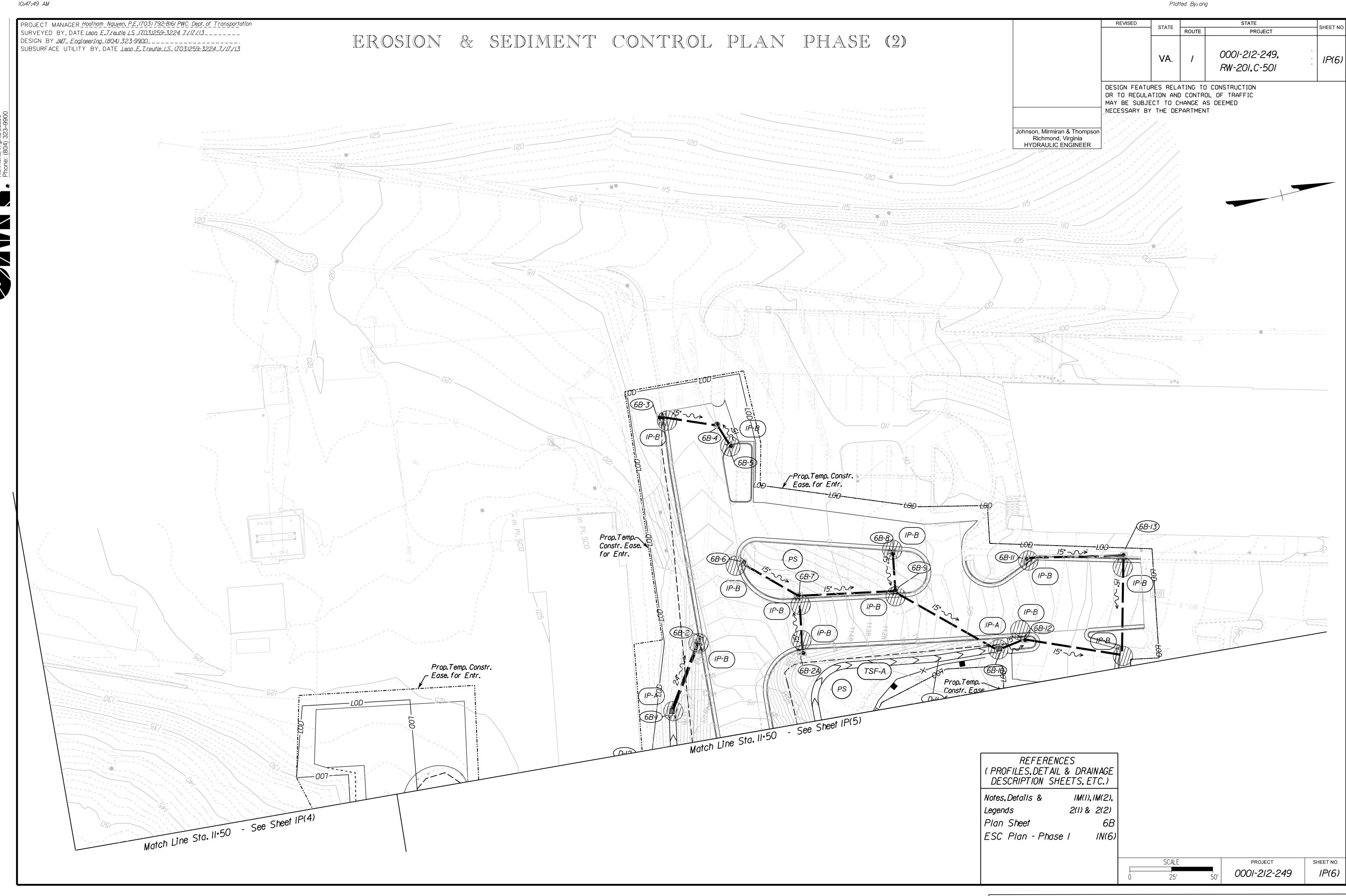


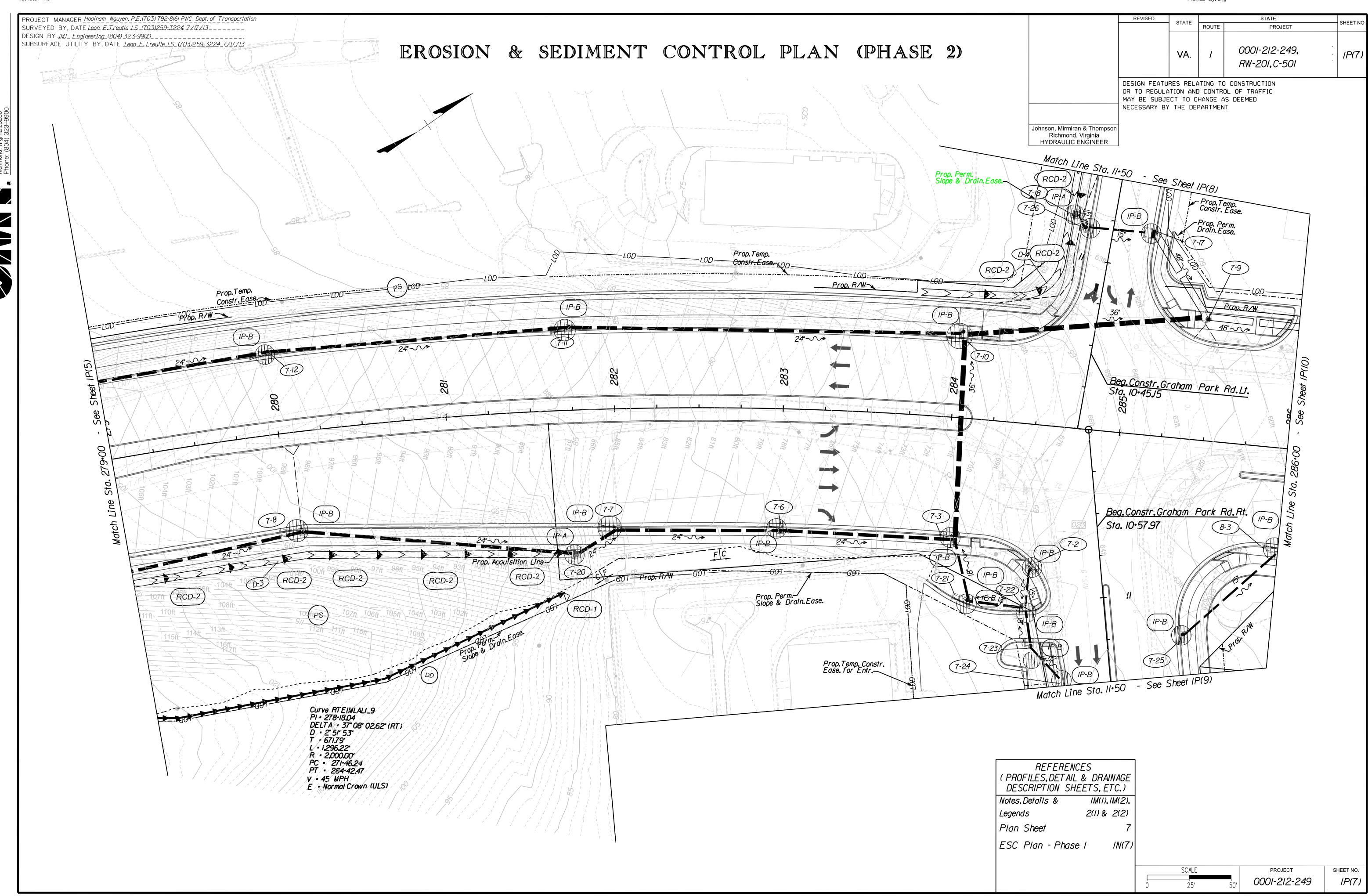




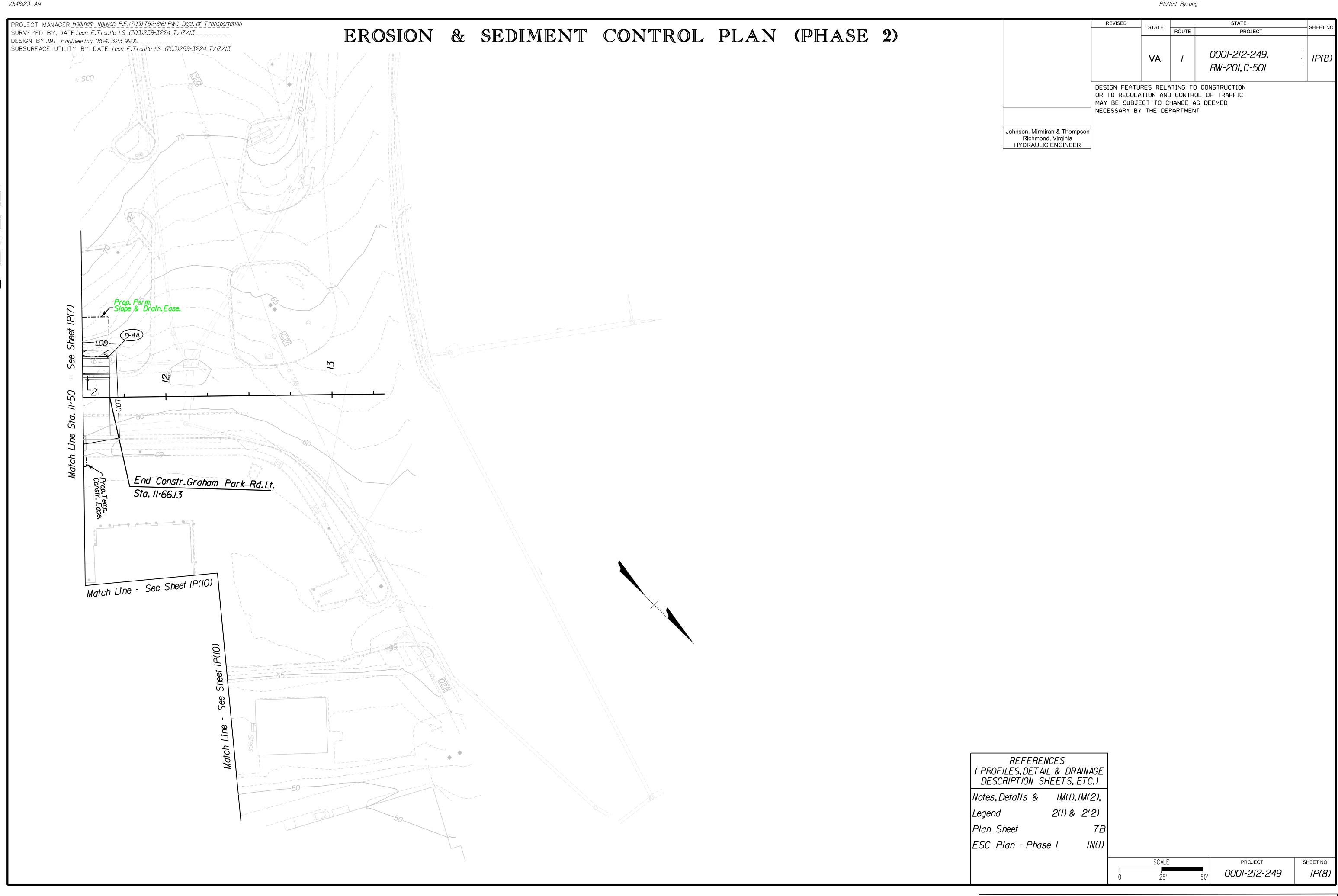


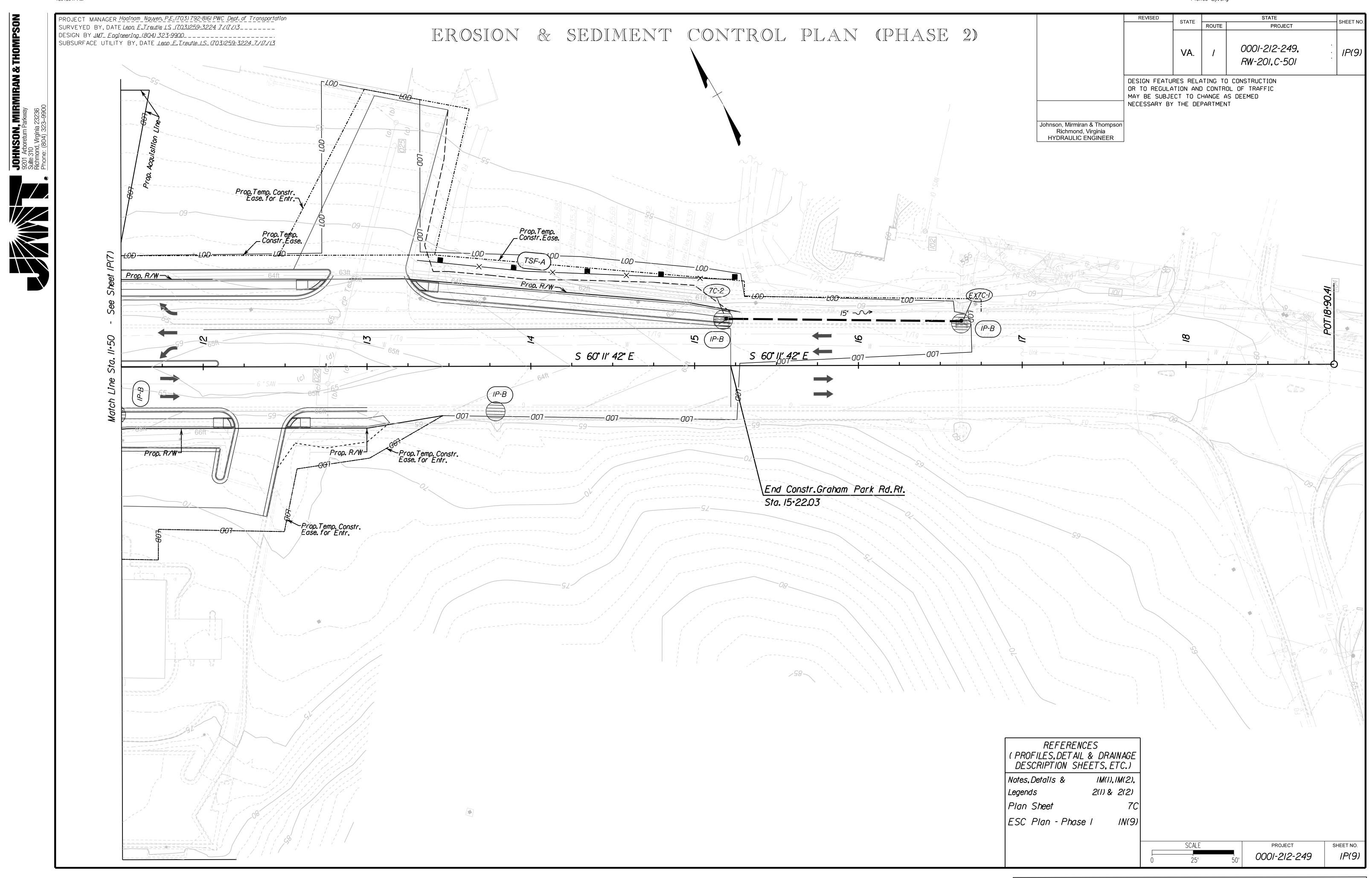


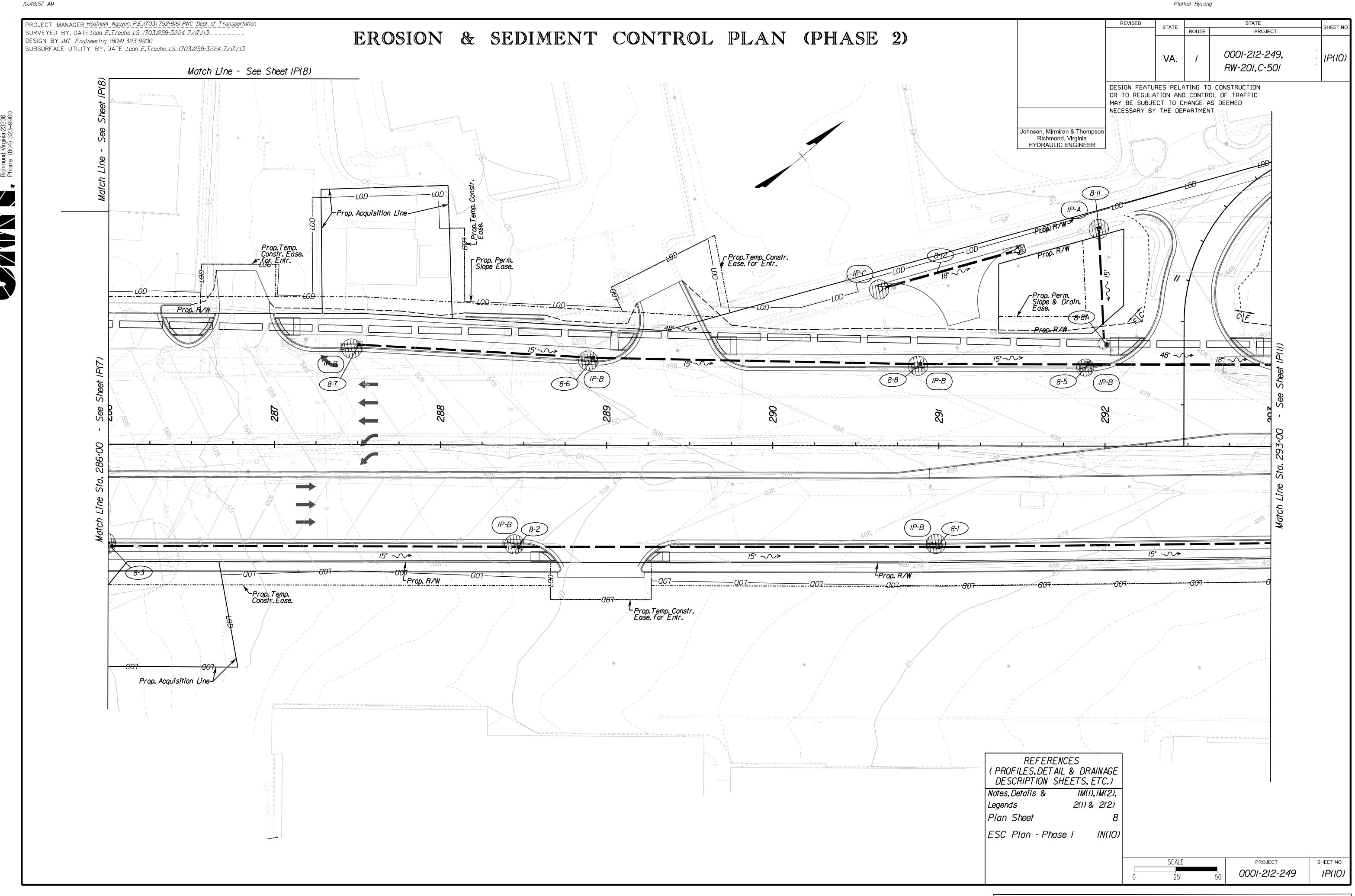


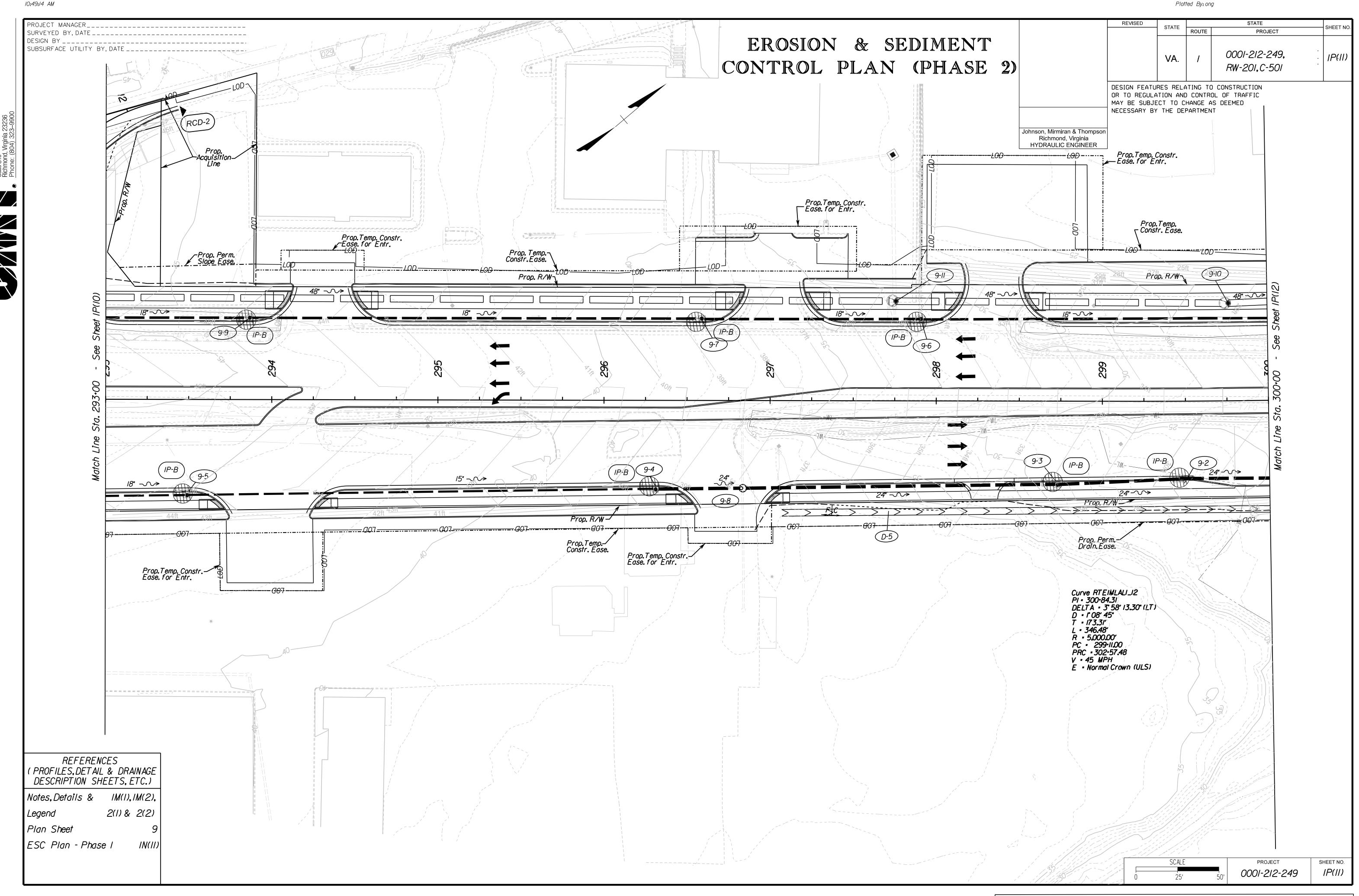


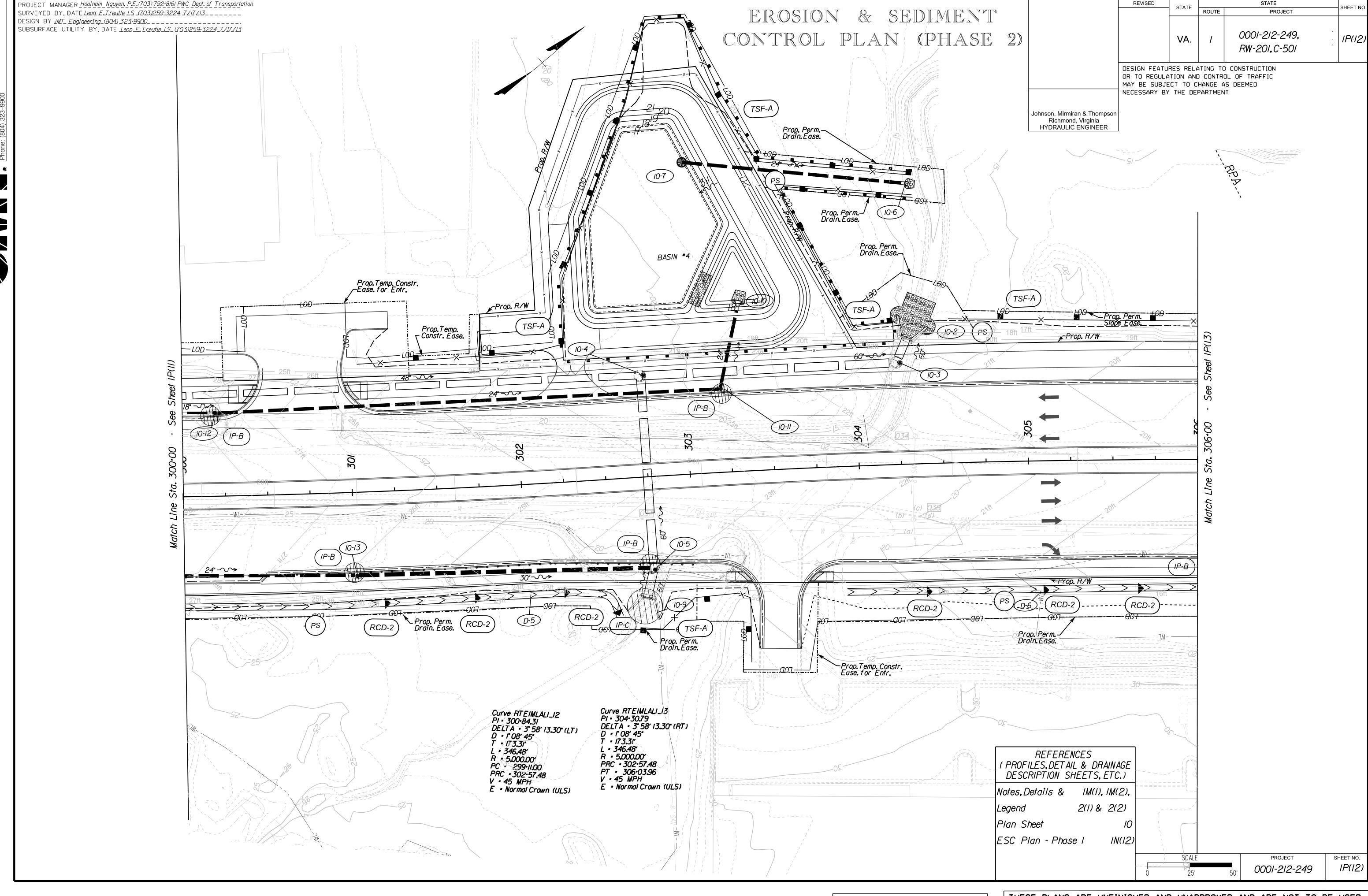
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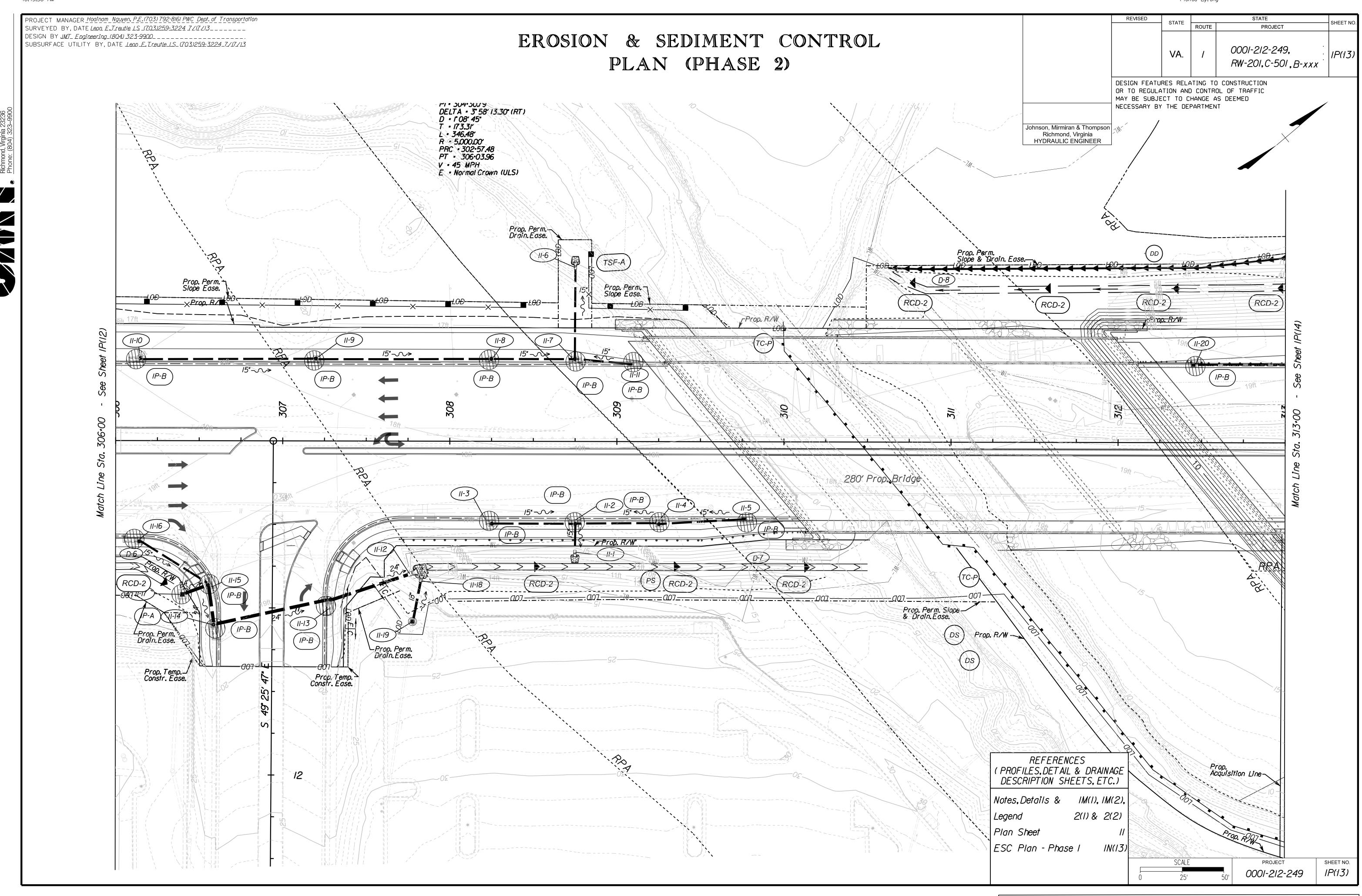


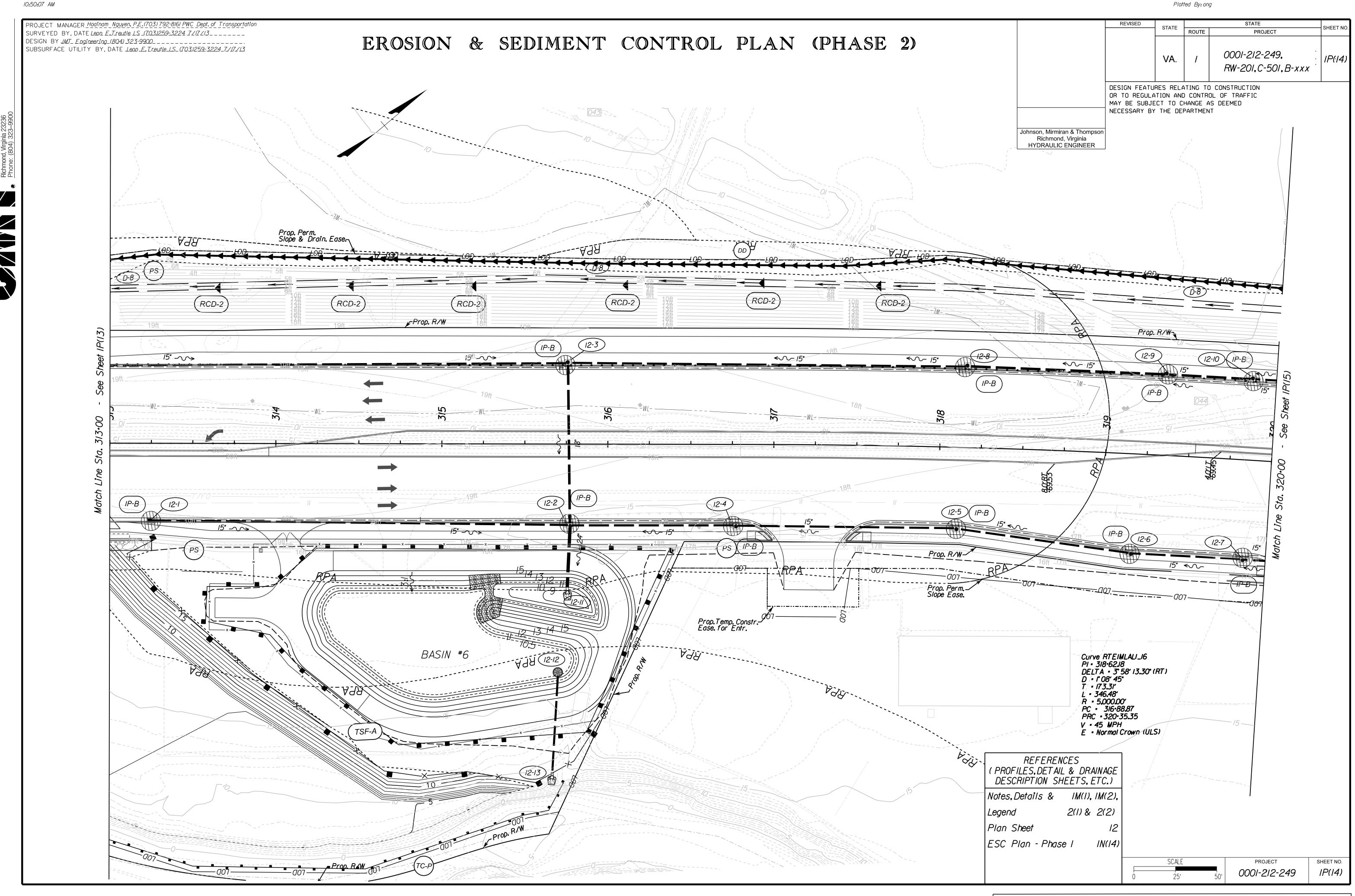


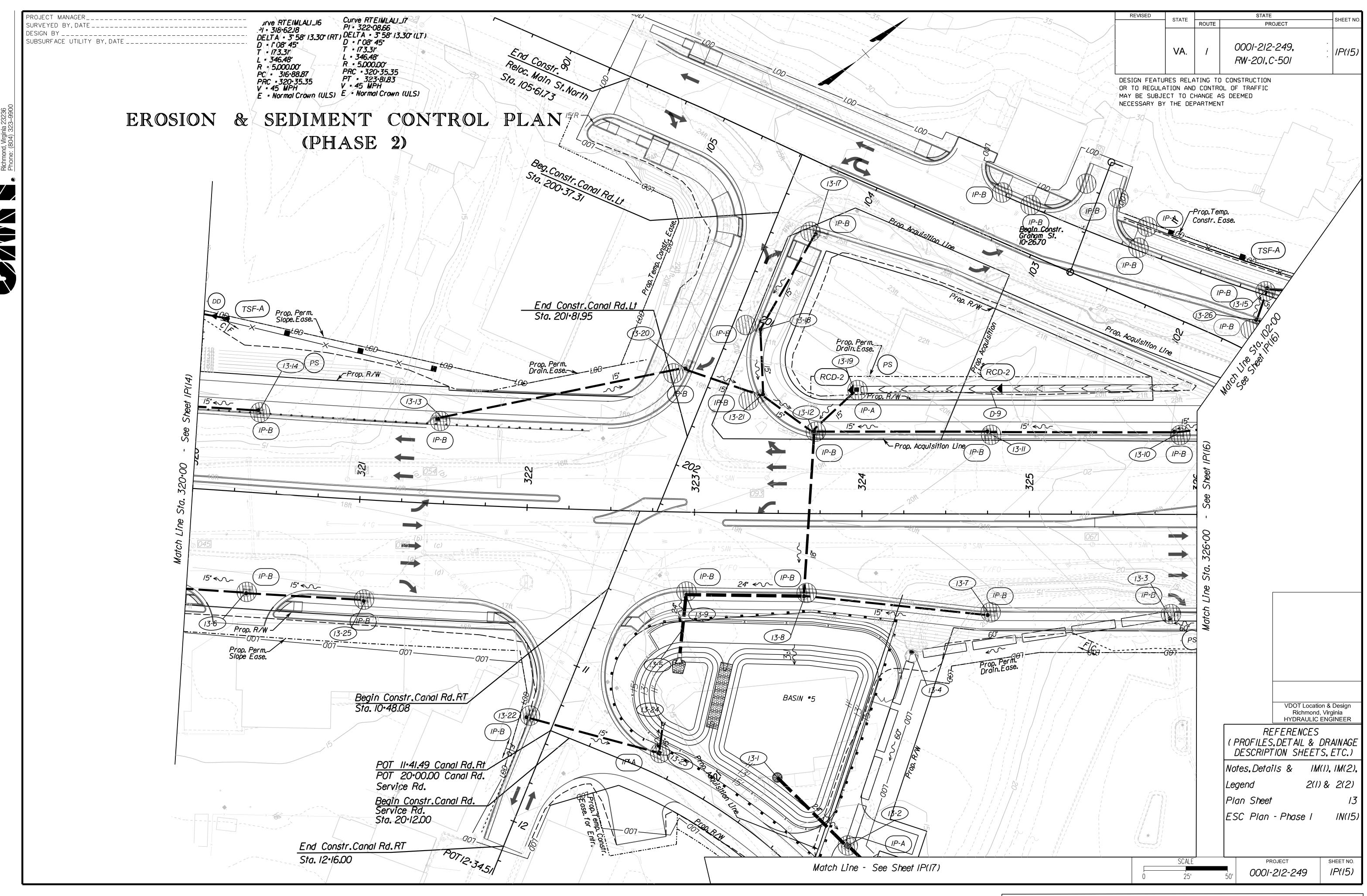


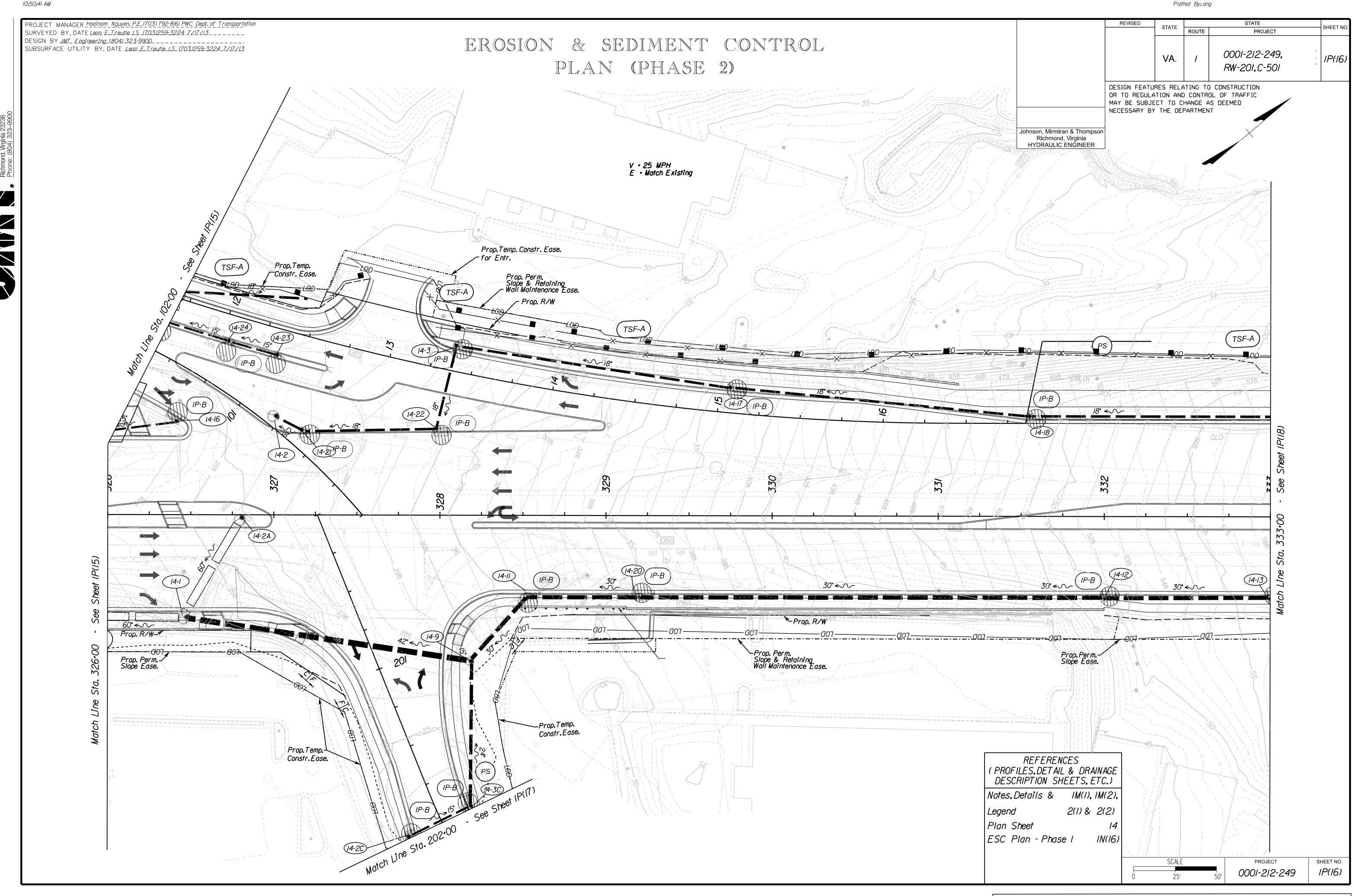


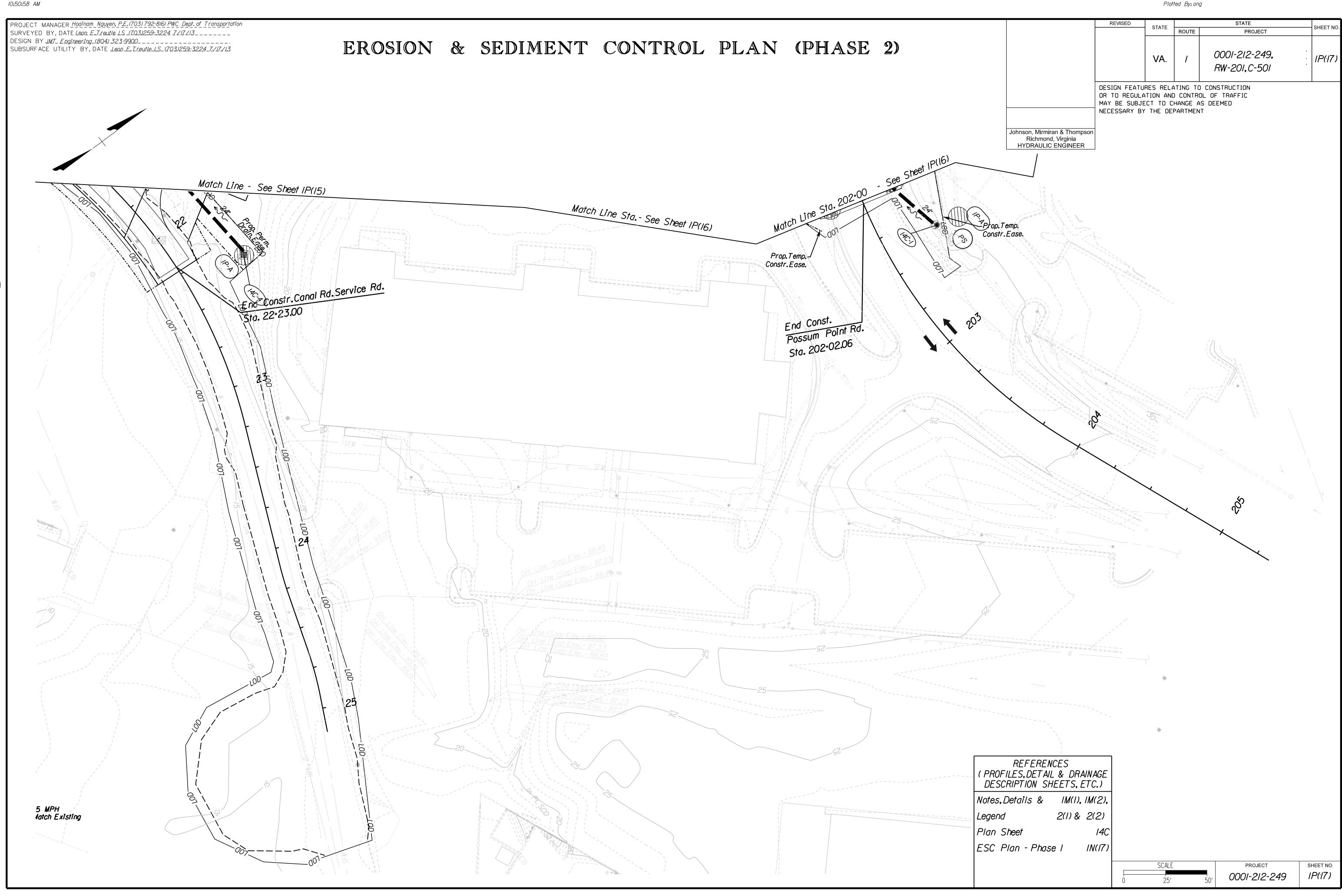


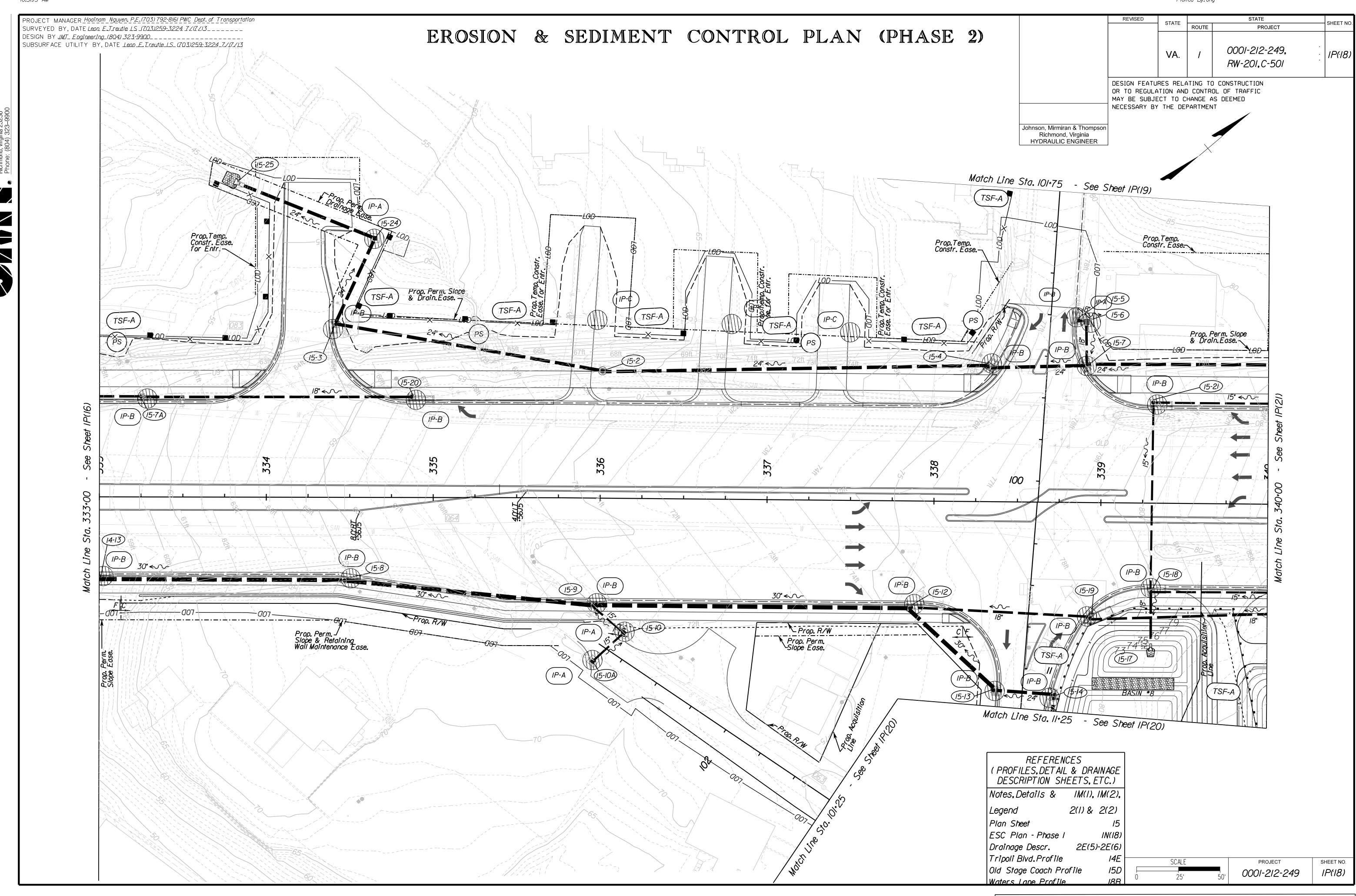


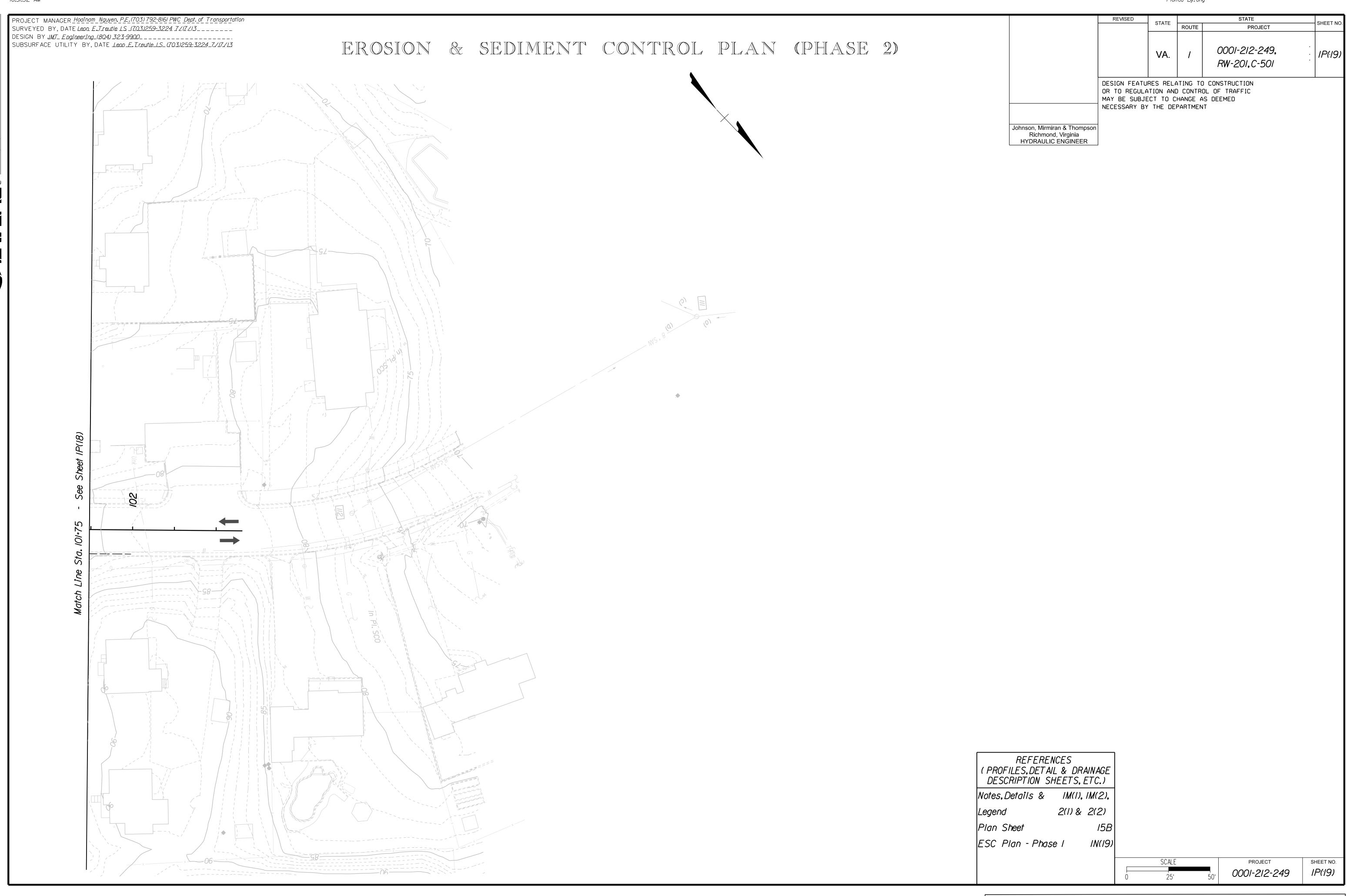




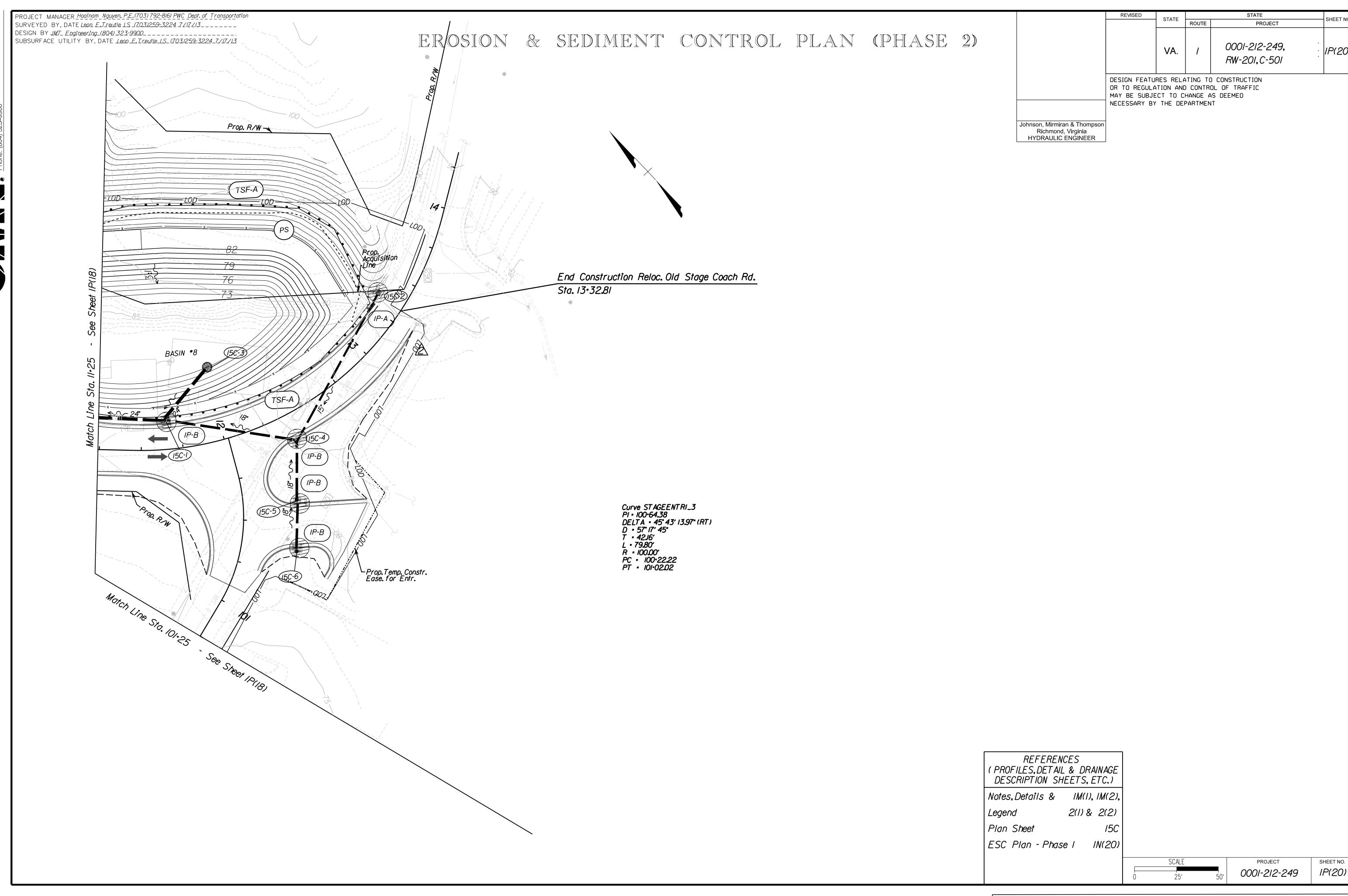


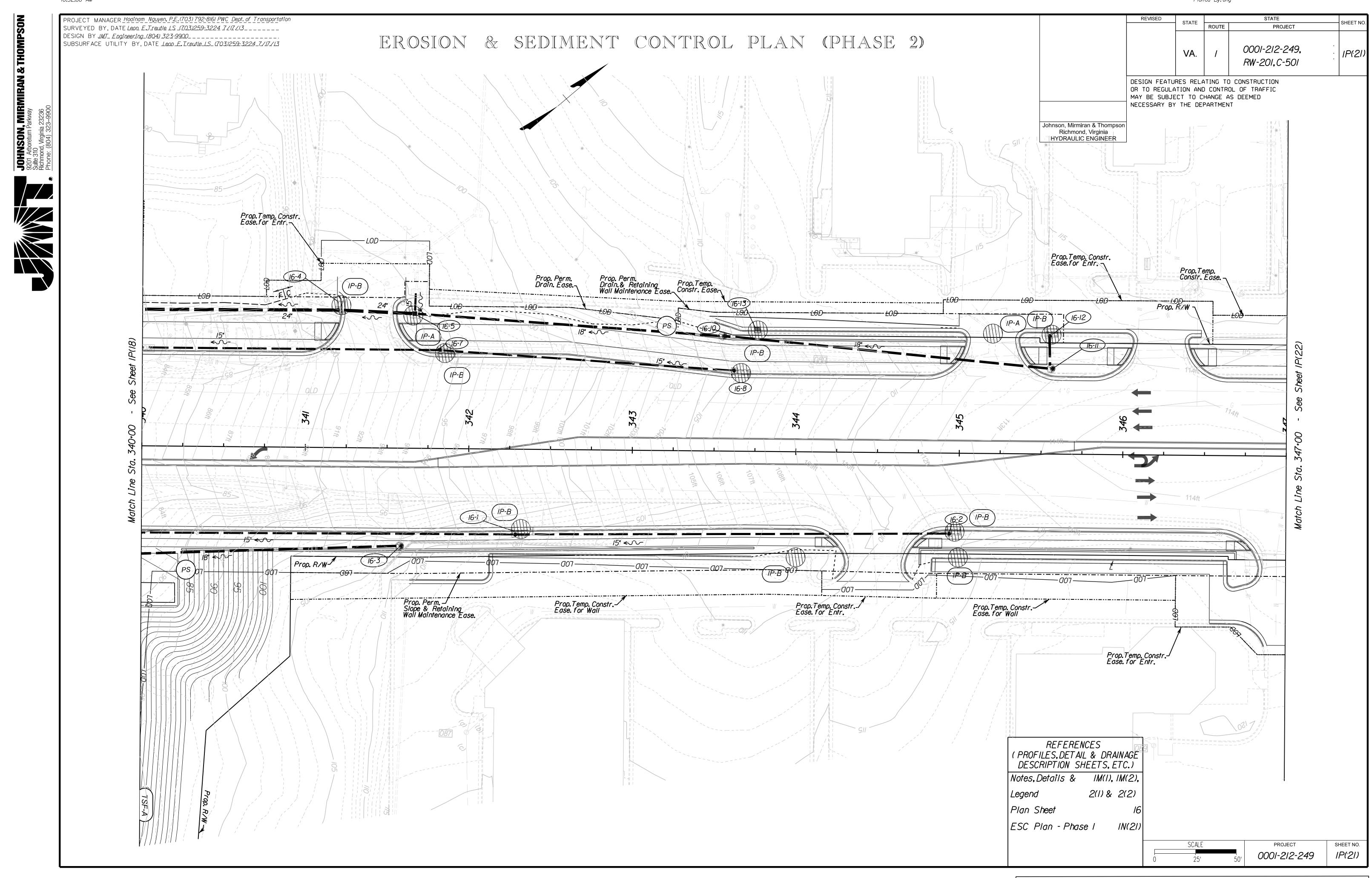


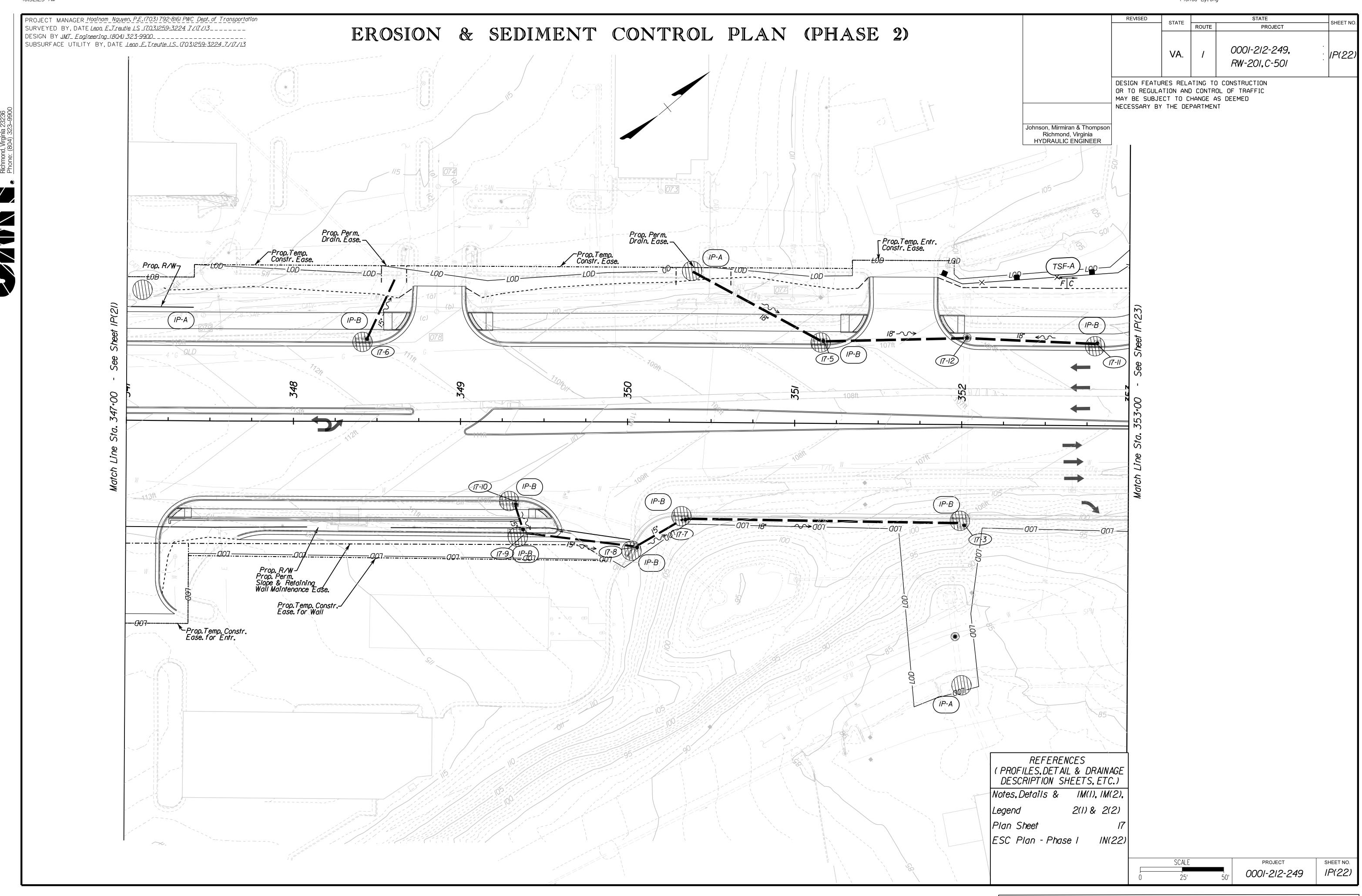


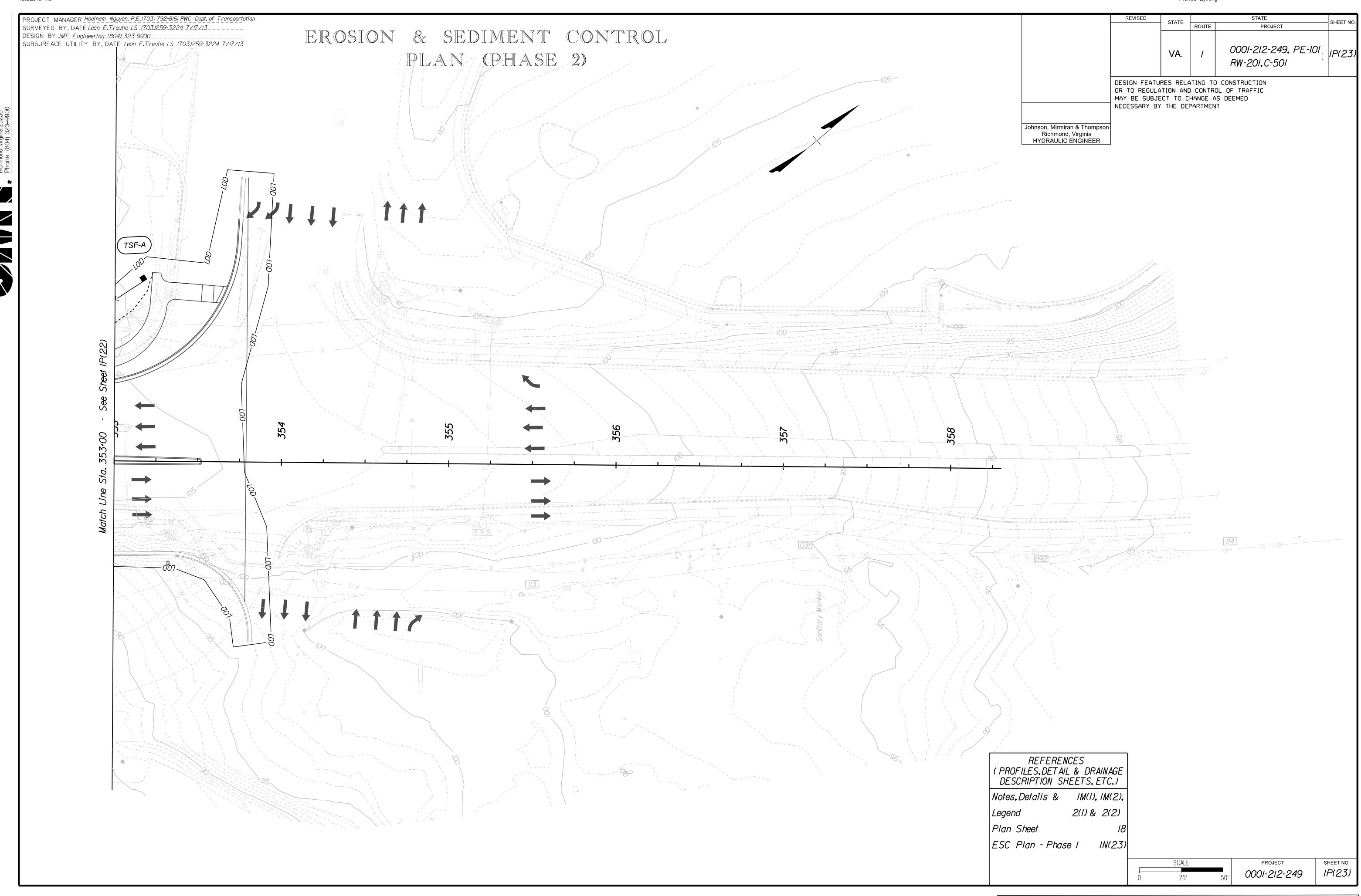


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| PROJECT MANAGER <u>Susie_Lue_(703)259-2918_NOV_A_District</u> |
|--|
| SURVEYED BY, DATE <i>Leon E.T.reutle LS_(70.3)259-3224_7/17/13</i> |
| DESIGN BY <i>JMT_Engineering_(804).323-</i> 9900 |
| SUBSURFACE LITHITY BY DATE Lean F Treutle 15 (703)259-3224 7/17/13 |

| | REVISED | STATE | | SHEET N | |
|--|---------|----------|-------|----------------------------------|-----|
| | | STHIL | ROUTE | PROJECT | |
| | | VA. | 7 | 0001-212-249,PE-101 _. | IQ. |
| | | TION AND | CONTR | | |
| VDOT Location & Design Richmond, Virginia HYDRAULIC ENGINEER | | | | `` | |

HYDROLOGIC DATA

The data presented herein was statistically derived by empirical methods and from field observations. It is presented as an estimate of the hydraulic performance of these facilities during the passage of actual flood

I. Estimated IOO year frequency flood data (unless otherwise noted.) This magnitude of flooding may pass through the proposed facility or it may obtain the necessary hydraulic conveyance by partial inundation of roadways and/or partial by pass of the facility.

2. Specified frequency flood data. It is anticipated that this magnitude of flooding will be conveyed through the proposed hydraulic facility under estimated conditions which satisfy the design criteria applicable to the site.

3. This data was obtained from observations by persons familiar with the area and/or official records combined with an evaluation by empirical methods. The reliability of this data is relative to the accuracy of the source. A future flood of the same magnitude may achieve a significantly different stage elevation from that shown due to changes in the physical characteristics of the watershed.

| FIELD | INSPECTIO | ON STAGE | FINAL DESIG | GN STAGE | BASE | FLOOD | DE | SIGN FLOC | D 2. | OVERTO FLC | | | HIST OI DAT | | 3. |
|--------------|---|----------------|------------------|-------------------|-----------------------|-----------------------------|-----------------------|--|-----------------------------|-----------------------------|--|--------------|-----------------------------|-----------------------|--|
| Sheet No. | Station | Stream Name | Drainage Area | Structure Size | Discharge (C.F.S.) | Stage Elevation (Ft.) | Discharge (C.F.S.) | Estimated Exceedance Probability % | Stage Elevation (Ft.) | Stage Elevation (Ft.) | Estimated Exceedance Probability % | Date | Stage Elevation (Ft.) | Discharge (C.F.S.) | Estimated Exceedance Probability % |
| // | 310+50 | Quantico Creek | 27.1 sq, mi. | 280-ft Bridge | 11,800 | 18.4 | 9,800 | 2% | 16.8 | 17.6 | 1.5% | June, 1972 | 17.80 | N/A | >1.5% |
| | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | |
| | | | | | REM | ARKS | | Discharges d | ı dopted froi | m FEMA FIG | nod Insurance St | udy 51153CVC | DOIA dated | August 3, 2 | 015 |
| | Historical Flood Data Source is XX(Hurricane Agnes) | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | |

PROJECT SHEET NO. 0001-212-249,PE-101 /Q

PROJECT MANAGER <u>Hodinam Nguyen</u>, P.E. (703) 792-8161 PWC <u>Dept. of Transportation</u>
SURVEYED BY, DATE <u>Leon E. Treutle LS</u> (703)259-3224 7/17/13

DESIGN BY <u>JMT Engineering</u> (804) 323-9900

SUBSURFACE UTILITY BY, DATE <u>Leon E. Treutle LS</u> (703)259-3224 7/17/13

GENERAL NOTES (SHEET 1)

GRADING

- G-1 The grade line denotes top of finished pavement unless shown otherwise on typical sections or plans.
- G-2 Earthwork quantities on this project are based on anticipated settlement and may require adjusting during construction.
- G-4 The cost of removal of all existing concrete items located in the area to be graded, including, but not limited to the following, shall be included in the price bid for regular excavation: concrete pipes, entrances, sidewalk, retaining walls, curbing, medians, ditches & bridge approaches
- G-5 The excavation of unsuitable material as specified on these plans is based on previously conducted subsurface soil investigation. If, during construction, it is deemed necessary to change the depth more than one foot, or the limits of such excavation, such change is to be made at the direction of the Engineer and measurement and payment shall be made in accordance with Section 303 of the applicable VDOT Road and Bridge Specifications.
- G-6 The borrow material for this project shall be a minimum CBR 5 or as approved by the Materials Engineer.

DRAINAGE

- D-1 The horizontal location of all drainage structures shown on these plans is approximate only, with the exception of structures showing specific stations, special design bridges and storm sewer systems.
- D-2 The horizontal location and invert elevations shown for proposed culverts and storm sewer outfall pipes are based on existing survey data and required design criteria. If during construction, it is found that the horizontal location or invert elevations shown on the plans differ significantly from the horizontal location or elevations of the stream or swale in which the culvert or storm sewer outfall pipe is to be placed, the Engineer shall confer with, and get approval from, the applicable District Drainage Engineer before installing the culvert or storm sewer outfall pipe.
- D-3 The "H" dimensions shown on plans for drop inlets and junction boxes and the "L.F." dimensions shown for manholes are for estimating purposes and are based on the proposed invert elevations shown for the structure and the anticipated top (rim) elevation based on existing or proposed finished grade. The actual "H" or "L.F." dimensions are to be determined by the contractor from field conditions.
- D-4 At Station (specify station number), the fill shall be placed and allowed to settle and displace all soft materials. Any necessary temporary drainage shall be installed. When directed by the Engineer, that part of the fill where the permanent drainage structure is to reside shall be removed and the structure placed. The cost of installing and removing the temporary drainage facility, the cost of removing the fill above the original ground for installation of permanent drainage structure and the cost of backfill shall be included in the unit price bid for regular excavation. Excavation below the original ground necessary for the installation of the permanent drainage structure will be measured and paid for in accordance with Section 303 of the applicable VDOT Road and Bridge Specifications.
- At locations where Structural Plate Steel Pipe or Pipe Arch with a concrete invert is required or is allowable as an option to Corrugated Steel Pipe or Pipe Arch, the concrete invert is to be field applied and shall cover, at a minimum, the bottom 25% of the circumference of a circular shape structure or the bottom and corner plates of an arch shape structure. As an option to providing the concrete invert, the plates along the bottom 25% (minimum) of the circumference of the Structural Plate Steel Pipe or the bottom and corner plates (minimum) of the Structural Plate Steel Pipe Arch shall be a minimum of two sheet thickness (gages) heavier than the sheet thickness (gage) indicated in the applicable VDOT Road and Bridge Standard PC-1 for the specified height of cover for the structure. Example: For a pipe with height of cover requiring 0.109" sheet thickness (12 gage) plates, the bottom plates shall be 0.168" sheet thickness (8 gage). The sheet thickness (gage) of the remainder of the pipe plates shall either conform to those specified in Standard PC-1 for the applicable height of cover or to the heavier plates used in the bottom of the pipe.
- Pipes shall conform to any of the allowable types shown on sheet number 2E(6), within the applicable height of cover limitations. For strength, sheet thickness, or class designation; available sizes; height of cover limitations; and other restrictions for a particular pipe type or height of cover, see the VDOT Road and Bridge Standard PC-1. Structural plate pipe may be substituted for corrugated pipe of the same size, provided the substitution complies with the applicable sections of the VDOT Road and Bridge Standards PC-1.

- D-8 Where open joint pipe is to be used, no joint shall be opened a distance exceeding 25% of the spigot length. Sealing of the pipe joint shall be in accordance with Section 302 of the applicable VDOT Road and Bridge Specifications.
- D-9 A pipe joint length different from that stated on the plans may be used. An adjustment in the percentage of open joint (not to exceed 25% of the spigot length) or amount of bevel shall be made that will obtain the radius stated on the plans. Extra payment for this adjustment will not be allowed. The proposed adjustment shall be approved by the Engineer prior to installation of the pipe line.
- D-10 The proposed riprap may be omitted by the Engineer if the slope designated for placement of riprap is found to be comprised of solid rock or closely consolidated boulders with soundness, size and weight equal to, or exceeding, the specifications for the proposed riprap.
- D-11 The proposed granular filter blanket for the proposed riprap may be omitted by the Engineer if the slope on which it is to be placed is found to be comprised of material which is coarser than that specified for the proposed granular filter blanket.
- D-12 All existing drainage facilities labeled "To Be Abandoned" shall be left in place, backfilled and plugged in accordance with the VDOT <u>Road and Bridge Standard</u> PP-1. Basis of Payment will be C.Y. of Flowable Backfill.
- D-13 Existing drainage facilities being utilized as a part of the drainage system, and designated on the plans "To Be Cleaned Out" shall be cleaned as directed by the Engineer. The cost incidental to this shall be included in the contract price for other items.
- D-14 Proposed drop inlets with a height (H) less than the standard minimum shown in the VDOT Road and Bridge Standards shall be considered and paid for as Standard Drop Inlets for the type specified. Pipes with less than standard minimum finished height of cover shall be noted as such in the drainage description for the pipe. Specific pipe bedding and cover requirements are provided in the applicable PB-1 and PC-1 standard drawings of the VDOT Road and Bridge Standards.
- D-16 When CG-6 or CG-7 is specified on a radius (such as at a street intersection), the Engineer may approve a decrease in the cross slope of the gutter to facilitate proper drainage.
- D-17 St'd. SL-1 Safety Slab locations are based on the assumed use of precast structures. If cast-in-place structures are utilized, and the interior chamber dimensions (length and width, or diameter) are less than 4 feet, the safety slabs shall not be installed.

PAVEMENT

- P-1 If any settlement occurs in concrete pavement adjacent to bridges prior to acceptance of the project by the Department, the contractor shall restore the pavement to the original grade either by the mud jack method or by replacing the pavement. In the event the pavement cracks or becomes damaged, it shall be replaced, if directed by the Engineer.
- P-2 The pavement materials on this project will be paid for on a tonnage basis. The weight will vary in accordance with the specific gravity of the aggregates and the asphaltic content of the mix actually used to secure the design depth. The weight of the asphalt concrete is based on 95% of the theoretical maximum density.

| REVISED | STATE | | STATE | SHEET NO. |
|---------|-------|-------|--------------------------------|-----------|
| | SIAIE | ROUTE | PROJECT | SHEET NO. |
| | VA. | / | 0001-212-249, RW-201, C-501 | : 2(1) |

DESIGN FEATURES RELATING TO CONSTRUCTION
OR TO REGULATION AND CONTROL OF TRAFFIC
MAY BE SUBJECT TO CHANGE AS DEEMED
NECESSARY BY THE DEPARTMENT

INCIDENTALS

- 1-3 Service Roads are to be constructed, and private entrances connected thereto prior to the permanent severing of private entrances by other phases of the proposed construction.
- 1-4 All trees located within the Clear Zone or within a minimum of 30 feet of the edge of pavement, within the limits of the right of way or construction easement, unless otherwise noted on plans or directed by the Engineer, shall be removed, as provided for a Section 301 of the applicable VDOT Road and Bridge Specifications.
- 1-5 That portion of the right of way lying within the Clear Zone or within a minimum of 10 feet from the edge of pavement or surfacing or within the limits of the construction slopes beyond 10 feet, shall be cleared and grubbed in accordance with the applicable VDOT Road and Bridge Specifications, Section 301. where sufficient right of way or construction easement is provided.
- 1-7 Where Standard slope roundoffs would damage trees, bushes or other desirable vegetation, they shall be omitted when so ordered by the Engineer.
- 1-9 When no centerline alignment is shown for a proposed entrance, the entrance shall be constructed in the same location as the existing entrance.
- I-12 St'd. RM-2 right of way monuments shall be set by the Contractor.
- I-14 Salvaged guardrail materials not used in the new construction shall become the property of the Contractor and shall be disposed of at a licensed landfill, recycled or be retained by the Contractor.
- I-16 The "underground utilities" survey data on this project has been provided by consultant and copies are available from the Department.
- I-17 For method of constructing Straight-Line Taper Lanes in curb and/or curb and gutter sections, see typical details on Sheet 2B.
- I-18 All pavement markings and traffic flow arrows shown on the roadway construction plans are schematic only. The actual location and application of pavement markings shall be in accordance with Section 704 of the applicable VDOT Road and Bridge Specifications, MUTCD, sequence of construction/traffic control plans, pavement marking plan sheets 21(1) thru 21(18) and as directed by the Engineer.
- I-19 The following outside sources, under contract with VDOT, have provided information on this project.

Hydraulic Design - JMT
Roadway Design - JMT
Utility Design - JMT and others
Utility Designation - VDOT
Utility Location - VDOT
Survey - VDOT, JMT & RDA
Bridge Design - JMT
Traffic Design - JMT
Landscape Design - N/A

If questions or problems arise during construction, please contact the Area Construction Engineer. <u>DO NOT CONTACT THE OUTSIDE SOURCES</u>.

1-20 The Official Electronic PDF Version of the plans will override the paper copies or prints of specific layers.

Portions of this plan assembly have been CADD generated. To assist in the preparation of the bid and construction of the project, Microstation format (.dgn) files will be made available to the prime contractor during bids and after award of the contract.

I-21 All electronic plan assemblies will include the construction plans in two formats: PDF files and MicroStation format (.dgn) files. Only the PDF files will be considered as part of the official plan assembly.

The MicroStation format (.dgn) files are furnished only as information for the contractor. These plans are developed in layers (levels) to aid in readability. (See the VDOT CADD Manual for CADD Level Structure). However, the construction items may or may not be in the proper layering scheme as described in the VDOT CADD Manual. The Microstation files will only match the scanned files if all required levels are turned on. A Microstation Software license is required to be able to read these files.

PROJECT SHEET NO. 2(1)

GENERAL NOTES (Sheet 2)

STORMWATER MANAGEMENT

- S-1 CLEARING AND GRUBBING OF SWM BASIN SITE The area where the dam is to be constructed and the area upstream of the dam, to an elevation equal to the crest of the dam (maximum ponded water elevation), shall be cleared and grubbed in accordance with Section 301 of the applicable VDOT Road and Bridge Specifications.
- S-2 SWM BASIN DAM CONSTRUCTION The dam for detention basins (no permanent pool) shall conform to the details contained in the plans and shall be constructed in accordance with Section 303 of the applicable VDOT Road and Bridge Specifications. The native material on which the dam will set shall meet the specifications for AASHTO Type A-4 or finer material. Where the native material does not meet this requirement, the area beneath the dam is to be excavated a minimum of 4' and backfilled with a material meeting the AASHTO Type A-4 or finer classification unless otherwise specified in the plans. The material used for the embankment of the dam shall be AASHTO Type A-4 or finer or otherwise specified in the plans. Dams with foundation and embankment material not meeting the above requirements or dams greater than 15' in height, or dams for retention basins (permanent pool) shall incorporate a membrane-lined trench, a homogenous embankment with seepage controls, a zoned embankment or other such approved designs as specified in the plans.
- S-3 SWM BASIN OUTLET PIPE The pipe culvert under or through the dam for detention basins (no permanent pool) shall be reinforced concrete pipe with rubber gaskets in accordance with Section 232 and 212 of the applicable VDOT Road and Bridge Specifications. A concrete cradle shall extend the full length of the pipe culvert in accordance with the Standard Drawings. The connection between the pipe culvert and the SWM-1 Drainage Structure (or other control structure) shall be made watertight as approved by the Engineer and the cost shall be included in the price bid for pipe.
- S-4 The SWM-1 Drainage Structure (or other control structure) shall have 4" high numbers and 1" wide stripes painted at 1' intervals as shown on the Standard Drawings or detail sheets. The numbers and stripes are to be installed at the time of the initial installation of the SWM-1 Drainage Structure (or other control structure). Paint and application shall be in accordance with Section 231 and 411 of the applicable VDOT Road and Bridge Specifications and the cost is to be included in the price bid for the applicable structure.
- S-5 All SWM Basins designated for use as temporary sediment basins shall be constructed during the initial phase of earth moving activities or as specified by the plans or directed by the Engineer. During project construction, the SWM-1 Drainage Structure (or other control structure) shall be modified in accordance with the Standard Drawings or plan details in order to provide a temporary sediment basin with both a "wet" storage volume (permanent pool) and a "dry" storage volume. Sediment accumulated in the basin shall be removed when the volume of the "wet" storage (permanent pool) has been reduced by 50%. Sediment shall be disposed of in accordance with Section 106.04 of the applicable VDOT Road and Bridge Specifications. When project construction is complete to a stage where no additional sediment from the project is expected to enter the basin, as determined by the Engineer, the basin shall be cleaned out and restored to the original design elevations, the area stabilized and all temporary modifications to the SWM-1 Drainage Structure (or other control structure) removed.

EROSION AND SEDIMENT CONTROL (ESC)

- E-1 If the removal of Brush Silt Barrier is specified by the plans or required by the Engineer, the cost of removal and disposal of brush shall be in accordance with Section 109 of the applicable VDOT Road and Bridge Specifications.
- E-2 Rock for Check Dams, Inlet Protection, Erosion Control Stone and Riprap shall be in accordance with Section 203 and Section 414 of the applicable VDOT Road and Bridge Specifications.
- E-3 The following symbols are used to depict Erosion Control items in the plan assembly:

Denotes Rolled Erosion Control Product, Temporary, St'd. EC-2 Type 1, 2, 3 or 4 >>>>>>(EC-2, Ty. 1) (EC-2, Ty. 2) (EC-2, Ty. 3) (EC-2, Ty. 4) ≥ ≥ ≥ ≥ (EC-3, Ty. 1) (EC-3, Ty. 2) Denotes Rolled Erosion Control Product, Permanent, St'd. EC-3 Type 1, 2 or 3 (EC-3, Ty. 3) TSF-B Denotes Temporary Silt Fence, St'd EC-5 Type A or B TCD Denotes Temporary Check Dam, St'd EC-16 TDC) Denotes Temporary Diversion Channel, St'd EC-12 (DD) Denotes Temporary Diversion Dike, St'd EC-9 Denotes Turbidity Curtain, Type - Impervious ------ Denotes Turbidity Curtain, Type - Pervious RCD-1 Denotes Rock Check Dam, Type I; St'd EC-4 RCD-2 Denotes Rock Check Dam, Type II; St'd EC-4 (IP-A Denotes Inlet Protection, Type A; St'd EC-6 IP-B Denotes Inlet Protection, Type B; St'd EC-6 TSI Denotes Slope Interrupter; St'd EC-15 (DS) Denotes Dewatering Basin; St'd EC-8

E-4 Permanent vegetation shall be established on all denuded areas not otherwise stabilized with non-erodible materials. See the Roadside Development sheet for details on permanent vegetation establishment.

DESIGN FEATURES RELATING TO CONSTRUCTION OR TO REGULATION AND CONTROL OF TRAFFIC MAY BE SUBJECT TO CHANGE AS DEEMED NECESSARY BY THE DEPARTMENT

PROJECT SHEET NO. 0001-212-249 2(2)

dll948l02a(l).dgn Plotted By:ong

PROJECT MANAGER <u>Hodinam Nguyen, P.E.(703) 792-8161 PWC Dept. of Transport</u>ation SURVEYED BY, DATE <u>Leon E.Treutle</u> LS <u>(703)259-3224 7/17/13</u> _____ DESIGN BY <u>JMT_Engineering</u> (804) 323-9900 _____ SUBSURFACE UTILITY BY, DATE <u>Leon E.Treutle</u> LS (703)259-3224_7/17/L3

TYPICAL SECTIONS

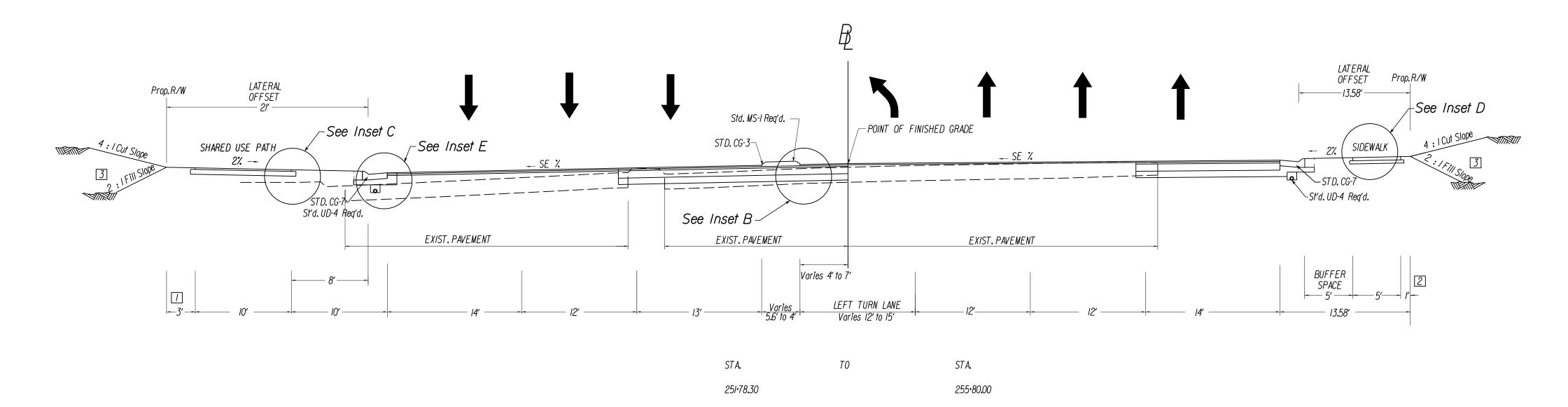
Route I (Fraley Boulevard) - 6 Lane Design Typical GS-5 - Urban Principal Arterial - 45 MPH Design Speed VA. / OOOI-212-249, : 2A(I)

DESIGN FEATURES RELATING TO CONSTRUCTION

DESIGN FEATURES RELATING TO CONSTRUCTION OR TO REGULATION AND CONTROL OF TRAFFIC MAY BE SUBJECT TO CHANGE AS DEEMED NECESSARY BY THE DEPARTMENT

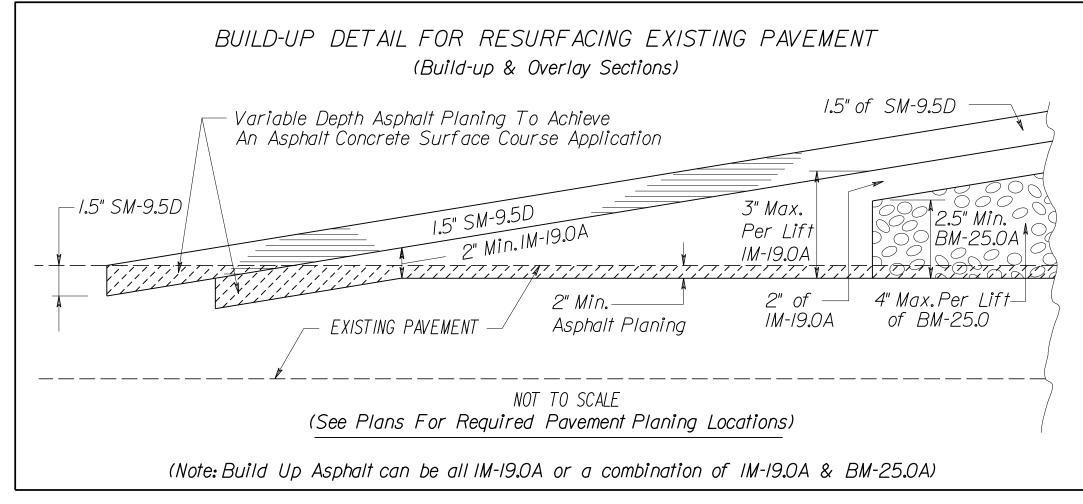
Johnson, Mirmiran & Thompson
Richmond, Virginia
ROADWAY ENGINEER

VDOT Materials
Fairfax, Virginia
MATERIALS ENGINEER



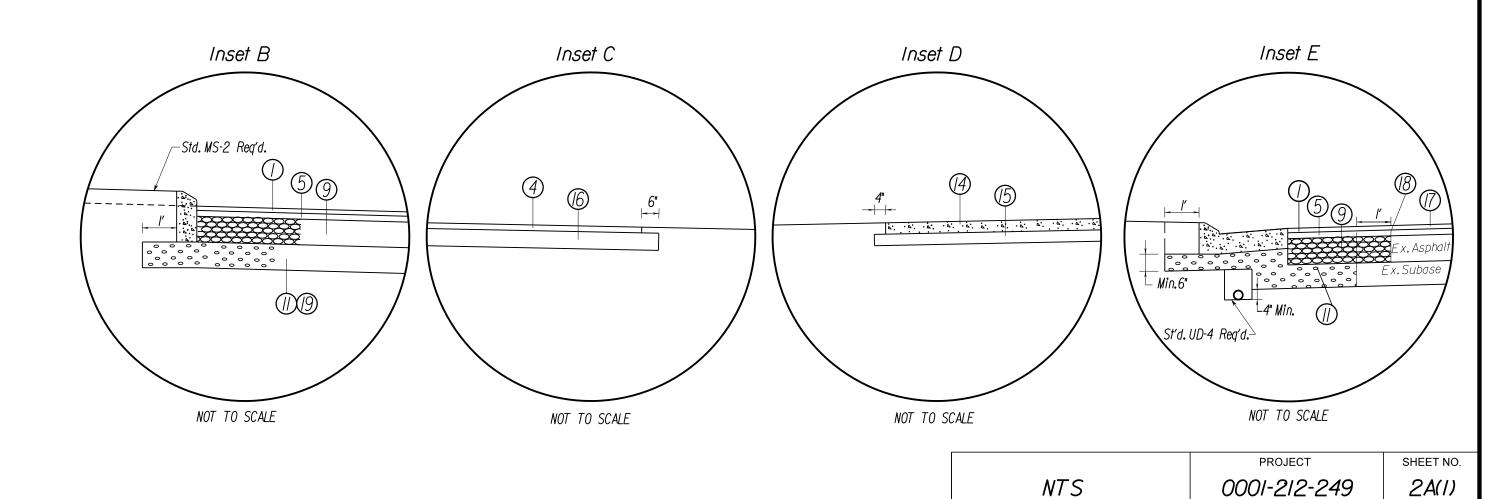
Pavement Legend

- (1) 1.5" Asphalt Concrete, Type SM-9.5D (estimated 175 lbs/sy)
- ② 1.5" Asphalt Concrete, Type SM-9.5A (estimated 175 lbs/sy)
- (3) 2" Asphalt Concrete, Type SM-9.5A (estimated 234 lbs/sy)
- 4 2" Asphalt Concrete, Type SM-9.5A (estimated 240 lbs/sy)
- (5) 2" Asphalt Concrete, Type IM-19.0A (estimated 234 lbs/sy)
- 6 4" Asphalt Concrete, Type BM-25.0A
- 7 6" Asphalt Concrete, Type BM-25.0A
- 8 7" Asphalt Concrete, Type BM-25.0A
- 9 9" Asphalt Concrete, Type BM-25.0A
- © 6" Aggregate Base Mat'I.Type I,No.2IB connected to a standard UD-4 edgedrain located beneath the curb and gutter.
- (I) Min. 8" Aggregate Base Mat'I. Type I, No. 21B or extended to the bottom of existing aggregate, whichever is greater, and connected to a standard UD-4 edgedrain located beneath the curb and gutter.
- 12" Aggregate Base Mat'I.Type I,No.2IB connected to a standard UD-4 edgedrain located beneath the curb and gutter.
- (3) 14" Aggregate Base Mat'I.Type I,No.21B connected to a standard UD-4 edgedrain located beneath the curb and gutter.
- (4) 4" Hydraulic Cement Concrete, Class A3
- (5) 4" Aggregate Base Mat'l. Type I, No. 2IB extended 4" beyond the edge of the surface material
- (6) 6" Aggregate Base Mat'l. Type I, No. 2IB extended 6" beyond the edge of the surface material
- \(\text{T} \) Existing pavement to be milled 2" and resurfaced
 \(\text{See Build-Up Detail for Resurfacing Existing Pavement Sheet 2A(I)} \)
- (8) Existing pavement is to be Saw Cut to the full depth of asphalt at least I foot from the edge of the existing pavement. Abut the new Pavement layers to existing layer per Std.WP-2.
- Replace 2IB with CTA for widening on high side of existing pavement cross slopes

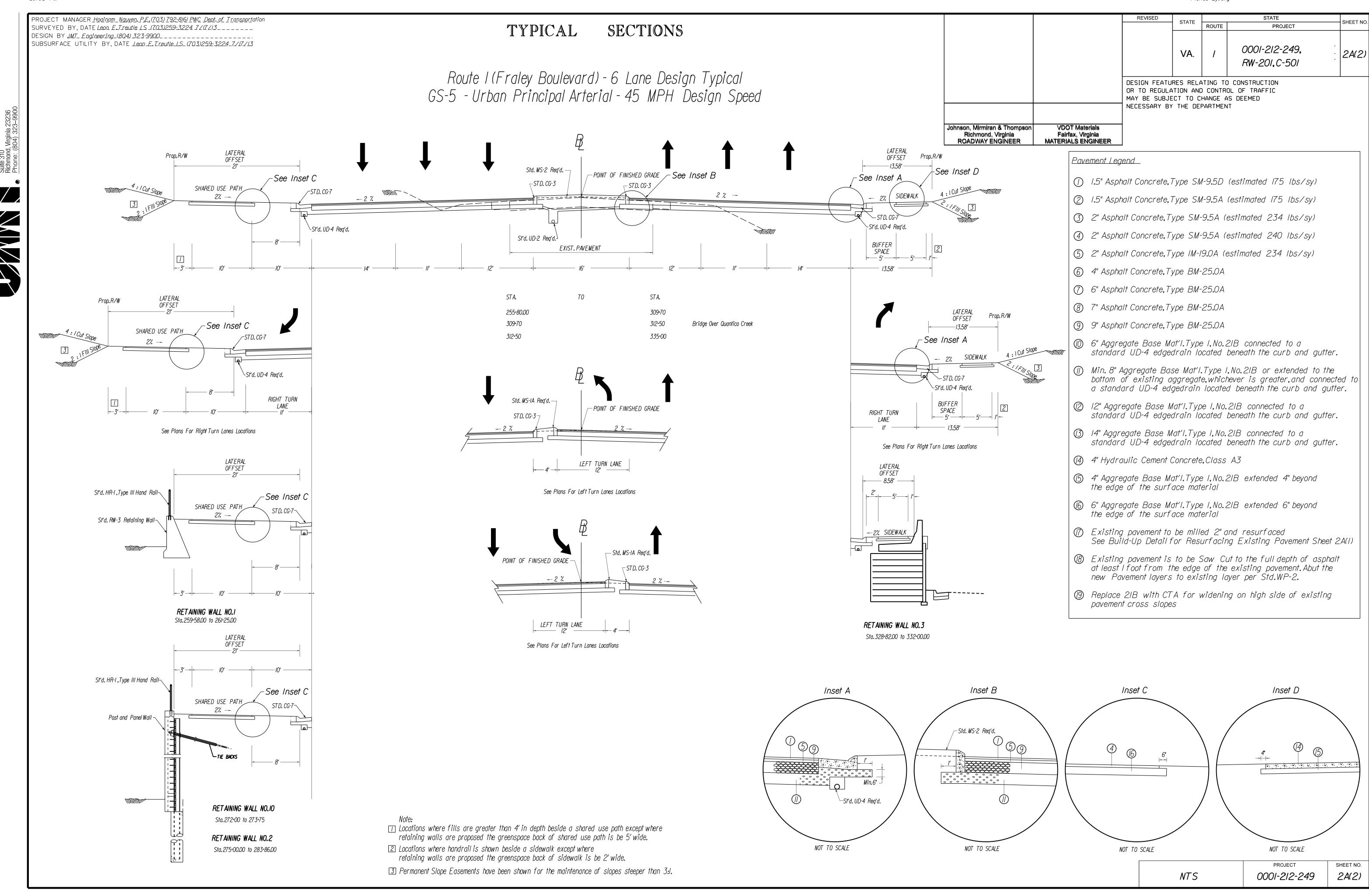


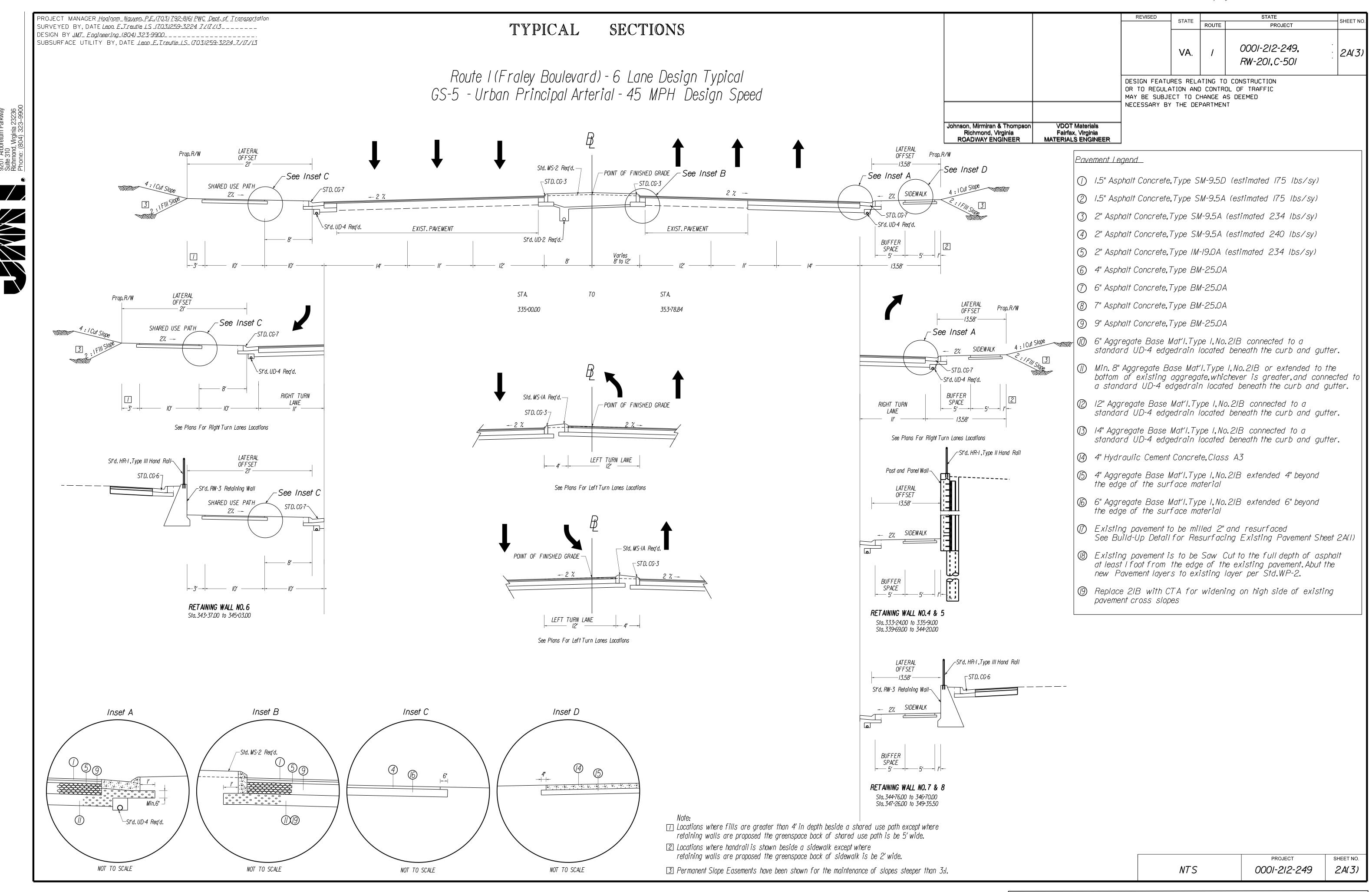
Note:

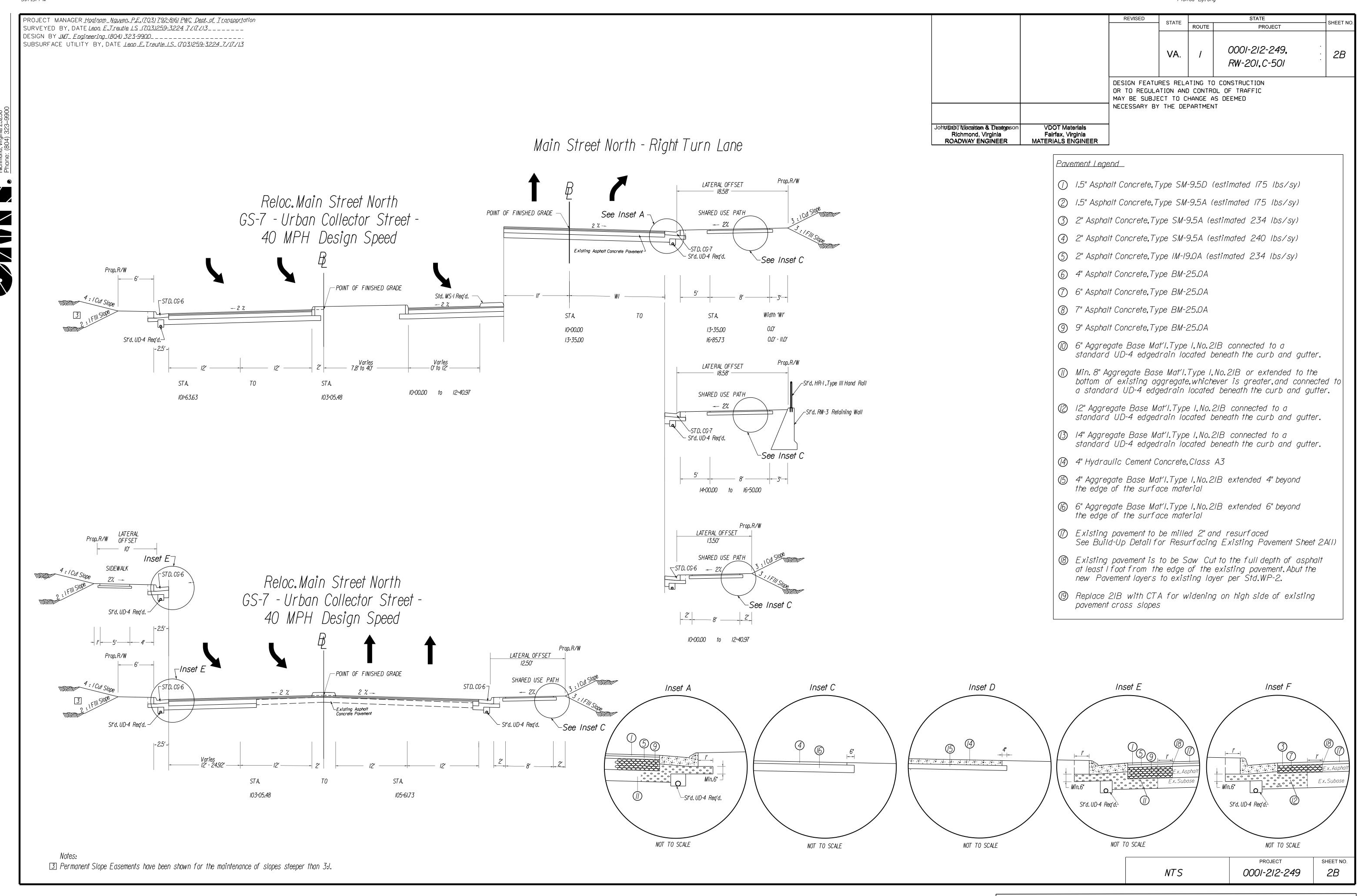
- Locations where fills are greater than 4' in depth beside a shared use path except where retaining walls are proposed the greenspace back of shared use path is be 5' wide.
- 2 Locations where handrail is shown beside a sidewalk except where retaining walls are proposed the greenspace back of sidewalk is be 2' wide.
- 3 Permanent Slope Easements have been shown for the maintenance of slopes steeper than 3:1.

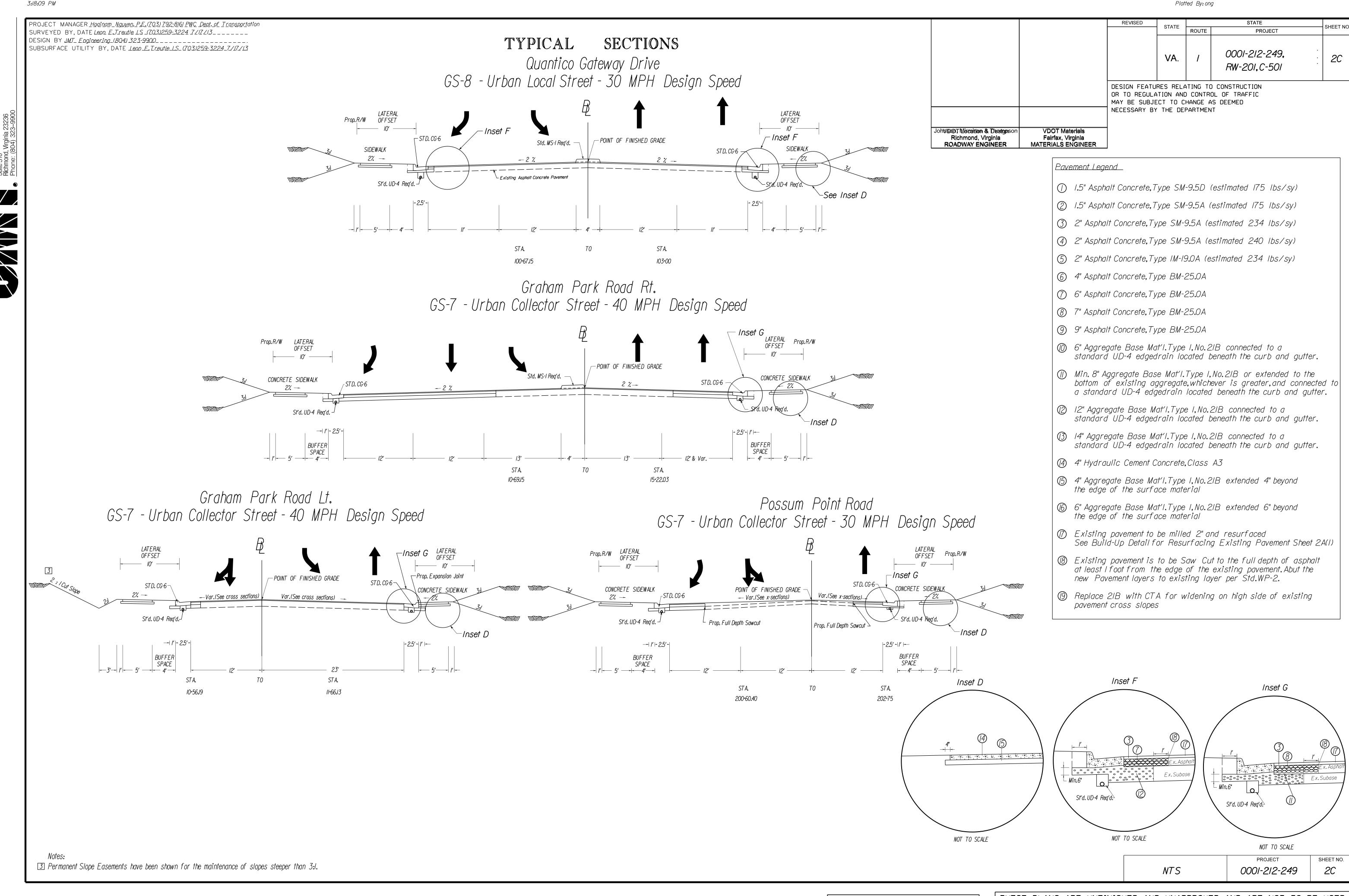


R/W PLANS









VA.

NECESSARY BY THE DEPARTMENT

DESIGN FEATURES RELATING TO CONSTRUCTION OR TO REGULATION AND CONTROL OF TRAFFIC MAY BE SUBJECT TO CHANGE AS DEEMED

ROUTE

PROJECT

2D(I)

0001-212-249,

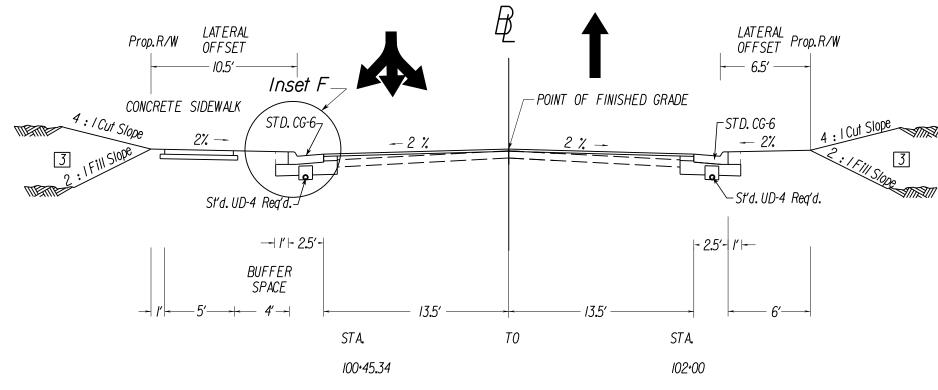
RW-201, C-501

PROJECT MANAGER <u>Hoainam Nguyen, P.E.(703) 792-8161 PWC Dept. of Transportation</u>
SURVEYED BY, DATE <u>Leon E.Treutle</u> LS <u>(703)259-3224 7/17/13</u>
DESIGN BY <u>JMT_Engineering</u> (804) 323-9900

SUBSURFACE UTILITY BY, DATE Lean F. Treutle LS (703)259-3224 7/17/13

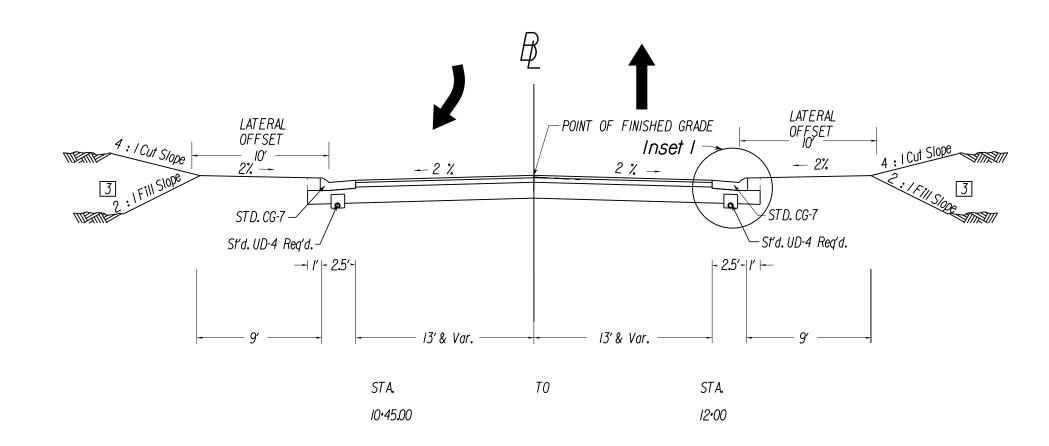
TYPICAL SECTIONS

Tripoli Boulevard GS-8 - Urban Local Street - 30 MPH Design Speed

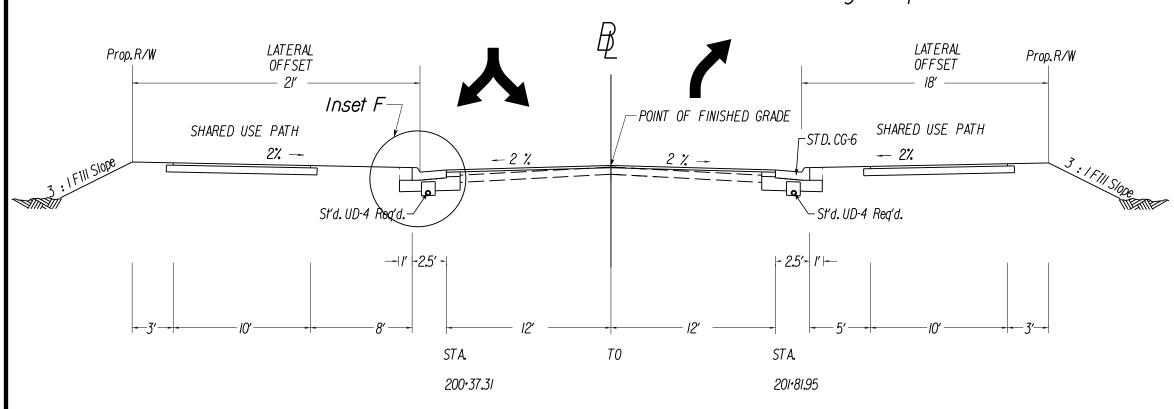


Notes:
All pavement widening shall be performed in accordance with standard WP-2.

Reloc. Duke Street GS-8 - Urban Local - 20 MPH Design Speed



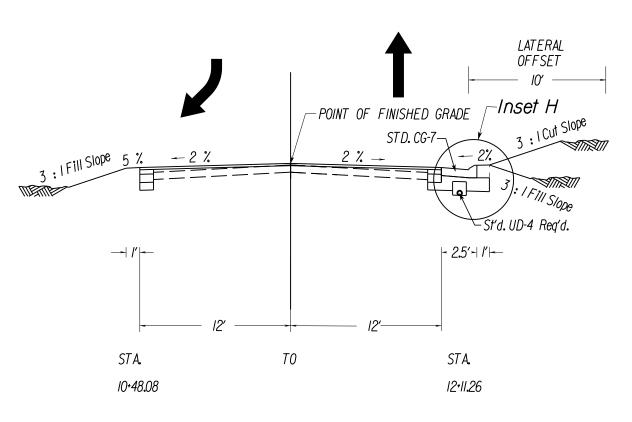
Canal Road LT GS-8 - Urban Local Street - 25 MPH Design Speed



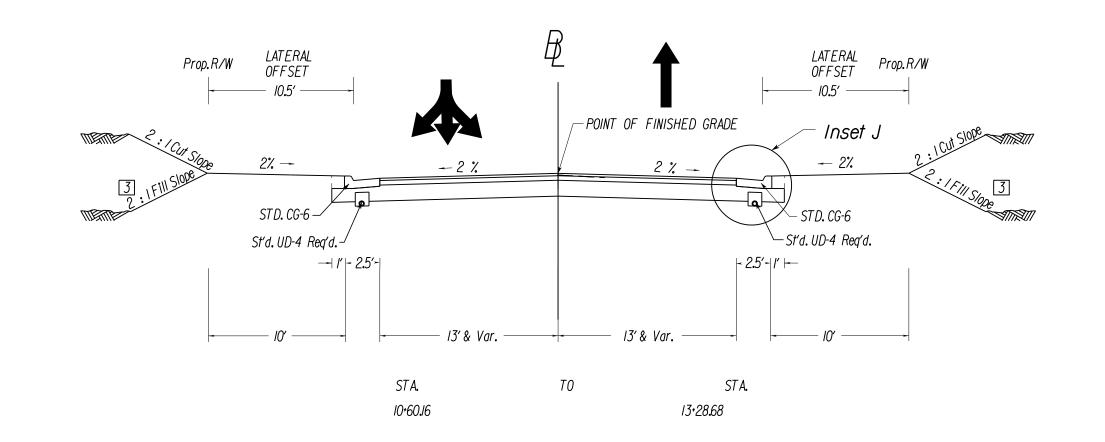
Notes:

3 Permanent Slope Easements have been shown for the maintenance of slopes steeper than 3:1.

Canal Road RT GS-8 - Urban Local Street - 25 MPH Design Speed



Reloc. Old Stage Coach Road GS-8 - Urban Local - 30 MPH Design Speed



Inset F

NOT TO SCALE

St'd. UD-4 Reg'd.

gn Speed

Richmond, Virginia

ROADWAY ENGINEER

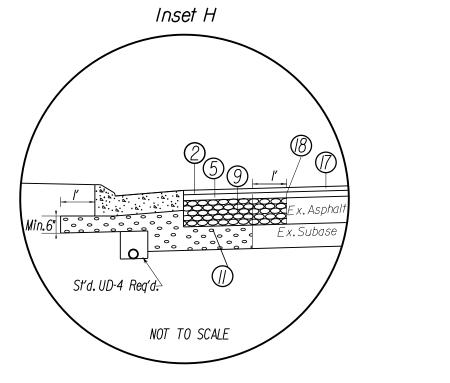
John Soot Microaitian & Designso

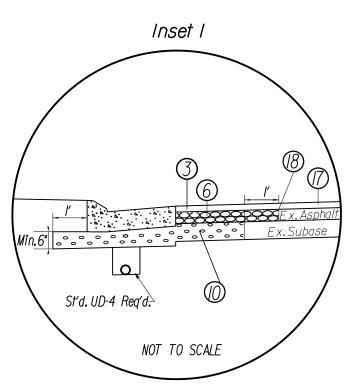
Pavement Legend

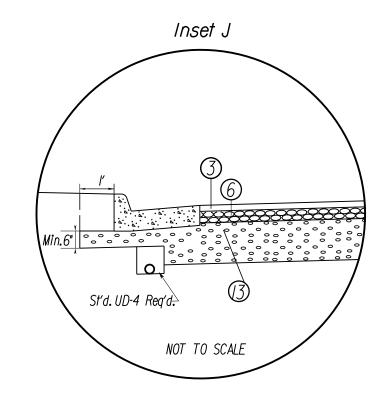
VDOT Materials

Fairfax, Virginla MATERIALS ENGINEER

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- 3) 2" Asphalt Concrete,Type SM-9.5A (estimated 234 lbs/sy)
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- Replace 2IB with CTA for widening on high side of existing pavement cross slopes







NTS

PROJECT 0001-212-249 SHEET NO.

2D(I)

R/W PLANS

PROJECT MANAGER <u>Hoainam Nguyen</u>, P.E. (703) 792-8161 PWC <u>Dept. of Transportation</u> SURVEYED BY, DATE Leon E.Treutle LS 17031259-3224 7/17/13_____ DESIGN BY JMT_Engineering_(804) 323-9900_______

SUBSURFACE UTILITY BY, DATE Lean E. Treutle LS (703)259-3224_7/17/L3

SECTIONS TYPICAL

PROJECT ROUTE 0001-212-249, VA. RW-201, C-501

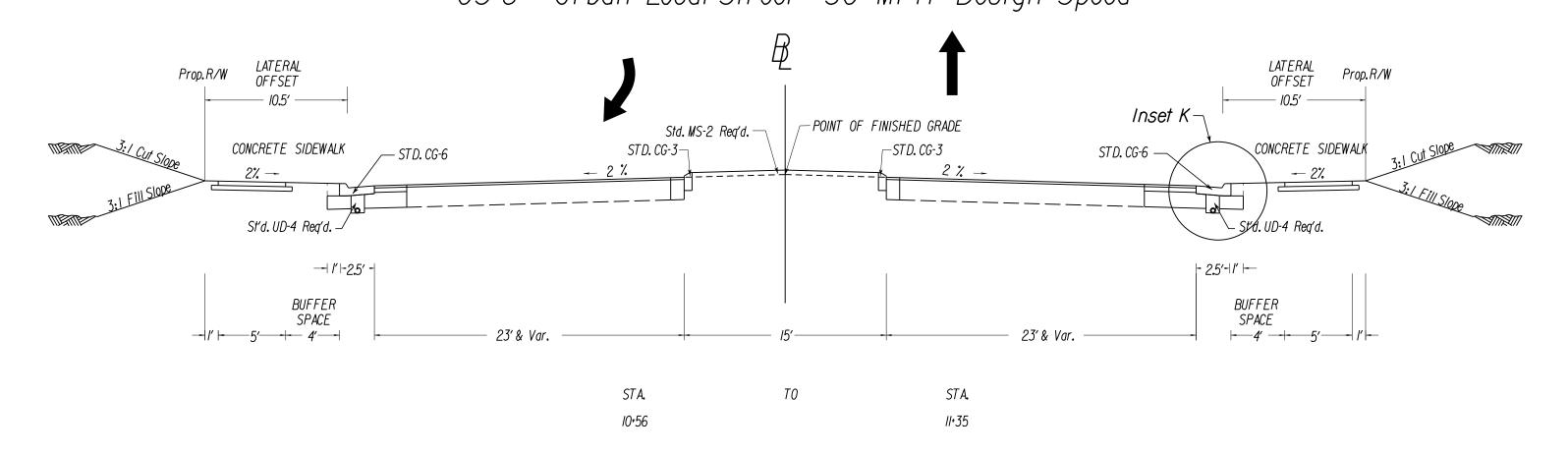
> DESIGN FEATURES RELATING TO CONSTRUCTION OR TO REGULATION AND CONTROL OF TRAFFIC MAY BE SUBJECT TO CHANGE AS DEEMED NECESSARY BY THE DEPARTMENT

Johnson, Mirmiran & Thompson Richmond, Virginia ROADWAY ENGINEER

VDOT Materials Fairfax, Virginia MATERIALS ENGINEER

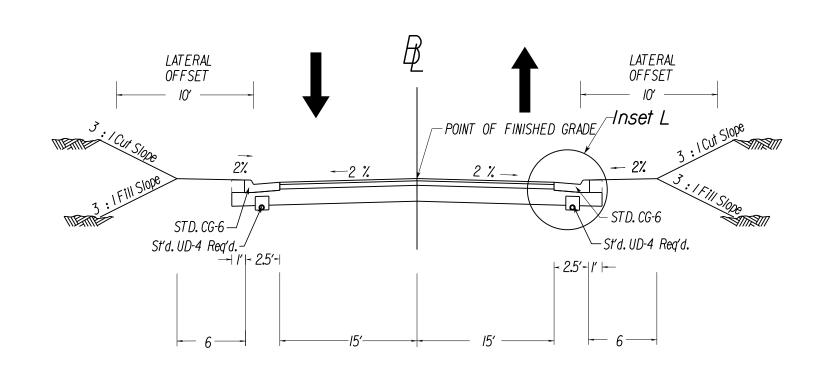
Pavement Legend

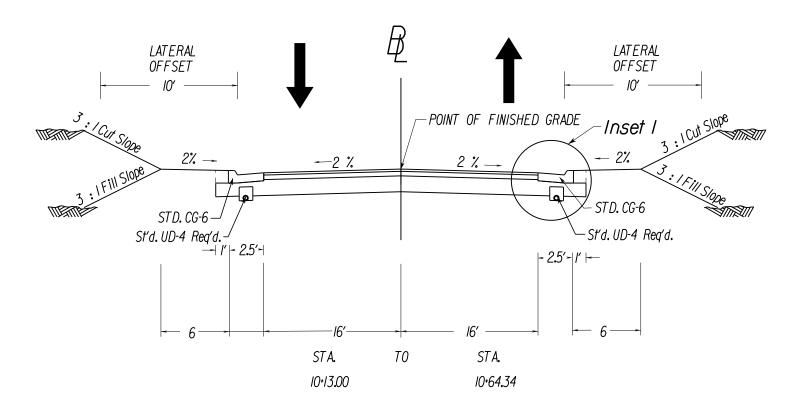
Williamstown Drive GS-8 - Urban Local Street - 30 MPH Design Speed

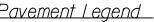


Triangle Shopping Plaza Entrance

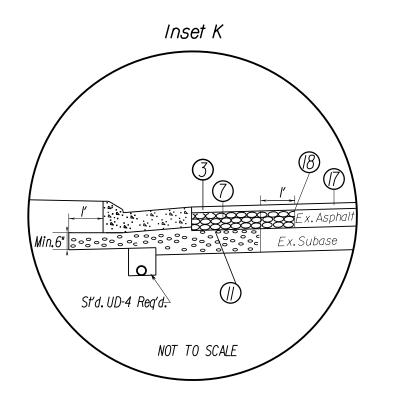
Town Square Ct GS-8 - Urban Local St - 25 MPH Design Speed

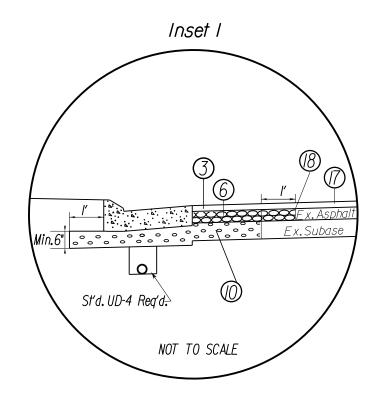


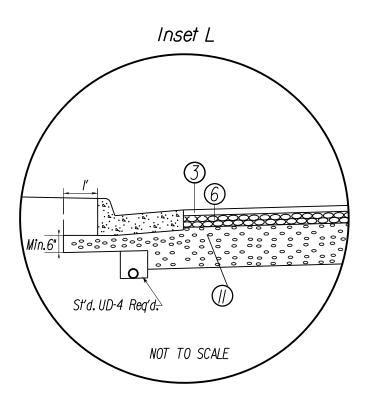




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- (3) 2" Asphalt Concrete, Type SM-9.5A (estimated 234 lbs/sy)
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- Min. 8" Aggregate Base Mat'I.Type I,No.2IB or extended to the bottom of existing aggregate, whichever is greater, and connected to a standard UD-4 edgedrain located beneath the curb and gutter.
- (2) 12" Aggregate Base Mat'l. Type I, No. 21B connected to a standard UD-4 edgedrain located beneath the curb and gutter.
- (3) 14" Aggregate Base Mat'I.Type I, No. 21B connected to a standard UD-4 edgedrain located beneath the curb and gutter.
- (4) 4" Hydraulic Cement Concrete, Class A3
- (5) 4" Aggregate Base Mat'l. Type I, No. 2IB extended 4" beyond the edge of the surface material
- 6 6 Aggregate Base Mat'l. Type I, No. 2IB extended 6 beyond the edge of the surface material
- © Existing pavement to be milled 2" and resurfaced
 See Build-Up Detail for Resurfacing Existing Pavement Sheet 2A(I)
- (8) Existing pavement is to be Saw Cut to the full depth of asphalt at least I foot from the edge of the existing pavement. Abut the new Pavement layers to existing layer per Std.WP-2.
- Replace 2IB with CTA for widening on high side of existing pavement cross slopes







NTS

PROJECT 0001-212-249

2D(2)

SHEET NO.

VA.

NECESSARY BY THE DEPARTMENT

DESIGN FEATURES RELATING TO CONSTRUCTION

OR TO REGULATION AND CONTROL OF TRAFFIC MAY BE SUBJECT TO CHANGE AS DEEMED

ROUTE

PROJECT

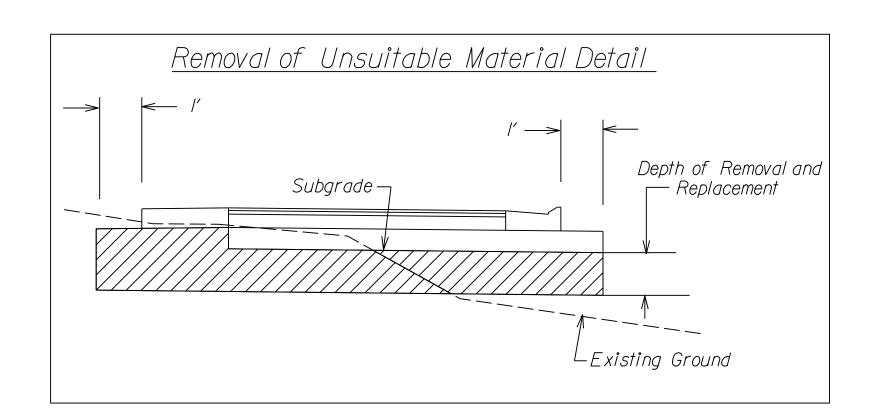
2D(3)

0001-212-249,

RW-201, C-501

PROJECT MANAGER <u>Hoainam Nguyen, P.E.(703) 792-8161 PWC Dept. of Transportation</u> SURVEYED BY, DATE Leon E.Treutle LS 1/2031259-3224 7/17/13_____ DESIGN BY JMT_Engineering_(804)323-9900_______ SUBSURFACE UTILITY BY, DATE Leon F. Treutle LS (703)259-3224_7/17/13

TYPICAL SECTIONS



| | Onsan | able Materials at,or Below Sub | gr dd c | |
|-------------|--|--------------------------------|-----------|---------------------------|
| Location | Depth of Unsuitables / Treatment | Estimated Limits | Direction | Reason/Comments |
| Route I | 12" (a) | 255+00 to 258+00 | NB+SB | Fat clay, wet soils |
| Route I | 12" (d) | 258+25 to 260+50 | SB | Wet soils |
| Route I | 24" (e) | 262+00 to 263+25 | NB | Soft, wet soils |
| Route I | 12" (d) | 270+75 to 271+75 | NB | Wet soils |
| Route I | 24" (e) | 272+25 to 274+00 | NB+SB | Soft, wet soils |
| Route I | 24" (b) | 274+00 to 276+50 | NB | Fat clay |
| Route I | 24" (b) | 279+50 to 282+00** | NB+SB | Fat clay |
| Route I | 24" (b) | 286+00 to 292+00 | NB | Fat clay |
| Route I | 24" (b) | 292+00 to 295+00 | SB | Fat clay,sat.soils |
| Route I | 24" (b) | 298+50 to 30I+50 | NB | Fat clay, very soft soils |
| Route I | 24" (b) | 303+50 to 306+00 | NB | Fat clay |
| Route I | 24" (c) | 304+25 to 306+00 | SB | Soft, wet soils |
| Route I | 24" (e) | 313+00 to 320+00 | SB | Wetlands |
| Route I | 36" (b) | 320+00 to 327+50 | SB | Organic and wet soils |
| Route I | 24" (a) | 324+00 to 336+25 | NB | Fat clay,asphalt millings |
| Route I | 24" (a) | 328+50 to 335+00 | SB | Fat clay,asphalt millings |
| Route I | 24" (a) | 338+50 to 342+00 | SB | Fat clay, wet soils |
| Route I | 24" (a) | 34I+75 to 344+50 | NB | Fat clay, wet soils |
| Route I | 24" (b) | 348+50 to 350+00 | NB | Fat clay |
| Main Street | 24" (b) | 100+75 to 103+00 | NB+SB | Organic soils |
| Main Street | 24" (b) | 176+00 to 177+75 | NB+SB | Fat clay |
| | | | | Fat clay |

(a) Excavate unsuitable material, replace with Select Material, Type I, Minimum CBR-30, placed on a woven geotextile subgrade stabilization fabric.

10+50 to 11+00

10+45 to 12+00

- (b) Excavate unsuitable material, replace with Select Material, Type i, Minimum CBR-30.
- (c) Excavate unsuitable material and replace with regular soil fill having a minimum CBR-5.

Fat clay

Fat clay, wet soils

EB+WB

EB+WB

(d) Scarify, aerate and re-compact.

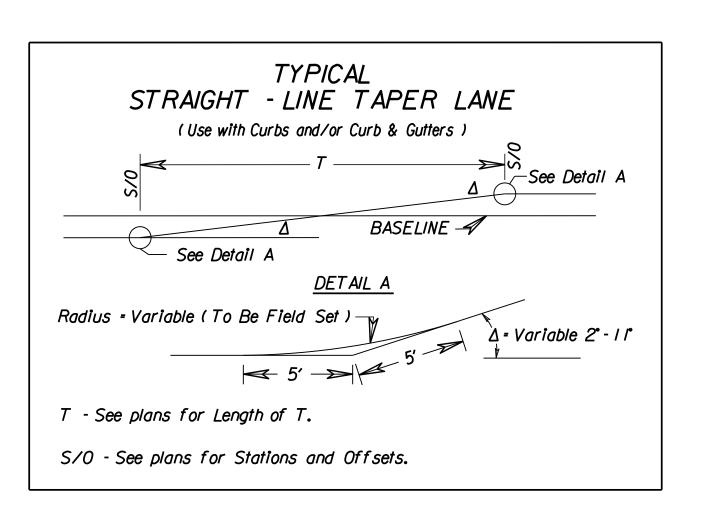
24"(b)

24" (b)

Williamstown Dr.

Reloc. Duke St.

(e) Use rock fill for the initial 24" of embankment.



Johnson, Mirmiran & Thompson

Richmond, Virginia

ROADWAY ENGINEER

VDOT Materials Fairfax, Virginia MATERIALS ENGINEER

| | | Slope Stability Recommen | dations |
|------------|----------|---------------------------|--|
| | Location | Station to Station,Offset | Slope Recommendation |
| Slope No.l | Route I | 262+00 to 263+50,Right | 3:I Fill Slope |
| Slope No.2 | Route I | 269+25 to 270+00,Right | 3:1 Cut Slope |
| Slope No.3 | Route I | 268+25 to 27I+50,Left | 3:1 Cut Slope |
| Slope No.4 | Route I | 272+50 to 274+75,Left | 2:I Fill Slope |
| Slope No.5 | Route I | 277+75 to 281+75,Right | 3:1 Cut Slope w/ one-row of 30-inch diameter drilled shafts installed 20 feet upslope form the toe and extended to a tip elevation of 68 feet. |
| Slope No.6 | Route I | 313+25 to 321+00,Left | 2:1 Fill Slope |

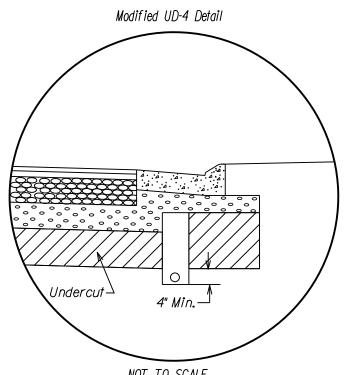
PRIVATE AND COMMERCIAL ENTRANCES

| TYPE I Crusher Run Aggr. | TYPE II Concrete |
|----------------------------------|---|
| | ·à··à··à··à··à· |
| 6" Crusher Run Aggr. 25 or 26 | Concrete Entrance Pavemeni 7" HES 4" Aggr. Base Mat'I. Ty. I No. 21B |

NOT TO SCALE TYPE III Asphalt Conc.Type SM-9.5D @ 220 Lbs. per S.Y. 4" Aggr. Base Mat'l. Ty. I No. 21B

TYPE IV Asphalt Commercial Asphalt Conc. Type SM-9.5D @ 165 Lbs. per S.Y. 4" Asphalt Conc. Base Course BM-25.0A 6" Aggr. Base Mat'l. Ty. I No. 2IB

The type of entrance (I, II, III, IV) to be constructed will be determined by the existing condition at the time of construction.



I. In undercut areas where UD-4 is to be installed, the UD-4 shall be lowered a minimum of 4" below the bottom of the undercut area. Outlets shall be checked for positive drainage.

| Modified UD-4 Detail | |
|----------------------|--|
| | |
| | |
| | |
| | |
| Undercut | |
| 4" Min.— | |
| NOT TO SCALE | |

Existing Pavement Data (Pavement depths shown are an average depths of existing paving materials) Type of Paving Materials (Average Depths) Station To Station Roadway Route | Southbound (Main Street) II" Asphalt Concrete 7" Hydraulic Cement Concrete Project Length 8.1" Asphlat Concrete 7.8" Hudralic Cement Concrete Route | Northbound (Fraley Boulevard) Project Length 9" Asphalt Concrete 9.2" Crushed Aggregate 9" Asphalt Concrete 9.2" Crushed Aggregate Project Length Existing Turn Lanes

> PROJECT SHEET NO. NTS 0001-212-249 2D(3)

SHEET 3

DI-3-2) I ST'D DI-3B REQ'D H=4.4' L=4' INV=148.00 ST'D ST-1 REQ'D CONNECT UD-4 TO STRUCTURE

DI-3-2 DI-4-8 269'-15" CONC. SSP CLASS III REQ'D (3' COVER)
SILT TIGHT JOINT TYPE
INV(IN)=148.00 INV(OUT)=144.98

DI-3-4) I ST'D DI-3B REQ'D H=5.4' L=6' INV=144.50 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

DI-3-4 DI-3B-5 56'-15" CONC.SSP CLASS III REQ'D (4' COVER)
SILT TIGHT JOINT TYPE
INV(IN)=144,50 INV(OUT)=143,54

DI-3-7 | ST'D DI-3B REQ'D H=4.8' L=6' INV=144.68 CONNECT TO EXISTING 15" RCP ST'D ST-1 REQ'D CONNECT UD-4 TO STRUCTURE

DI-3-8 | ST'D DI-I REQ'D H=5.7' INV=145.67 CONNECT TO EXISTING 15" RCP ST'D ST-I REQ'D

DI-3-9 | ST'D DI-3B REQ'D H=3.8' L=4' INV=151.30 CONNECT UD-4 TO STRUCTURE

DI-3-9
DI-3-13

DI-3-10) I ST'D DI-3B REQ'D H=3.8' L=6' INV=151.10 CONNECT UD-4 TO STRUCTURE

DI-3-10 — DI-3-II 55'-15" CONC.SSP CLASS III REQ'D (2' COVER)
SILT TIGHT JOINT TYPE
INV(IN)=151,10 INV(OUT)=150.00

DI-3-II) I ST'D DI-3BB REQ'D H=8.8' L=10' INV=145.10 ST'D ST-1 REQ'D CONNECT UD-4 TO STRUCTURE

DI-3-II — DI-4-IO 164'-18" CONC.SSP CLASS III REQ'D (7' COVER)
SILT TIGHT JOINT TYPE

DI-3-12) I ST'D DI-3BB REQ'D H=8.6' L=6' INV=148.80 ST'D ST-1 RFQ'D

3-12 DI-3-15 53'-15" CONC.SSP CLASS III REQ'D (7' COVER)
SILT TIGHT JOINT TYPE

INV(IN)=148.80 INV(OUT)=148.20

INV(IN)=145.10 INV(OUT)=143.20

DI-3-13) I ST'D DI-3B REQ'D H=7.2' L=6' INV=149.40 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

DI-3-13 — DI-3-12 43'-15" CONC.SSP CLASS III REQ'D (7' COVER)
SILT TIGHT JOINT TYPE
INV(IN)=149.40 INV(OUT)=148.90

DI-3-14) I ST'D DI-3BB REQ'D H=8.8' L=8' INV=146.40 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

DI-3-14 DI-3-II) 98'-15" CONC.SSP CLASS III REQ'D (7' COVER) SILT TIGHT JOINT TYPE

INV(IN)=146.40 INV(OUT)=145.20

DI-3-15 | ST'D DI-3BB REQ'D

H=9.0' L=8' INV=148.10

ST'D ST-1 REQ'D

DI-3-15 — DI-3-14 | 158'-15" CONC.SSP CLASS | III REQ'D (7' COVER) SILT TIGHT JOINT TYPE

SILT TIGHT JOINT TYPE
INV(IN)=148.10 INV(OUT)=146.50

DI-3-16 I ST'D DI-3B REQ'D

CONNECT UD-4 TO STRUCTURE

CONNECT UD-4 TO STRUCTURE

DI-3-16

DI-3-15

IOO'-15" CONC.SSP CLASS III REQ'D (3' COVER)

SILT TIGHT JOINT TYPE

INV(IN)=153.67 INV(OUT)=152.70

H=5.0' L=4' INV=153.67

DRAINAGE DESCRIPTIONS

SHEET 3B

DI-3B-I) I ST'D DI-3C REQ'D
H=4.4' L=6' INV=145.0I
CONNECT TO EXISTING 15" RCP
DOUBLE 4 L.F 15" RCP REQ'D FOR CONNECTION
ST'D ST-I REQ'D
CONNECT UD-4 & CD-2 TO STRUCTURE

DI-3B-2) I ST'D DI-3B REQ'D H=3.9' L=6' INV=146.4I ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

DI-3B-2 DI-3B-I 41'-15" CONC.SSP CLASS III REQ'D (2' COVER)
SILT TIGHT JOINT TYPE

CONNECT UD-4 TO STRUCTURE

DI-3B-3 DI-3B-5 38'-24" CONC.SSP CLASS III REQ'D (3' COVER)

SILT TIGHT JOINT TYPE

INV(IN)=144.40 INV(OUT)=144.00

MH-3B-3A) 6.0 L.F OF ST'D MH-1 OR 2 REQ'D CONNECT TO EXISTING 18" RCP 4 L.F OF 18" RCP REQ'D FOR CONNECTION 1 ST'D MH-1 FRAME & COVER REQ'D

INV=145.34

MH-3B-3A DI-3B-3 16'-24" CONC.SSP CLASS III REQ'D (4' COVER)

SILT TIGHT JOINT TYPE

INV(IN)=145.34 INV(OUT)=144.50

DI-3B-4) | ST'D DI-2B REQ'D H=4.5' L=6' |NV=147.60 ST'D ST-1 REQ'D

DI-3B-4 DI-3B-2 48'-15" CONC.SSP CLASS III REQ'D (3' COVER)
SILT TIGHT JOINT TYPE

INV(IN)=147.60 INV(OUT)=146.51

DI-3B-5 I ST'D DI-3C REQ'D
H=6.0' L=8' INV=143.44
CONNECT TO EXISTING 30" & 21" RCP
4 L.F 30" RCP REQ'D FOR CONNECTION
4 L.F 21" RCP REQ'D FOR CONNECTION
ST'D IS-I REQ'D
ST'D ST-I REQ'D

CONNECT UD-4 & CD-2 TO STRUCTURE

DI-3B-6 | ST'D DI-3A REQ'D |
H=5.8' INV=144.13 |
CONNECT TO EXISTING TWO 15" & 21" RCP |
ST'D ST-1 REQ'D

CONNECT UD-4 TO STRUCTURE

DI-3B-7) I ST'D DI-3B REQ'D H=3.3' L=4' INV=140.00 CONNECT UD-4 TO STRUCTURE

-3B-7 DI-Ex005 12'-15" CONC.SSP CLASS III REQ'D (3' COVER)
SILT TYPE JOINT TYPE
INV(IN)=140.00 INV(OUT)=138.00

DI-EXOOS MODIFY EXISTING DITO ACCEPT PROPOSED 15" RCP

DI-3B-8) I ST'D DI-3B REQ'D
H=3.6' L=4' INV=140.00
CONNECT TO EXISTING 15" RCP
DOUBLE 4 L.F. 15" RCP REQ'D FOR CONNECTION
ST'D IS-I REQ'D
CONNECT UD-4 TO STRUCTURE

SHEET 4

DI-4-I | ST'D DI-3B REQ'D H=4.5' L=6' INV=140.66 ST'D ST-I REQ'D

CONNECT UD-4 & UD-2 TO STRUCTURE

DI-4-I DI-5-3 246'-15" CONC.SSP CLASS III REQ'D (3' COVER)

SILT TIGHT JOINT TYPE

INV(IN)=140.66 INV(OUT)=137.71

DI-4-2) | ST'D DI-3B REQ'D H=4.6' L=8' | INV=140.19 ST'D ST-| REQ'D

CONNECT UD-4 & UD-2 TO STRUCTURE

DI-5-5 198'-18" CONC.SSP CLASS III REQ'D (3' COVER)
SILT TIGHT JOINT TYPE

INV(IN)=140.19 INV(OUT)=138.00

HW-4-3) | ST'D 42" EW-2 REQ'D 31.88 TONS ST'D EC-I CLASS | REQ'D TYPE B INSTALLATION INV=106.00 MH-4-4) 28.8 L.F OF ST'D MH-2 REQ'D
I ST'D MH-I FRAME & COVER REQ'D
INV=106.50
2 ST'D SL-I REQ'D
ST'D ST-I REQ'D

14-4)—(HW-4-3) 58'-42" CONC.SSP CLASS III REQ'D (26' COVER)
SILT TIGHT JOINT TYPE
INV(IN)=106.50 INV(OUT)=106.00

DI-4-5 | ST'D DI-3BB REQ'D H=9.3' L=10' INV=139.90 ST'D ST-1 REQ'D CONNECT UD-4 TO STRUCTURE

DI-4-5 MH-4-12 | 154'-24" CONC.SSP CLASS | III REQ'D (8' COVER) SILT TIGHT JOINT TYPE INV(IN)=139.90 | INV(OUT)=137.25

14-4-6 I6.9 L.F OF ST'D MH-2 REQ'D
I ST'D MH-I FRAME & COVER REQ'D
CONNECT TO EXISTING 36" RCP
4 L.F 36" RCP REQ'D FOR CONNECTION
INV=130.00
ST'D ST-I REQ'D
I ST'D SL-I REQ'D
CONNECT UD-2 TO STRUCTURE

MH-4-6 MH-4-12 III'-36" CONC.SSP CLASS III REQ'D (16' COVER)
SILT TIGHT JOINT TYPE
INV(IN)=130,00 INV(OUT)=128,25

DI-4-7 | ST'D DI-3B REQ'D H=4,7' L=6' INV=143,27 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

DI-4-7 DI-4-2) 275'-15" CONC.SSP CLASS III REQ'D (3' COVER)
SILT TIGHT JOINT TYPE
INV(IN)=143,27 INV(OUT)=140,29

DI-4-8) I ST'D DI-3B REQ'D H=4.4' L=8' INV=144.88 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

DI-4-8 DI-4-7 | 135'-15" CONC.SSP CLASS | III REQ'D (3' COVER) SILT TIGHT JOINT TYPE | INV(IN)=144.88 | INV(OUT)=143.37

> DI-4-9 | ST'D DI-3BB REQ'D H=9.0' L=8' INV=141.90 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

DI-4-9
DI-4-5

I52'-24" CONC. SSP CLASS III REQ'D (7' COVER)

SILT TIGHT JOINT TYPE

INV(IN)=141.90 INV(OUT)=140.00

DI-4-10) I ST'D DI-3BB REQ'D H=8.9' L=10' INV=143.10 ST'D ST-1 REQ'D CONNECT UD-4 TO STRUCTURE

DI-4-10 — DI-4-9 92'-24" CONC.SSP CLASS III REQ'D (7' COVER)
SILT TIGHT JOINT TYPE
INV(IN)=143,10 INV(OUT)=142,00

DI-4-II) I ST'D DI-3B REQ'D H=4.3' L=6' INV=142.73 ST'D ST-I REQ'D CONNECT UD-2 TO STRUCTURE

DI-4-II DI-4-I 164'-15" CONC.SSP CLASS III REQ'D (3' COVER)
SILT TIGHT JOINT TYPE
INV(IN)=142.73 INV(OUT)=140.76

MH-4-12) 19.8 L.F OF ST'D MH-2 REQ'D I ST'D MH-1 FRAME & COVER REQ'D INV=127.00 I ST'D SL-1 REQ'D

ST'D ST-I REQ'D

MH-4-12

MH-4-4

24'-36" CONC. SSP CLASS III REQ'D (I7' COVER)

SILT TIGHT JOINT TYPE

INV(IN)=127.00 INV(OUT)=126.70

SHEET 5

HW-5-I I ST'D 24" ES-I REQ'D 6.00 TONS ST'D EC-I CLASS AI REQ'D TYPE A INSTALLATION INV=134.00

DI-5-2) I ST'D DI-3B REQ'D H=5.9' L=8' INV=134.50 ST'D ST-I REQ'D CONNECT UD-4 & UD-2 TO STRUCTURE

HW-5-I) 34'-24" CONC.SSP CLASS III REQ'D (4' COVER) SILT TIGHT JOINT TYPE INV(IN)=134.50 INV(OUT)=134.00 REVISED
STATE
ROUTE
ROUTE
PROJECT

VA. / OOOI-212-249, PE-IOI. 2E(I)

DESIGN FEATURES RELATING TO CONSTRUCTION
OR TO REGULATION AND CONTROL OF TRAFFIC
MAY BE SUBJECT TO CHANGE AS DEEMED
NECESSARY BY THE DEPARTMENT

VDOT Location & Design Richmond, Virginia HYDRAULIC ENGINEER Note: In addition to the visual inspection performed by the Department during the initial installation of storm sewer pipes and pipe culverts, a post installation visual/video camera inspection shall be conducted by the Contractor in accordance with the requirements of this specification and VTM 123 on all storm sewer pipe and a selected number of pipe culverts.

ST'D ST-I REQ'D
CONNECT UD-4 TO STRUCTURE

(DI-5-3) (DI-5-2) I50'-I5" CONC.SSP CLASS III REQ'D (3' COVER)

IST'D DI-3B REQ'D

H=4.7' L=8' INV=137.61

DI-5-4) I ST'D DI-3B REQ'D H=4.6' L=6' INV=135.90 ST'D ST-I REQ'D CONNECT UD-4 & UD-2 TO STRUCTURE

INV(IN)=137.61 INV(OUT)=135.77

SILT TIGHT JOINT TYPE

DI-5-4 DI-5-2 IO4'-18" CONC. SSP CLASS III REQ'D (4' COVER)
SILT TIGHT JOINT TYPE
INV(IN)=135.90 INV(OUT)=134.75

DI-5-5) I ST'D DI-3B REQ'D H=4.6' L=6' INV=137.90 ST'D ST-1 REQ'D CONNECT UD-4 TO STRUCTURE

1-5-5 DI-5-4 171'-18" CONC.SSP CLASS III REQ'D (3' COVER)
SILT TIGHT JOINT TYPE
INV(IN)=137.90 INV(OUT)=136.00

DI-5-6) I ST'D 3BB REQ'D H=9.3' L=10' INV=127.81 ST'D ST-1 REQ'D CONNECT UD-4 TO STRUCTURE

DI-5-6 MH-6-14 146'-24" CONC.SSP CLASS III REQ'D (8' COVER)
LE AK-RESISTANT JOINT TYPE
INV(IN)=127.81 INV(OUT)=125.58

DI-5-7 | ST'D DI-3B REQ'D H=4.3' L=8' INV=132.69 ST'D ST-1 REQ'D CONNECT UD-4 TO STRUCTURE

DI-5-7 DI-6-13 125'-18" CONC.SSP CLASS III REQ'D (3' COVER)
SILT TIGHT JOINT TYPE

INV(IN)=132.69 INV(OUT)=130.35

DI-5-8 I ST'D DI-5 REQ'D
TYPE E COVER REQ'D
TYPE III GRATE REQ'D
H=6.7' INV=134.00
ST'D ST-I REQ'D
CONNECT UD-4 TO STRUCTURE

DI-5-8

DI-5-7

43'-18" CONC.SSP CLASS III REQ'D (4' COVER)

SILT TIGHT JOINT TYPE

INV(IN)=134,00 INV(OUT)=132,69

5-9 8.0' STD. SWM-I REQ'D
TOP ELEV=136.00
BOTTOM ELEV=128.00
GRAVEL DIAPHRAGM, AS PER DEATIL
SEE SHEETS 21(3)-21(3A) FOR DETAILS
CONNECT UD TO RISER STRUCTURE

DI-5-9 DI-5-6 205'-24" CONC.SSP CLASS III REQ'D (7' COVER)
LEAK-RESISTANT JOINT TYPE
INV(IN)=131,00 INV(OUT)=127.81
34.6 CY OF CONCRETE CRADLE REQ'D

SHEET 6

DI-6-3 | ST'D 3BB REQ'D H=15.7' L=10' INV=117.28 | ST'D SL-1 REQ'D ST'D ST-1 REQ'D CONNECT UD-4 TO STRUCTURE

-6-3 DI-6-8 288'-24" CONC. SSP CLASS III REQ'D (I3' COVER)
LEAK-RESISTANT JOINT TYPE
INV(IN)=II7.28 INV(OUT)=II0.00

DI-6-5) I ST'D 3BB REQ'D H=8,2' L=10' INV=106,20 ST'D ST-1 REQ'D CONNECT UD-4 & UD-2 TO STRUCTURE

SSP:STORM SEWER PIPE
ST'D IS-I REQ'D FOR ALL STRS.
PIPE:SPEC.SECTION 232 (AASHTO MI7O
GASKET:SPEC.SECTION 212 (ASTM C443

0001-212-249

2E(I)

10:53:20 AM REVISED PROJECT MANAGER*Susie_Lue_(7.0.3)259-2918_NOV_A_District______*

SURVEYED BY, DATE *Leon E.T.reutle LS_(70.3)259-3224_7/17/13_____* DESIGN BY JMT_Engineering_(804)323-9900_______ SUBSURFACE UTILITY BY, DATE Leon E. Treutle LS (703)259-3224_7/17/13

(DI-6-5)—(DI-7-12) 228'-24" CONC.SSP CLASS III REQ'D (II' COVER) SILT TIGHT JOINT TYPE INV(IN)=106.20 INV(OUT)=85.00

DI-6-8 | ST'D DI-5 REQ'D TYPE E COVER REQ'D TYPE III GRATE REQ'D H=16.7' INV=103.00 IST'D SL-IREQ'D ST'D ST-I REQ'D

(DI-6-8)—(DI-7-8) 294'-24" CONC.SSP CLASS III REQ'D (I4' COVER) LEAK-RESISTANT JOINT TYPE /NV(/N)=/03.00 /NV(OUT)=92.70

> IST'D DI-5 REQ'D DI-6-9 TYPE E COVER REQ'D TYPE III GRATE REQ'D H=6.6′ INV=125.70 ST'D ST-I REQ'D

(DI-6-9) (DI-6-3) II'-15" CONC.SSP CLASS III REQ'D (6' COVER) SILT TIGHT JOINT TYPF INV(IN)=125.70 INV(OUT)=125.00

IST'D DI-3BB REQ'D DI-6-10 H=8.2' L=10' 1NV=114.45 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

(DI-6-10)—(DI-6-5) 133'-18" CONC.SSP CLASS III REQ'D (6' COVER) SILT TIGHT JOINT TYPE INV(IN)=II4.45 INV(OUT)=I06.20

> IST'D DI-3BB REQ'D DI-6-11 H=8.2' L=8' INV=121.30 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

)—(DI-6-IO) 146'-18" CONC.SSP CLASS III REQ'D (6' COVER) SILT TIGHT JOINT TYPE INV(IN)=121.30 INV(OUT)=114.45

(DI-6-12) I ST'D DI-3B REQ'D H=6.5' L=6' INV=127.13 ST'D ST-I REQ'D

CONNECT UD-4 TO STRUCTURE (DI-6-12)—(DI-6-11) 140'-18" CONC.SSP CLASS III REQ'D (6' COVER) SILT TIGHT JOINT TYPE INV(IN)=127.13 INV(OUT)=121.30

> IST'D DI-3B REQ'D DI-6-13 H=5.0' L=6' INV=130.35 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

(DI-6-13)—(DI-6-12) 109'-18" CONC.SSP CLASS III REQ'D (5' COVER) SILT TIGHT JOINT TYPE /NV(/N)=/30.35 /NV(OUT)=/27./3

> (MH-6-14) 14.3 L.F OF ST'D MH-1 REQ'D IST'D MH-IFRAME & COVER REQ'D CONNECT TO EXISTING 15" RCP 4 L.F 15" RCP REQ'D FOR CONNECTION INV=120.38 IST'D SL-IREQ'D ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

(MH-6-14)—(DI-6-3) 142'-24" CONC.SSP CLASS III REQ'D (13' COVER) LEAK-RESISTANT JOINT TYPE INV(IN)=120.38 INV(OUT)=117.28

SHEET 6B

(DI-6B-I) I ST'D DI-5 REQ'D TYPE E COVER REQ'D TYPE III GRATE REQ'D H=3.8' INV=III.70

(DI-6B-I)—(DI-6B-2) 42'-24" CONC.SSP CLASS III REQ'D (IO' COVER) SILT TIGHT JOINT TYPE /NV(/N)=///.70 /NV(OUT)=/08.09

> (DI-6B-2) I ST'D DI-3BB REQ'D H=12.3' L=6' INV=108.09 CONNECT UD-4 TO STRUCTURE CONNECT TO EXISTING 24" PIPE

(DI-6B-2)—(MH-6B-4) 129'+-, EXISTING 24" SSP /// INV(IN)=108.09 INV(OUT)=105.77

(DI-6B-2A) I ST'D DI-3B REQ'D H=4.0' L=4' |NV=|13.70 CONNECT UD-4 TO STRUCTURE

(DI-6B-2A)—(DI-6B-7) 32'-15" CONC.SSP CLASS III REQ'D (4' COVER) SILT TIGHT JOINT TYPE /NV(/N)=//3.70 /NV(OUT)=///.50

DRAINAGE DESCRIPTIONS

(DI-6B-3) I ST'D DI-3B REQ'D H=3.9' L=6' |NV=|09.00 CONNECT UD-4 TO STRUCTURE

- (MH-6B-4) 32'-15" CONC.SSP CLASS III REQ'D (4' COVER) SILT TIGHT JOINT TYPE /NV(/N)=/09.00 /NV(OUT)=/07.50

(MH-6B-4) 6.2 L.F OF ST'D MH-1 REQ'D IST'D MH-IFRAME & COVER REQ'D CONNECT TO EXISTING 24" PIPE

(DI-6B-5) I ST'D DI-3A REQ'D H=4.0′ INV=108.50 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE -(MH-6B-4) 12'-15" CONC.SSP CLASS III REQ'D (3' COVER) SILT TIGHT JOINT TYPE

/NV(/N)=/08.50 /NV(OUT)=/08.00 IST'D DI-3A REQ'D H=3.9′ INV=112.40 ST'D ST-I REQ'D

CONNECT UD-4 TO STRUCTURE -(DI-6B-7) 37'-15" CONC.SSP CLASS III REQ'D (5' COVER) SILT TIGHT JOINT TYPE /NV(/N)=//2.40 /NV(OUT)=///.00

(DI-6B-7) I ST'D DI-3B REQ'D H=6.4' L=4' |NV=||1.00 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

-(DI-6B-9) 56'-15" CONC.SSP CLASS III REQ'D (5' COVER) SILT TIGHT JOINT TYPE /NV(/N)=///.00 /NV(OUT)=/08.00

> DI-6B-8) I ST'D DI-3B REQ'D H=3.9' L=4' INV=104.60 CONNECT UD-4 TO STRUCTURE

-(DI-6B-9) 20'-15" CONC.SSP CLASS III REQ'D (6' COVER) SILT TIGHT JOINT TYPE /NV(/N)=/04.60 /NV(OUT)=/04.00

> (DI-6B-9) I ST'D DI-3B REQ'D H=7.7' L=4' INV=104.00 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

(DI-6B-ID) 69'-15" CONC.SSP CLASS III REQ'D (6' COVER) SILT TIGHT JOINT TYPE /NV(/N)=104.00 /NV(OUT)=101.50

> (DI-6B-IO) I ST'D DI-7 REQ'D H=3.0' INV=101.50 CONNECT UD-4 TO STRUCTURE

(DI-6B-12) 13'-15" CONC.SSP CLASS III REQ'D (2' COVER) SILT TIGHT JOINT TYPE /NV(/N)=/0/.50 /NV(OUT)=/0/.00

(DI-6B-II) I ST'D DI-3B REQ'D H=3.9' L=4' INV=99.20 CONNECT UD-4 TO STRUCTURE

(DI-6B-13) 56'-15" CONC.SSP CLASS III REQ'D (2' COVER) SILT TIGHT JOINT TYPE

/NV(/N)=99.20 /NV(OUT)=97.00 (DI-6B-12) I ST'D DI-3B REQ'D

H=6.8' L=4' INV=98.00 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

(DI-6B-12)—(DI-Ex062) 60'-15" CONC.SSP CLASS III REQ'D (5' COVER) SILT TIGHT JOINT TYPE /NV(/N)=98.00 /NV(OUT)=96.00

> (DI-6B-I3) I ST'D DI-3B REQ'D H=4.0′ L=4′ INV=95.10 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

(DI-6B-13)—(DI-Ex062) 58'-15" CONC.SSP CLASS III REQ'D (3' COVER) SILT TIGHT JOINT TYPE /NV(/N)=95./0 /NV(OUT)=94.00 SHEET 7

> ST'D DI-3C REQ'D H=3.7' L=6' INV=61.50 ST'D ST-I REQ'D CONNECT UD-4 & CD-2 TO STRUCTURE

(DI-7-22) 21'-15" CONC.SSP CLASS III REQ'D (4' COVER) /NV(/N)=61.50 /NV(OUT)=60.50 DI-7-3 | ST'D DI-3BB REQ'D H=10.5' L=10' 1NV=59.40

CONNECT UD-4 TO STRUCTURE

ST'D ST-I REQ'D

(DI-7-3) (DI-7-10) 118'-36" CONC.SSP CLASS III REQ'D (9' COVER) LEAK-RESISTANT JOINT TYPE INV(IN)=59.40 INV(OUT)=58.00

> IST'D DI-3BB REQ'D H=8.4' L=8' INV=69.00 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

— (DI-7-3) IOI'-24" CONC.SSP CLASS III REQ'D (6' COVER) SILT TIGHT JOINT TYPE /NV(/N)=69.00 /NV(OUT)=65.50

IST'D DI-3B REQ'D H=12.3' L=12' INV=72.00 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

-(DI-7-6) 94'-24" CONC.SSP CLASS III REQ'D (IO' COVER) SILT TIGHT JOINT TYPE /NV(/N)=72.00 /NV(OUT)=69.00

> IST'D 3BB REQ'D H=15.3' L=14' INV=82.70 IST'D SL-IREQ'D ST'D ST-I REQ'D CONNECT UD-4 & UD-2 TO STRUCTURE

—(DI-7-20) 159'-24" CONC.SSP CLASS III REQ'D (13' COVER) LEAK-RESISTANT JOINT TYPE INV(IN)=82.70 INV(OUT)=77.82

MH-7-9 13.3 L.F OF ST'D MH-2 REQ'D IST'D MH-IFRAME & COVER REQ'D /NV=47.25 IST'D SL-IREQ'D ST'D ST-I REQ'D

(MH-8-8A) 651'-48" CONC.SSP CLASS III REQ'D (10' COVER) LEAK-RESISTANT JOINT TYPE INV(IN)=47.25 INV(OUT)=36.00

> IST'D DI-4BB REQ'D H=12.6' L=12' 1NV=57.90 ST'D ST-I REQ'D IST'D SL-IREQ'D CONNECT UD-4 TO STRUCTURE

MH-7-9) 139'-36" CONC.SSP CLASS III REQ'D (9' COVER) LEAK-RESISTANT JOINT TYPE INV(IN)=57.90 INV(OUT)=55.00

>)/ST'D 3BB REQ'D H=11.5' L=10' 1NV=75.00 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

(DI-7-10) 231'-24" CONC.SSP CLASS III REQ'D (IO' COVER) SILT TIGHT JOINT TYPE /NV(/N)=75.00 /NV(OUT)=57.90

DI-7-12 | IST'D 3BB REQ'D H=13.7' L=12' INV=85.00 ST'D ST-I REQ'D ST'D SL-I REQ'D CONNECT UD-4 & UD-2 TO STRUCTURE

— DI-7-II) 177'-24" CONC.SSP CLASS III REQ'D (II' COVER) SILT TIGHT JOINT TYPE /NV(/N)=85.00 /NV(OUT)=75.00

> DI-7-17 | MODIFIED DI-2DD REQ'D H=10.6' L=5' 1NV=50.58 CONNECT TO EXISTING 48" RCP 4 L.F OF 48" RCP REQ'D FOR CONNECTION ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

-(MH-7-9) 55'-48" CONC.SSP CLASS III REQ'D (8' COVER) SILT TIGHT JOINT TYPE /NV(/N)=50.58 /NV(OUT)=49.47 225.4 CY MINOR STRUCTURE EXCAVATION

STATE SHEET NO. STATE ROUTE PROJECT 0001-212-249, PE-101 | 2E(2) VA. DESIGN FEATURES RELATING TO CONSTRUCTION

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Plotted By: ong

OR TO REGULATION AND CONTROL OF TRAFFIC MAY BE SUBJECT TO CHANGE AS DEEMED NECESSARY BY THE DEPARTMENT

VDOT Location & Design Richmond, Virginia HYDRAULIC ENGINEER

Note: In addition to the visual inspection performed by the Department during the initial installation of storm sewer pipes and pipe culverts, a post installation visual/video camera inspection shall be conducted by the Contractor in accordance with the requirements of this specification and VTM 123 on all storm sewer pipe and a selected number of pipe culverts.

DI-7-18 | I ST'D DI-5 REQ'D TYPE E COVER REQ'D TYPE III GRATE REQ'D H=3.7' INV=57.00 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

(DI-7-26) 8'-15" CONC.SSP CLASS III REQ'D (4' COVER) SILT TIGHT JOINT TYPE /NV(/N)=57.00 /NV(OUT)=56.50

(DI-7-20) I ST'D DI-5 REQ'D TYPE E COVER REQ'D TYPE III GRATE REQ'D H=13.7' INV=72.98 IST'D SL-IREQ'D ST'D ST-I REQ'D

(DI-7-7) 25'-24" CONC.SSP CLASS III REQ'D (II' COVER) LEAK-RESISTANT JOINT TYPE /NV(/N)=72.98 /NV(OUT)=72.00

(DI-7-21) I ST'D DI-3BB REQ'D H=9.6' L=10' 1NV=59.90 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

(DI-7-3) 35'-18" CONC.SSP CLASS III REQ'D (9' COVER) /NV(/N)=59.90 /NV(OUT)=59.40

(DI-7-22) I ST'D DI-3B REQ'D H=5.6' L=4' INV=60.30 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

— (DI-7-21) 32'-18" CONC.SSP CLASS III REQ'D (7' COVER) /NV(/N)=60.30 /NV(OUT)=60.00

(DI-7-23) I ST'D DI-3B REQ'D ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

(DI-7-23)—(DI-7-22) 30'-18" CONC.SSP CLASS III REQ'D (4' COVER) SILT TIGHT JOINT TYPE /NV(/N)=60.60 /NV(0UT)=60.40

> (DI-7-24) I ST'D DI-3B REQ'D H=3.9' L=4' INV=61.00 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

(DI-7-24)—(DI-7-23) 17'-15" CONC.SSP CLASS III REQ'D (4' COVER) SILT TIGHT JOINT TYPE /NV(/N)=61.00 /NV(OUT)=60.70

> (DI-7-25) I ST'D DI-3B REQ'D H=6.8' L=6' INV=57.10 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

(DI-7-25)—(DI-8-3) 74'-15" CONC.SSP CLASS III REQ'D (8' COVER) SILT TIGHT JOINT TYPE /NV(/N)=57.10 /NV(OUT)=55.20

> (DI-7-26) I ST'D DI-3B REQ'D H=6.2' L=6' INV=56.50 ST'D ST-I REQ'D

CONNECT UD-4 TO STRUCTURE (DI-7-26)—(DI-7-17) 34'-15" CONC.SSP CLASS III REQ'D (9' COVER) SILT TIGHT JOINT TYPE /NV(/N)=56.50 /NV(OUT)=50.58

SHEET 7C

(DI-7C-2) I ST'D DI-3B REQ'D H=5.5' L=8' INV=57.06 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE (DI-7C-2)—(DI-EX7C-)) 141'-15" CONC.SSP CLASS III REQ'D (4' COVER) SILT TIGHT JOINT TYPE

INV(IN)=57.06 INV(OUT)=55.30 SSP: STORM SEWER PIPE ST'D IS-I REQ'D FOR ALL STRS. PIPE: SPEC. SECTION 232 (AASHTO MI70)

GASKET: SPEC. SECTION 212 (ASTM C443) SHEET NO. PROJECT 2E(2)0001-212-249

PROJECT MANAGER<u>Susie Lue (7.0.3)259-2918_NOVA_District</u>_____ SURVEYED BY, DATE Leon E.Treutle LS_(70.3)259-3224 7/17/13_____ DESIGN BY JMT_ Engineering_(804)_323-9900______ SUBSURFACE UTILITY BY, DATE Lean F. Treutle LS (703)259-3224 7/17/13

SHEET 8

- IST'D DI-3B REQ'D H=7.9' L=8' INV=40.00 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE
- (DI 9-5) 247'-15" CONC.SSP CLASS III REQ'D (IO' COVER) DI-8-I SILT TIGHT JOINT TYPE INV(IN)=40.00 INV(OUT)=34.00
 - (DI-8-2) I ST'D DI-3B REQ'D H=4.5' L=10' INV=46.10 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE
- (DI-8-1) 250'-15" CONC.SSP CLASS III REQ'D (6' COVER) SILT TIGHT JOINT TYPF INV(IN)=46.10 INV(OUT)=40.00
 - (DI-8-3) I ST'D DI-3B REQ'D H=3.7' L=10' INV=55.10 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE
- (DI-8-2) 242'-15" CONC.SSP CLASS III REQ'D (2' COVER) DI-8-3 SILT TIGHT JOINT TYPE INV(IN)=55.10 INV(OUT)=47.50
 - H=6.0' L=6' INV=41.21 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE
- (DI-9-9) 195'-18" CONC.SSP CLASS III REQ'D (5' COVER) SILT TIGHT JOINT TYPE INV(IN)=41.21 INV(OUT)=37.76
 - H=4.4' L=8' INV=45.40 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE
- (DI-8-8) 194'-15" CONC.SSP CLASS III REQ'D (4' COVER) SILT TIGHT JOINT TYPE
- INV(IN)=45.40 INV(OUT)=42.93 DI-8-7) I ST'D DI-3B REQ'D H=4.4' L=12' INV=48.50

ST'D ST-I REQ'D

- CONNECT UD-4 TO STRUCTURE (DI-8-6) 139'-15" CONC.SSP CLASS III REQ'D (3' COVER) SILT TIGHT JOINT TYPE
- INV(IN)=48.50 INV(OUT)=45.50 DI-8-8 | IST'D DI-3B REQ'D
- ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE (DI-8-5) 98'-15" CONC.SSP CLASS III REQ'D (4' COVER) DI-8-8 SILT TIGHT JOINT TYPE

H=5.2' L=6' INV=42.83

- INV(IN)=42.83 INV(OUT)=41.31 (MH-8-8A) 10.8 L.F OF ST'D MH-2 REQ'D IST'D MH-IFRAME & COVER REQ'D
- INV=36**.**00 -(MH-9-II) 571'-48" CONC.SSP CLASS III REQ'D (8' COVER) MH-8-8A)-LEAK-RESISTANT JOINT TYPE INV(IN)=36.00 INV(OUT)=25.50
 - 1857.3 CY MINOR STRUCTURE EXCAVATION (DI-8-II) I ST'D DI-I REQ'D H=2.8' INV=41.21
- -(MH-8-8A) 66'-15" CONC.SSP CLASS III REQ'D (7' COVER) DI-8-11 SILT TIGHT JOINT TYPE INV(IN)=41.21 INV(OUT)=39.00
 - 80'-18" CONC.PIPE CLASS III REQ'D (I' COVER) INV (IN)=45.50 INV (OUT)=45.00. 2 STD.18" ES-I REQ'D 3.38 TONS ST'D EC-I CLASS AI REQ'D. TYPE A INSTALLATION

SHEET 9

IST'D DI-2BB REQ'D (DI-9-2) H=10.4' L=6' INV=19.50 ST'D ST-I REQ'D CONNECT UD-4 & UD-2 TO STRUCTURE

DRAINAGE DESCRIPTIONS

- -(DI-10-13) 152'-24" CONC.SSP CLASS III REQ'D (8' COVER) SILT TIGHT JOINT TYPE INV(IN)=19.50 INV(OUT)=17.50
 - (DI-9-3) I ST'D DI-3BB REQ'D H=10.7' L=10' INV=21.50 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE
- (DI-9-2) 73'-24" CONC.SSP CLASS III REQ'D (8' COVER) SILT TIGHT JOINT TYPE INV(IN)=21.50 INV(OUT)=19.50
 - IST'D 3BB REQ'D DI-9-4 H=12.5' L=10' INV=27.00 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE
- DI-9-8 51'-24" CONC.SSP CLASS III REQ'D (IO' COVER) SILT TIGHT JOINT TYPE INV(IN)=27.00 INV(OUT)=25.80
 - DI-9-8 II.7 LF OF ST'D MH-IOR MH-2 REQ'D IST'D MH-IFRAME & COVER REQ'D INV=25**.**80 ST'D ST-I REQ'D CONNECT EXISTING PIPE TO MANHOLE CONNECT UD-4 TO STRUCTURE
- (DI-9-3) 187'-24" CONC.SSP CLASS III REQ'D (10' COVER) SILT TIGHT JOINT TYPE INV(IN)=25.80 INV(OUT)=21.50
 - IST'D 3BB REQ'D H=II.3' L=8' INV=34.00 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE
- DI-9-4) 279'-15" CONC.SSP CLASS III REQ'D (II' COVER) SILT TIGHT JOINT TYPE INV(IN)=34.00 INV(OUT)=27.00
 - (DI-9-6) I ST'D DI-3B REQ'D H=7.4' L=8' INV=27.47 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE
- (DI-IO-I2) 225'-18" CONC.SSP CLASS III REQ'D (6' COVER) SILT TIGHT JOINT TYPE INV(IN)=27.47 INV(OUT)=23.40
 - DI-9-7) I ST'D DI-3B REQ'D H=6.4' L=10' INV=32.49 ST'D ST-I REQ'D CONNECT UD-4 & UD-2 TO STRUCTURE
- DI-9-6) 129'-18" CONC.SSP CLASS III REQ'D (5' COVER) SILT TIGHT JOINT TYPE INV(IN)=32.49 INV(OUT)=29.85
 - DI-9-9 | ST'D DI-3B REQ'D H=7.2' L=8' INV=37.66 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE
- DI-9-7) 270'-18" CONC.SSP CLASS III REQ'D (5' COVER) DI-9-9 SILT TIGHT JOINT TYPE INV(IN)=37.66 INV(OUT)=32.59
 - (MH-9-10) II.3 L.F OF ST'D MH-2 REQ'D IST'D MH-IFRAME & COVER REQ'D 1NV=16**.**70 IST'D SL-IREQ'D
- ST'D ST-I REQ'D (MH-10-4) 292'-48" CONC.SSP CLASS III REQ'D (8' COVER) LEAK-RESISTANT JOINT TYPE INV(IN)=16.70 INV(OUT)=12.00 1130.6 CY MINOR STRUCTURE EXCAVATION
 - MH-9-11 13.7 L.F OF ST'D MH-2 REQ'D IST'D MH-IFRAME & COVER REQ'D CONNECT TO EXISTING 24" RCP 4 L.F OF 24" RCP REQ'D FOR CONNECTION INV=20.50
- ST'D ST-I REQ'D (MH-9-10) 196'-48" CONC.SSP CLASS III REQ'D (10' COVER) LEAK-RESISTANT JOINT TYPE INV(IN)=20.50 INV(OUT)=16.70 881.8 CY MINOR STRUCTURE EXCAVATION

SHEET 10

IST'D SL-IREQ'D

- (HW-10-2) | ST'D 60" EW-2 REQ'D 66.00 TONS ST'D EC-ICLASS I REQ'D TYPE B INSTALLATION 1NV=10**.**50
- (MH-10-3) 9.3 L.F OF ST'D MH-2 REQ'D *IST'D MH-IFRAME & COVER REQ'D* INV=10.60 ST'D ST-I REQ'D
- -(HW-10-2) 12'-60" CONC.SSP CLASS III REQ'D (5' COVER) (MH-10-3) LEAK-RESISTANT JOINT TYPE INV(IN)=10.60 INV(OUT)=10.50 65.0 CY MINOR STRUCTURE EXCAVATION

- (MH-10-4) IIJ L.F OF ST'D MH-2 REQ'D IST'D MH-IFRAME & COVER REQ'D INV=12.00, ST'D ST-I REQ'D CONNECT UD-2 TO STRUCTURE
- -(MH-10-3) 149'-60" CONC.SSP CLASS III REQ'D (7' COVER) LEAK-RESISTANT JOINT TYPE INV(IN)=12.00 INV(OUT)=10.60 571.5 CY MINOR STRUCTURE EXCAVATION
 - 8.5' ST'D PRECAST MH-2 REQ'D IST'D PRECAST T-DI-3 TOP REQ'D L=8' INV=13.14 ST'D ST-I REQ'D FOR MANHOLE BASE

CONNECT UD-4 & UD-2 TO STRUCTURE

- (MH-10-4) 110'-60" CONC.SSP CLASS III REQ'D (7' COVER) LEAK-RESISTANT JOINT TYPE INV(IN)=13.14 INV(OUT)=12.00 428.6 CY MINOR STRUCTURE EXCAVATION
 - IST'D 24" ES-IREQ'D 6.00 TONS ST'D EC-I CLASS AI REQ'D TYPE A INSTALLATION /NV=/1**.**75
 - 7.3' STD. SWM-I REQ'D TOP ELEV=17.50 BOTTOM ELEV=10.20 GRAVEL DIAPHRAGM, AS PER DEATIL SEE SHEETS 21(5)-21(5A) FOR DETAILS CONNECT BMP UD TO RISER STRUCTURE
- -(HW-10-6) 130'-24" CONC.SSP CLASS III REQ'D (4' COVER) LEAK-RESISTANT JOINT TYPE INV(IN)=13.20 INV(OUT)=12.50 21.8 CY OF CONCRETE CRADLE REQ'D
 - (HW-10-9) | ST'D 60" EW-2 REQ'D /NV=16**.**00
- (DI-10-5) 16'-60" CONC.SSP CLASS III REQ'D (2' COVER) SILT TIGHT JOINT TYPE INV(IN)=16.00 INV(OUT)=15.81 49.3 CY MINOR STRUCTURE EXCAVATION
 - (HW-10-10) | ST'D 24" ES-1 REQ'D 8.00 TONS ST'D EC-ICLASS I REQ'D TYPE A INSTALLATION INV=17**.**00
 - IST'D DI-3B REQ'D H=5.2' L=8' INV=17.62 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE
- (DI-IO-II)—(HW-IO-IO) 36'-24" CONC.SSP CLASS III REQ'D (3' COVER) SILT TIGHT JOINT TYPE INV(IN)=17.62 INV(OUT)=17.00
 - (DI-10-12) I ST'D DI-3B REQ'D H=4.6' L=8' INV=23.30 ST'D ST-I REQ'D CONNECT UD-4 & UD-2 TO STRUCTURE
- DI-IO-II) 297'-24" CONC.SSP CLASS III REQ'D (2' COVER) SILT TIGHT JOINT TYPE INV(IN)=23.30 INV(OUT)=18.40
 - IST'D DI-3BB REQ'D (DI-10-13) H=8.6' L=6' INV=17.50 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE
- 172'-30" CONC.SSP CLASS III REQ'D (7' COVER) (DI-10-13 LEAK-RESISTANT JOINT TYPE INV(IN)=17.50 INV(OUT)=13.30
 - SHEET II I ST'D I5" ES-I REQ'D 2.34 TONS ST'D EC-I CLASS AI REQ'D TYPE A INSTALLATION
 - INV=12.35 IST'D DI-3C REQ'D
- H=4.7' L=6' INV=12.51 IST'D ST-IREQ'D CONNECT UD-4 & CD-2 TO STRUCTURE) 15'-15" CONC.SSP CLASS III REQ'D (3' COVER)
 - SILT TIGHT JOINT TYPE INV(IN)=12.51 INV(OUT)=12.35 IST'D DI-3B REQ'D
- IST'D ST-IREQ'D CONNECT UD-4 TO STRUCTURE

H=4.3' L=4' INV=12.90

- `\ 47'-15" CONC.SSP CLASS III REQ'D (3' COVER) SILT TIGHT JOINT TYPE
 - INV(IN)=12.90 INV(OUT)=12.65 IST'D DI-3B REQ'D H=4.5' L=4' INV=12.76

IST'D ST-IREQ'D

- CONNECT UD-4 TO STRUCTURE
 - (DI-II-2) 48'-15" CONC.SSP CLASS III REQ'D (3' COVER) SILT TIGHT JOINT TYPE INV(IN)=12.76 INV(OUT)=12.51

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DESIGN FEATURES RELATING TO CONSTRUCTION OR TO REGULATION AND CONTROL OF TRAFFIC MAY BE SUBJECT TO CHANGE AS DEEMED NECESSARY BY THE DEPARTMENT

VDOT Location & Design Richmond, Virginia HYDRAULIC ENGINEER

Note: In addition to the visual inspection performed by the Department during the initial installation of storm sewer pipes and pipe culverts, a post installation visual/video camera inspection shall be conducted by the Contractor in accordance with the requirements of this specification and VTM 123 on all storm sewer pipe and a selected number of pipe culverts.

- IST'D DI-2B REQ'D H=4.2' L=6' INV=13.25 IST'D ST-IREQ'D CONNECT UD-4 TO STRUCTURE
- DI-II-4 49'-15" CONC.SSP CLASS III REQ'D (3' COVER) SILT TIGHT JOINT TYPE INV(IN)=13.25 INV(OUT)=12.94
 - (HW-II-6) I ST'D 15" ES-I REQ'D 3.12 TONS ST'D EC-I CLASS I REQ'D TYPE A INSTALLATION INV=10**.**58
 -) I ST'D DI-3C REQ'D H=5.0' L=6' INV=12.20 IST'D ST-IREQ'D CONNECT UD-4 & CD-2 TO STRUCTURE
- (HW-II-6) 53'-15" CONC.SSP CLASS III REQ'D (3' COVER) SILT TIGHT JOINT TYPE INV(IN)=12.20 INV(OUT)=10.58
 - DI-II-8 I ST'D DI-3B REQ'D H=4.4' L=4' INV=12.80 IST'D ST-IREQ'D CONNECT UD-4 TO STRUCTURE
- DI-II-7 48'-15" CONC.SSP CLASS III REQ'D (3' COVER) SILT TIGHT JOINT TYPE INV(IN)=12.80 INV(OUT)=12.30
 - IST'D DI-3B REQ'D H=4.3′ L=6′ INV=13.46 IST'D ST-IREQ'D
- \ 102'-15" CONC.SSP CLASS III REQ'D (3' COVER) SILT TIGHT JOINT TYPE INV(IN)=13.46 INV(OUT)=12.90
 - (DI-II-IO) I ST'D DI-3B REQ'D H=4.3' L=8' INV=14.60 IST'D ST-IREQ'D CONNECT UD-4 & UD-2 TO STRUCTURE
- DI-II-9 103'-15" CONC.SSP CLASS III REQ'D (3' COVER) SILT TIGHT JOINT TYPE INV(IN)=14.60 INV(OUT)=13.56
 - \ I ST'D DI-2B REQ'D H=4.3' L=6' INV=12.87 IST'D ST-IRFQ'D CONNECT UD-4 TO STRUCTURE
- DI-II-7) 31'-15" CONC.SSP CLASS III REQ'D (3' COVER) SILT TIGHT JOINT TYPE INV(IN)=12.87 INV(OUT)=12.30
 - (HW-II-I2) I ST'D 24" ES-I REQ'D 6.00 TONS ST'D EC-I CLASS AI REQ'D TYPE A INSTALLATION INV=12.07
- DI-II-I3) I ST'D DI-3B REQ'D H=5.9' L=10' INV=12.57 IST'D ST-IREQ'D CONNECT UD-4 TO STRUCTURE
- (HW-II-I2) 48'-24" CONC.SSP CLASS III REQ'D (3' COVER) SILT TIGHT JOINT TYPE INV(IN)=12.57 INV(OUT)=12.07
- H=5.6' L=10' INV=13.38 IST'D ST-IREQ'D CONNECT UD-4 TO STRUCTURE DI-II-I3) 68'-24" CONC.SSP CLASS III REQ'D (3' COVER)
- SILT TIGHT JOINT TYPE INV(IN)=13.38 INV(OUT)=12.67 (DI-II-15) I ST'D DI-3C REQ'D H=4.4' L=6' INV=13.73

DI-II-14) I ST'D DI-3B REQ'D

- IST'D ST-IREQ'D CONNECT UD-4 & CD-2 TO STRUCTURE
- (DI-II-I4) 22'-24" CONC.SSP CLASS SSP: STORM SEWER PIPE III REQ'D (3' COVER) ST'D IS-I REQ'D FOR ALL STRS. SILT TIGHT JOINT TYPE PIPE: SPEC. SECTION 232 (AASHTO MI70 INV(IN)=13.73 INV(OUT)=13.48 GASKET: SPEC.SECTION 212 (ASTM C443 PROJECT SHEET NO. 2E(3) 0001-212-249

PLANS

DI-11-16

SHEET II CONTINUED

(DI-II-16) | ST'D DI-3B REQ'D H=3.8' L=10' INV=14.89 CONNECT UD-4 & U.

CONNECT UD-4 & UD-2 TO STRUCTURE

DI-II-15 47'-15" CONC.SSP CLASS III REQ'D (2' COVER)
SILT TIGHT JOINT TYPE

INV(IN)=14.89 INV(OUT)=14.40

DI-II-I7 | ST'D DI-I REQ'D
H=2.5' INV=14.00

I-I7 — DI-II-I5 14'-24" CONC.SSP CLASS III REQ'D (2' COVER)
SILT TIGHT JOINT TYPE
INV(IN)=13.71 INV(OUT)=13.83

HW-II-I8) I ST'D 18" ES-I REQ'D 3.38 TONS ST'D EC-I CLASS AI REQ'D TYPE A INSTALLATION INV=13.59

MH-II-19 7.6 L.F. OF ST'D MH-I REQ'D
I ST'D MH-I FRAME & COVER REQ'D
CONNECT TO EXISTING 15" & 18" RCP
4 L.F. OF 15" RCP REQ'D FOR CONNECTION
4 L.F. OF 18" RCP REQ'D FOR CONNECTION
INV=13.83
ST'D ST-I REQ'D

MH-II-19 HW-II-18 22'-18" CONC. SSP CLASS III REQ'D (7' COVER)
SILT TIGHT JOINT TYPE
INV(IN)=13.83 INV(OUT)=13.59

DI-II-20 | ST'D DI-2B REQ'D H=3.6' L=6' INV=15.20 CONNECT UD-4 TO STRUCTURE

DI-II-20 — DI-I2-3 329'-I5" CONC.SSP CLASS III REQ'D (7' COVER)
SILT TIGHT JOINT TYPE
INV(IN)=15,20 INV(OUT)=10,20

SHEET 12

DI-12-I | ST'D DI-2B REQ'D H=3.6' L=4' INV=15.50 CONNECT UD-4 TO STRUCTURE

DI-12-I

DI-12-2

25I'-15" CONC.SSP CLASS III REQ'D (6' COVER)

SILT TIGHT JOINT TYPE

INV(IN)=15.50 INV(OUT)=11.50

DI-12-2) I ST'D DI-3BB REQ'D
H=9.4' L=4' INV=9.30
ST'D ST-I REQ'D

CONNECT UD-4 TO STRUCTURE

HW-12-11 36'-24" CONC.SSP CLASS III REQ'D (7' COVER)
SILT TIGHT JOINT TYPE

INV(IN)=9.30 INV(OUT)=9.00

DI-12-3 | ST'D DI-3BB REQ'D H=8.6' L=6' INV=10.00 ST'D ST-1 REQ'D

CONNECT UD-4 TO STRUCTURE

-DI-12-2 95'-18" CONC.SSP CLASS III REQ'D (8' COVER)

SILT TIGHT JOINT TYPE

INV(IN)=10.00 INV(OUT)=9.30

DI-12-4) I ST'D DI-3B REQ'D H=8.0' L=4' INV=10.10 ST'D ST-1 REQ'D CONNECT UD-4 TO STRUCTURE

DI-12-4 DI-12-2 97'-15" CONC. SSP CLASS III REQ'D (8' COVER)
SILT TIGHT JOINT TYPE
INV(IN)=10,10 INV(OUT)=9,30

DI-12-5) I ST'D DI-3B REQ'D H=6.4' L=4' INV=II.JO I ST'D ST-I REQ'D

CONNECT UD-4 & UD-2 TO STRUCTURE

- DI-12-4 | 130'-15" CONC.SSP CLASS | III REQ'D (6' COVER)

SILT TIGHT JOINT TYPE

INV(IN)=II.JO INV(OUT)=I0.JO

DI-12-6 | I ST'D DI-3B REQ'D | H=5.J' L=4' INV=II.90 | ST'D ST-I REQ'D

DI-12-5

CONNECT UD-4 & UD-2 TO STRUCTURE

12-5) 104'-15" CONC.SSP CLASS III REQ'D (5' COVER

DI-12-6 DI-12-5 104'-15" CONC. SSP CLASS III REQ'D (5' COVER)
SILT TIGHT JOINT TYPE
INV(IN)=II.90 INV(OUT)=II.10

DI-12-7 I ST'D DI-3C REQ'D

12-7) I ST'D DI-3C REQ'D H=4.5' L=6' INV=12.50 ST'D ST-I REQ'D CONNECT UD-4 & CD-2 TO STRUCTURE

DRAINAGE DESCRIPTIONS

DI-12-7 DI-12-6 65'-15" CONC.SSP CLASS III REQ'D (4' COVER)
SILT TIGHT JOINT TYPE
INV(IN)=12.50 INV(OUT)=11.90

DI-12-8) I ST'D DI-3B REQ'D H=6.0' L=6' INV=II.80 I ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

DI-12-8 DI-12-3 238'-15" CONC.SSP CLASS III REQ'D (7' COVER)
SILT TIGHT JOINT TYPE
INV(IN)=II.80 INV(OUT)=IO.00

DI-12-9 | ST'D DI-3B REQ'D H=4.5' L=4' INV=12.70 ST'D ST-1 REQ'D CONNECT UD-4 & UD-2 TO STRUCTURE

DI-12-9
DI-12-8
II8'-15" CONC.SSP CLASS III REQ'D (4' COVER)
SILT TIGHT JOINT TYPE
INV(IN)=12.70 INV(OUT)=11.80

DI-12-10) I ST'D DI-3C REQ'D
H=4J' L=6' INV=13J0
ST'D ST-I REQ'D
CONNECT UD-4 & CD-2 TO STRUCTURE

DI-12-10 DI-12-9 50'-15" CONC.SSP CLASS III REQ'D (3' COVER)
SILT TIGHT JOINT TYPE
INV(IN)=13.10 INV(OUT)=12.70

HW-12-II) | ST'D 24" ES-I REQ'D 6.00 TONS ST'D EC-I CLASS AI REQ'D TYPE A INSTALLATION INV=9.00

DI-12-12) 4.7' STD. SWM-I REQ'D
TOP ELEV=11.50
BOTTOM ELEV=6.80
GRAVEL DIAPHRAGM, AS PER DEATIL
SEE SHEETS 21(6)-21(6A) FOR DETAILS
CONNECT BMP UD TO RISER STRUCTURE

DI-12-12 DI-12-13 58'-24" CONC.SSP CLASS III REQ'D (6' COVER)
LEAK-RESISTANT JOINT TYPE
INV(IN)=7.50 INV(OUT)=7.00
21.4 CY OF CONCRETE CRADLE REQ'D

DI-12-13) I ST'D 24" ES-I REQ'D 6.00 TONS ST'D EC-I CLASS AI REQ'D TYPE A INSTALLATION INV=7.00

SHEET 13

DI-13-1 8.0' STD. SWM-I REQ'D
TOP ELEV=12.00
BOTTOM ELEV=4.00
GRAVEL DIAPHRAGM, AS PER DEATIL
SEE SHEETS 21(6)-21(6A) FOR DETAILS

CONNECT BMP UD TO RISER STRUCTURE

DI-13-1

DI-13-2

56'-24" CONC.SSP CLASS III REQ'D (4' COVER)

LEAK-RESISTANT JOINT TYPE

INV(IN)=7.00 INV(OUT)=6.50

9.7 CY OF CONCRETE CRADLE REQ'D

DI-13-2 6.5' ST'D PRECAST MH-2 REQ'D
I ST'D PRECAST T-DI-7 TOP REQ'D
INV=5.33
CONNECT TO EXISTING 60" RCP
4'-60" CONC.SSP REQ'D FOR CONNECTION
ST'D ST-I REQ'D FOR MANHOLE BASE

DI-13-3 10.5' ST'D PRECAST MH-2 REQ'D
I ST'D PRECAST T-DI-3 TOP REQ'D
L=8' INV=9.90
I ST'D SL-I REQ'D
CONNECT TO EXISTING 24" RCP
4'-24" CONC.SSP REQ'D FOR CONNECTION
ST'D ST-I REQ'D FOR MANHOLE BASE
CONNECT UD-4 & UD-2 TO STRUCTURE

DI-13-3 — MH-13-4 150'-60" CONC.SSP CLASS III REQ'D (7' COVER) LEAK-RESISTANT JOINT TYPE INV(IN)=9.90 INV(OUT)=6.98 687.5 CY MINOR STRUCTURE EXCAVATION

> MH-13-4) 9.5 L.F OF ST'D MH-2 REQ'D I ST'D MH-1 FRAME & COVER REQ'D INV=6.88 ST'D ST-1 REQ'D

> > CONNECT UD-4 TO STRUCTURE

-DI-13-2 | 114'-60" CONC.SSP CLASS | 111 REQ'D (5' COVER) LEAK-RESISTANT JOINT TYPE INV(IN)=6.88 | INV(OUT)=5.33 447.4 CY MINOR STRUCTURE EXCAVATION ES-13-5) | ST'D 24" ES-1 REQ'D 6.00 TONS ST'D EC-1 CLASS AI REQ'D TYPE A INSTALLATION INV=11.00

DI-13-6) I ST'D DI-3B REQ'D H=4,1' L=4' INV=12,90 I ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

DI-12-7 47'-15" CONC.SSP CLASS III REQ'D (3' COVER)
SILT TIGHT JOINT TYPE
INV(IN)=12.90 INV(OUT)=12.50

DI-13-7) I ST'D DI-3B REQ'D H=4.9' L=6' INV=15.00 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

DI-13-7 DI-13-8 108'-15" CONC.SSP CLASS III REQ'D (3' COVER)
SILT TIGHT JOINT TYPE
INV(IN)=15.00 INV(OUT)=14.00

DI-13-8) I ST'D DI-3B REQ'D H=7.0' L=4' INV=II.90 ST'D ST-I REQ'D ST'D IS-I REQ'D CONNECT UD-4 TO STRUCTURE

01-13-8 DI-13-9 68'-24" CONC.SSP CLASS III REQ'D (5' COVER)
SILT TIGHT JOINT TYPE
INV(IN)=II.90 INV(OUT)=II.50

DI-13-9) I ST'D DI-3B REQ'D H=7.0' L=4' INV=II.40 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

DI-13-9 — ES-13-5) 21'-24" CONC.SSP CLASS III REQ'D (5' COVER) SILT TIGHT JOINT TYPE INV(IN)=II.40 INV(OUT)=II.00

> DI-13-10) I ST'D DI-3B REQ'D H=6.2' L=6' INV=16.30 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

DI-13-10 DI-13-11 109'-15" CONC.SSP CLASS III REQ'D (5' COVER)
SILT TIGHT JOINT TYPE
INV(IN)=16.30 INV(OUT)=15.18

DI-13-II) I ST'D DI-3B REQ'D H=5.2' L=6' INV=15.08 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

DI-13-11 DI-13-12 IOI'-15" CONC.SSP CLASS III REQ'D (4' COVER)
SILT TIGHT JOINT TYPE
INV(IN)=15.08 INV(OUT)=14.05

DI-13-12) I ST'D DI-3B REQ'D H=6.6' L=4' INV=12.50 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

DI-13-12 DI-13-8 96'-18" CONC.SSP CLASS III REQ'D (5' COVER)
SILT TIGHT JOINT TYPE

INV(IN)=12.50 INV(OUT)=12.00

DI-13-13 | ST'D DI-2B REQ'D

H=3.8' L=6' INV=14.14

CONNECT UD-4 TO STRUCTURE

DI-13-13 — DI-13-20 140'-15" CONC.SSP CLASS III REQ'D (4' COVER)

SILT TIGHT JOINT TYPE

INV(IN)=14.14 INV(OUT)=13.44

DI-13-14) | ST'D DI-3B REQ'D H=3.7' L=4' |NV=13.50 ST'D ST-| REQ'D

CONNECT UD-4 TO STRUCTURE

DI-13-14 DI-12-10 48'-15" CONC.SSP CLASS III REQ'D (3' COVER)

SILT TIGHT JOINT TYPE

INV(IN)=13.50 INV(OUT)=13.10

DI-13-15) I ST'D 2D REQ'D

H=5.4' L=8' INV=20.63

ST'D ST-I REQ'D

CONNECT UD-4 TO STRUCTURE

DI-13-15 — WH-Ex193 77'-18" CONC.SSP CLASS III REQ'D (6' COVER)

SILT TYPE JOINT TYPE

SILT TYPE JOINT TYPE
INV(IN)=20.63 INV(OUT)=20.06

(DI-13-17) I ST'D DI-3B REQ'D

CONNECT UD-4 TO STRUCTURE

(DI-13-17) DI-13-18) 66'-15" CONC.SSP CLASS III REQ'D (2' COVER)
SILT TIGHT JOINT TYPE

H=4.1' L=8' INV=20.21

ST'D ST-I REQ'D

DI-13-18) I ST'D DI-3B REQ'D H=3.3' L=6' INV=17.70 ST'D IS-I REQ'D CONNECT UD-4 TO STRUCTURE

INV(IN)=20.21 INV(OUT)=17.80

REVISED STATE ROUTE PROJECT

VA. / OOOI-212-249, PE-IOI 2E:(4)

DESIGN FEATURES RELATING TO CONSTRUCTION

DESIGN FEATURES RELATING TO CONSTRUCTION
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VDOT Location & Design Richmond, Virginia HYDRAULIC ENGINEER Note: In addition to the visual inspection performed by the Department during the initial installation of storm sewer pipes and pipe culverts, a post installation visual/video camera inspection shall be conducted by the Contractor in accordance with the requirements of this specification and VTM 123 on all storm sewer pipe and a selected number of pipe culverts.

DI-13-18 — DI-13-21) 41'-15" CONC.SSP CLASS III REQ'D (2' COVER)
SILT TIGHT JOINT TYPE
INV(IN)=17.70 INV(OUT)=16.00

DI-13-19) I ST'D DI-1 REQ'D H=2.8' INV=15.67 ST'D ST-1 REQ'D H LESS THAN MIN

DI-13-19
DI-13-12

33'-15" CONC.SSP CLASS III REQ'D (2' COVER)
SILT TIGHT JOINT TYPE
INV(IN)=15.67 INV(OUT)=15.00

DI-13-20) I ST'D DI-3B REQ'D H=6.0' L=6' INV=13.34 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

DI-13-20 — DI-13-21 49'-15" CONC.SSP CLASS III REQ'D (5' COVER)
SILT TIGHT JOINT TYPE
INV(IN)=13.34 INV(OUT)=13.00

DI-13-21) I ST'D DI-3C REQ'D H=6.4' L=6' INV=12.90 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

DI-13-21 — DI-13-12 40'-15" CONC. SSP CLASS III REQ'D (5' COVER)
SILT TIGHT JOINT TYPE
INV(IN)=12,90 INV(OUT)=12,60

DI-13-22) I ST'D DI-3B REQ'D H=3,6' L=6' INV=II.48 CONNECT UD-4 TO STRUCTURE

DI-13-22 DI-13-23 78'-15" CONC.SSP CLASS III REQ'D (2' COVER)
SILT TIGHT JOINT TYPE
INV(IN)=II.48 INV(OUT)=II.07

DI-13-23) | ST'D DI-1 REQ'D H=2.9' INV=11.07

DI-13-23 - HW-13-24) 13'-15" CONC.SSP CLASS III REQ'D (2' COVER)
SILT TIGHT JOINT TYPE
INV(IN)=II.07 INV(OUT)=II.00

HW-13-24) I ST'D 15" ES-I REQ'D 2.34 TONS ST'D EC-I CLASS AI REQ'D TYPE A INSTALLATION INV=11.00

(DI-13-25) | ST'D DI-3B REQ'D H=3.8' L=4' | INV=13.40 CONNECT UD-4 TO STRUCTURE

DI-13-6) 68'-15" CONC.SSP CLASS III REQ'D (3' COVER)
SILT TIGHT JOINT TYPE
INV(IN)=13.40 INV(OUT)=12.90

(DI-13-26) I ST'D DI-3A REQ'D H=5.6' INV=21.19 ST'D ST-1 REQ'D CONNECT UD-4 TO STRUCTURE

DI-13-26 — DI-13-15) 17'-15" CONC.SSP CLASS III REQ'D (4' COVER) SILT TIGHT JOINT TYPE INV(IN)=21.19 INV(OUT)=20.98

SHEET 14

MH-14-1) 12.0 L.F OF ST'D MH-2 REQ'D
I ST'D MH-1 FRAME & COVER REQ'D
INV=11.3
ST'D ST-1 REQ'D
CONNECT UD-4 TO STRUCTURE

MH-14-1 DI-13-3 58'-60" CONC.SSP CLASS III REQ'D (8' COVER) LE AK-RESISTANT JOINT TYPE INV(IN)=11.30 INV(OUT)=10.00

300.7 CY MINOR STRUCTURE EXCAVATION

MH-14-2) 9.4 L.F OF ST'D MH-2 REQ'D

I ST'D MH-I FRAME & COVER REQ'D

CONNECT TO EXISTING 60" RCP

4 L.F OF 60" RCP REQ'D

FOR CONNECTION

SSP: STORM SEWER PIPE

INV=18,II

ST'D ST-I REQ'D

PIPE: SPEC. SECTION 232 (AASH)

PIPE: SPEC. SECTION 232 (AASHTO MITO GASKET: SPEC. SECTION 212 (ASTM C44.

PROJECT SHEET NO.

0001-212-249 2E:(4)

R/W PLANS

(DI-14-2C)

SHEET 14 CONTINUED

MH-14-2A) 7.4 L.F OF ST'D MH REQ'D
I ST'D MH-I FRAME & COVER REQ'D
CONNECT TO EXISTING 60" RCP
4 L.F OF 60" RCP REQ'D FOR CONNECTION
INV=18.00
CONNECT UD-2 TO STRUCTURE

MH-14-2A DI-14-I 62'-60" CONC.SSP CLASS III REQ'D (3' COVER)
SILT TIGHT JOINT TYPE
INV(IN)=18.00 INV(OUT)=16.00
209.7 CY MINOR STRUCTURE EXCAVATION

(DI-14-2C) I ST'D DI-3B REQ'D

H=3.3' L=4' INV=20.70 CONNECT UD-4 TO STRUCTURE — DI-14-3C 38'-15" CONC.SSP CLASS III REQ'D (2' COVER) SILT TIGHT JOINT TYPE

DI-I4-3) I ST'D DI-3B REQ'D H=4.6' L=10' INV=23.60 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

DI-14-3 DI-14-22) 51'-18" CONC. SSP CLASS III REQ'D (7' COVER) SILT TIGHT JOINT TYPE INV(IN)=23.60 INV(OUT)=20.90

INV(IN)=20.70 INV(OUT)=19.80

DI-14-3C) I ST'D DI-3B REQ'D H=5.2' L=8' INV=18.60 ST'D ST-1 REQ'D CONNECT UD-4 TO STRUCTURE

94-3C MH-14-9 85'-24" CONC.SSP CLASS III REQ'D (10' COVER)
SILT TIGHT JOINT TYPE
INV(IN)=18,60 INV(OUT)=16,50

MH-14-9 13.7 L.F OF ST'D MH-2 REQ'D
1 ST'D MH-1 FRAME & COVER REQ'D
CONNECT EXISTING HDPE TO STRUCTURE
1NV=13.82
ST'D ST-1 REQ'D
CONNECT UD-4 TO STRUCTURE

MH-14-9 MH-14-1) 169'-42" CONC.SSP CLASS III REQ'D (II' COVER)
LEAK-RESISTANT JOINT TYPE
INV(IN)=13.82 INV(OUT)=11.40

DI-14-II) I ST'D DI-4BB REQ'D H=9.6' L=8' INV=22.93 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

14-11)—(MH-14-9) 46'-30" CONC.SSP CLASS III REQ'D (7' COVER) LEAK-RESISTANT JOINT TYPE INV(IN)=22.93 INV(OUT)=22.20

DI-14-12) I ST'D DI-3BB REQ'D H=15.3' L=8' INV=36.64 I ST'D SL-I REQ'D CONNECT UD-4 & UD-2 TO STRUCTURE

DI-14-12 DI-14-20 278'-30" CONC.SSP CLASS III REQ'D (12' COVER)
LEAK-RESISTANT JOINT TYPE
INV(IN)=36.64 INV(OUT)=31.04

DI-14-13) I ST'D DI-3BB REQ'D H=15.3' L=8' INV=42.4I I ST'D SL-I REQ'D CONNECT UD-4 TO STRUCTURE

DI-14-13 DI-14-12 98'-30" CONC.SSP CLASS III REQ'D (12' COVER)
LEAK-RESISTANT JOINT TYPE
INV(IN)=42.41 INV(OUT)=40.40

DI-14-16) I ST'D DI-3BB REQ'D H=9.3' L=8' INV=16.75 ST'D ST-1 REQ'D

CONNECT UD-4 TO STRUCTURE

DI-14-16 DI-13-10 53'-15" CONC. SSP CLASS III REQ'D (8' COVER)

SILT TIGHT JOINT TYPE

INV(IN)=16.75 INV(OUT)=16.40

DI-14-17) I ST'D DI-3B REQ'D H=7.8' L=10' INV=30.50 ST'D ST-1 REQ'D CONNECT UD-4 TO STRUCTURE

DI-14-17 DI-14-3 164'-18" CONC. SSP CLASS III REQ'D (6' COVER)
SILT TIGHT JOINT TYPE
INV(IN)=30.50 INV(OUT)=24.00

DI-14-18) I ST'D DI-3B REQ'D H=7.8' L=10' INV=41.40 ST'D ST-1 REQ'D CONNECT UD-4 & UD-2 TO STRUCTURE

DRAINAGE DESCRIPTIONS

DI-14-18 DI-14-17 179'-18" CONC. SSP CLASS III REQ'D (6' COVER)
SILT TIGHT JOINT TYPE
INV(IN)=41,40 INV(OUT)=30,60

DI-14-20) I ST'D DI-3BB REQ'D H=II.9' L=I0' INV=24.04 I ST'D SL-I REQ'D CONNECT UD-4 TO STRUCTURE

DI-14-20 DI-14-II 65'-30" CONC.SSP CLASS III REQ'D (9' COVER) LEAK-RESISTANT JOINT TYPE INV(IN)=24.04 INV(OUT)=23.03

> DI-14-21) I ST'D DI-3B REQ'D H=7.5' L=6' INV=19.35 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

DI-14-21 MH-14-2) 18'-18" CONC.SSP CLASS III REQ'D (8' COVER)
SILT TIGHT JOINT TYPE
INV(IN)=19.35 INV(OUT)=18.90

DI-14-22) I ST'D DI-3BB REQ'D H=9.3' L=8' INV=20.80 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

DI-14-22)— DI-14-21) 76'-18" CONC.SSP CLASS III REQ'D (7' COVER) SILT TIGHT JOINT TYPE INV(IN)=20.80 INV(OUT)=19.45

> DI-14-23) I ST'D DI-3A REQ'D H=4,5' INV=22,10 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

DI-14-23)—(DI-14-24) 28'-15" CONC.SSP CLASS III REQ'D (3' COVER) SILT TIGHT JOINT TYPE INV(IN)=22.JO INV(OUT)=21.79

DI-14-24) I ST'D DI-3C REQ'D

H=4.8' L=6' INV=21.69

ST'D ST-I REQ'D

CONNECT UD-4 & CD-2 TO STRUCTURE

24)—DI-13-26) 37'-15" CONC.SSP CLASS III REQ'D (4' COVER)

SILT TIGHT JOINT TYPE

INV(IN)=21.69 INV(OUT)=21.29

(MH-Ex193) MODIFY EXISTING DITO ACCEPT PROPOSED 18" RCP

SHEET 140

MH-14C-I) MODIFY EXISTING STRUCTURE TO ACCEPT PROPOSED 24" RCP FILL OUTGOING EXISTING 24" RCP WITH FILL MATERIALS I ST'D MH-I FRAME & COVER REQ'D INV=19,26

ST'D ST-I REQ'D

MH-I4C-I)—DI-I4-3C 32'-24" CONC.SSP CLASS III REQ'D (4' COVER)

SILT TIGHT JOINT TYPE

INV(IN)=19.26 INV(OUT)=18.70

DI-14C-4) | ST'D DI-7 REQ'D H=4.0' INV=9.80

DI-14C-4 DI-13-2 56'-24" CONC.SSP CLASS III REQ'D (5' COVER)
SILT TIGHT JOINT TYPE

INV(IN)=9.80 INV(OUT)=5.33

SHEET 15

MH-15-2) 21.6 L.F OF ST'D MH-1 OR 2 REQ'D I ST'D MH-1 FRAME & COVER REQ'D INV=48.25 ST'D ST-1 REQ'D I ST'D SL-1 REQ'D

1-15-2 DI-15-3 159'-24" CONC.SSP CLASS III REQ'D (20' COVER) SILT TIGHT JOINT TYPE INV(IN)=48,25 INV(OUT)=45,82

DI-15-3) I ST'D DI-3BB REQ'D
H=13.7' L=6' INV=45.72
ST'D ST-I REQ'D
I ST'D SL-I REQ'D
CONNECT UD-4 TO STRUCTURE

DI-15-3 — DI-15-24) 54'-24" CONC.SSP CLASS III REQ'D (II' COVER)
SILT TIGHT JOINT TYPE
INV(IN)=45.72 INV(OUT)=44.87

DI-15-4) I ST'D DI-3BB REQ'D H=II.6' L=6' INV=65.00 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

-MH-15-2) 228'-24" CONC.SSP CLASS III REQ'D (9' COVER) SILT TIGHT JOINT TYPE INV(IN)=65.00 INV(OUT)=61.52 DI-15-5) I ST'D DI-3B REQ'D H=4.6' L=6' INV=72.52 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

DI-15-6) 2'-18" CONC.SSP CLASS III REQ'D (3' COVER)
SILT TIGHT JOINT TYPE
INV(IN)=72.52 INV(OUT)=72.40

DI-15-6) I ST'D DI-5 REQ'D
TYPE D COVER REQ'D
TYPE III GRATE REQ'D
H=3.6' INV=72.30
ST'D IS-I REQ'D

DI-15-6 DI-15-7 22'-18" CONC.SSP CLASS III REQ'D (4' COVER)
SILT TIGHT JOINT TYPE
INV(IN)=72.30 INV(OUT)=71.78

DI-15-7 | ST'D DI-3C REQ'D H=6.0' L=6' INV=71.68 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

DI-15-7 DI-15-4 57'-24" CONC. SSP CLASS III REQ'D (4' COVER)
SILT TIGHT JOINT TYPE
INV(IN)=71.68 INV(OUT)=70.50

DI-15-7A) I ST'D DI-3BB REQ'D H=9.5' L=10' INV=49.39 ST'D ST-1 REQ'D

CONNECT UD-4 & CD-2 TO STRUCTURE

1-15-7A DI-14-18 167'-18" CONC.SSP CLASS III REQ'D (8' COVER)

SILT TIGHT JOINT TYPE

INV(IN)=49.39 INV(OUT)=44.30

DI-15-8) I ST'D DI-3BB REQ'D H=12.7' L=10' INV=52.41 I ST'D SL-1 REQ'D CONNECT UD-4 TO STRUCTURE

DI-15-8 DI-14-13 144'-30" CONC.SSP CLASS III REQ'D (10' COVER)
LE AK-RESISTANT JOINT TYPE
INV(IN)=52.41 INV(OUT)=49.47

DI-15-9 I ST'D DI-3BB REQ'D
H=14.4' L=8' INV=55.5I
ST'D SL-I REQ'D
I ST'D ST-I REQ'D
CONNECT UD-4 TO STRUCTURE

DI-15-9 DI-15-8 147'-30" CONC.SSP CLASS III REQ'D (12' COVER)
LEAK-RESISTANT JOINT TYPE
INV(IN)=55.51 INV(OUT)=52.51

DI-15-10) I ST'D DI-I REQ'D H=4.3' INV=64.70 ST'D ST-I REQ'D

DI-15-10 DI-15-9 23'-15" CONC.SSP CLASS III REQ'D (4' COVER)
SILT TIGHT JOINT TYPE
INV(IN)=64.70 INV(OUT)=64.00

(DI-15-10A) I ST'D DI-1 REQ'D H=3,J' INV=65,30

DI-15-10A DI-15-10 23'-15" CONC.SSP CLASS III REQ'D (3' COVER)
SILT TIGHT JOINT TYPE
INV(IN)=65.30 INV(OUT)=64.80

DI-15-12) I ST'D 3BB REQ'D
H=15.3' L=8' INV=59.4I
ST'D SL-I REQ'D
ST'D ST-I REQ'D
CONNECT UD-4 & UD-2 TO STRUCTURE

DI-15-12 DI-15-9 187'-30" CONC.SSP CLASS III REQ'D (13' COVER) LEAK-RESISTANT JOINT TYPE INV(IN)=59.41 INV(OUT)=55.61

DI-15-13) | ST'D 3BB REQ'D H=16.0' L=4' |NV=61.00 ST'D SL-1 REQ'D ST'D ST-1 REQ'D

DI-15-13 — DI-15-12 69'-30" CONC.SSP CLASS III REQ'D (13' COVER) LEAK-RESISTANT JOINT TYPE

CONNECT UD-4 TO STRUCTURE

INV(IN)=61,00 INV(OUT)=59,94

DI-15-14 | ST'D 3CC REQ'D
H=15.5' L=6' INV=61.65
ST'D SL-1 REQ'D
I ST'D ST-1 REQ'D
CONNECT UD-4 TO STRUCTURE

DI-15-13) 34'-24" CONC.SSP CLASS III REQ'D (13' COVER) LEAK-RESISTANT JOINT TYPE INV(IN)=61.65 INV(OUT)=61.10

HW-15-17) | ST'D 18" ES-1 REQ'D 4.52 TONS ST'D EC-1 CLASS | REQ'D TYPE A INSTALLATION INV=73.00 REVISED STATE STATE PROJECT

VA. / OOOI-212-249, PE-IOI 2E(5)

DESIGN FEATURES RELATING TO CONSTRUCTION

DESIGN FEATURES RELATING TO CONSTRUCTION
OR TO REGULATION AND CONTROL OF TRAFFIC
MAY BE SUBJECT TO CHANGE AS DEEMED
NECESSARY BY THE DEPARTMENT

VDOT Location & Design Richmond, Virginia HYDRAULIC ENGINEER Note: In addition to the visual inspection performed by the Department during the initial installation of storm sewer pipes and pipe culverts, a post installation visual/video camera inspection shall be conducted by the Contractor in accordance with the requirements of this specification and VTM 123 on all storm sewer pipe and a selected number of pipe culverts.

DI-15-18) I ST'D DI-3B REQ'D H=6.8' L=10' INV=73.60 ST'D ST-1 REQ'D CONNECT UD-4 TO STRUCTURE

DI-15-18 — (HW-15-17) 26'-18" CONC. SSP CLASS III REQ'D (5' COVER) SILT TIGHT JOINT TYPE INV(IN)=73.60 INV(OUT)=73.00

> DI-15-19) I ST'D DI-3BB REQ'D H=8.7' L=4' INV=69.40 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

01-15-19 DI-15-12 106'-18" CONC.SSP CLASS III REQ'D (7' COVER)
SILT TIGHT JOINT TYPE
INV(IN)=69.40 INV(OUT)=66.00

DI-15-20 | ST'D DI-3B REQ'D H=4.5' L=14' INV=61.95 ST'D ST-1 REQ'D CONNECT UD-4 TO STRUCTURE

SILT TIGHT JOINT TYPE
INV(IN)=61.95 INV(OUT)=54.40

DI-15-21) I ST'D DI-3B REQ'D H=4.5' L=12' INV=75.60 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

5-21 DI-15-18 110'-15" CONC.SSP CLASS 111 REQ'D (5' COVER)
SILT TIGHT JOINT TYPE
INV(IN)=75.60 INV(OUT)=73.70

DI-15-24) I ST'D DI-I REQ'D
CONNECT TO EXISTING 21" RCP
4 L.F OF 21" RCP REQ'D FOR CONNECTION
H=8.4' INV=44.77
ST'D ST-I REQ'D

-15-24)—ES-15-25) 83'-24" CONC.SSP CLASS III REQ'D (6' COVER) SILT TIGHT JOINT TYPE INV(IN)=44,77 INV(OUT)=43,50

> ES-15-25 | ST'D 24" ES-1 REQ'D 8.00 TONS ST'D EC-1 CLASS | REQ'D TYPE A INSTALLATION INV=43.50

> > SHEET 15C

DI-15C-I) I ST'D DI-3BB REQ'D H=9.4' L=4' INV=68.52 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

C-I) DI-15-14) 47'-24" CONC.SSP CLASS III REQ'D (7' COVER) LEAK-RESISTANT JOINT TYPE INV(IN)=68,52 INV(OUT)=67.61 8.2 CY OF CONCRETE CRADLE REQ'D

DI-15C-2) I ST'D DI-5 REQ'D
TYPE A2 COVER REQ'D
TYPE III GRATE REQ'D
H=8,1' INV=77,00
ST'D ST-1 REQ'D

DI-15C-4) 99'-15" CONC.SSP CLASS III REQ'D (6' COVER)
SILT TIGHT JOINT TYPE
INV(IN)=77.00 INV(OUT)=72.92

CONNECT UD-4 TO STRUCTURE

DI-15C-3 8.0' STD. SWM-I REQ'D
TOP ELEV=74.00
BOTTOM ELEV=66.00
GRAVEL DIAPHRAGM, AS PER DEATIL
SEE SHEETS 2i(7)-2i(7A) FOR DETAILS
CONNECT BMP UD TO RISER STRUCTURE

DI-15C-I) 40'-24" CONC.SSP CLASS III REQ'D (7' COVER)
LEAK-RESISTANT JOINT TYPE
INV(IN)=69.00 INV(OUT)=68.52
6.9 CY OF CONCRETE SSP.STORM SEV

CRADLE REQ'D

SSP:STORM SEWER PIPE
ST'D IS-I REQ'D FOR ALL STRS.
PIPE:SPEC.SECTION 232 (AASHTO MI70
GASKET:SPEC.SECTION 212 (ASTM C443

0001-212-249 2E(5)

R/W PLANS

VA.

ROUTE

REVISED

STATE

PROJECT

0001-212-249,PE-101 | 2E(6)

PROJECT MANAGERSusie_Lue_(7.0.3)259-2918_NOVA_District______ SURVEYED BY, DATE Leon E.Treutle LS_(703)259-3224 7/17/13_____ DESIGN BY JMT_ Engineering_(804)_323-9900______ SUBSURFACE UTILITY BY, DATE Lean F. Treutle LS (703)259-3224 7/17/13

DRAINAGE DESCRIPTIONS & DITCH TYPICALS

SHEET ISC CONTINUED

(DI-15C-4) I ST'D DI-3B REQ'D H=7.4' L=8' INV=72.82 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

(DI-15C-I) 77'-18" CONC.SSP CLASS III REQ'D (6' COVER) (DI-15C-4**)**-SILT TIGHT JOINT TYPE INV(IN)=72.82 INV(OUT)=69.66

> (DI-15C-5) I ST'D DI-3B REQ'D H=5.4' L=6' INV=73.80 ST'D ST-I REQ'D

CONNECT UD-4 TO STRUCTURE (DI-15C-5)—(DI-15-C4) 32'-18" CONC.SSP CLASS III REQ'D (5' COVER) SILT TIGHT JOINT TYPE INV(IN)=73.80 INV(OUT)=73.45

> (DI-15C-6) I ST'D DI-3B REQ'D H=4.7' L=4' INV=74.20 ST'D ST-I REQ'D

CONNECT UD-4 TO STRUCTURE —(DI-15C-5) 27'-18" CONC.SSP CLASS III REQ'D (3' COVER) SILT TIGHT JOINT TYPE INV(IN)=74.20 INV(OUT)=73.90

SHEET 16

IST'D DI-3B REQ'D DI-16-1 H=2.5' L=12' INV=95.10 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

DI-15-18) 296'-15" CONC.SSP CLASS III REQ'D (5' COVER) SILT TIGHT JOINT TYPE INV(IN)=95.10 INV(OUT)=73.70

> (DI-16-2) I ST'D DI-3B REQ'D H=4,4' L=8' INV=107.40 ST'D ST-I REQ'D

CONNECT UD-4 TO STRUCTURE (DI-16-1) 263'-15" CONC.SSP CLASS III REQ'D (3' COVER) SILT TIGHT JOINT TYPE INV(IN)=107.40 INV(OUT)=95.20

(MH-16-3) 13.7 L.F OF ST'D MH-2 REQ'D IST'D MH-IFRAME & COVER REQ'D INV=78.79 IST'D SL-IREQ'D ST'D ST-I REQ'D

CONNECT TO EXISTING 36" PIPE MH-I6-3 DI-I5-I9) 261'-18" CONC.SSP CLASS III REQ'D (13' COVER)
SILT TIGHT JOINT TYPE INV(IN)=78.79 INV(OUT)=69.50

> (DI-16-4) I ST'D DI-3BB REQ'D H=14.8' L=6' INV=77.75 CONNECT OUTLET CONC.PIPE FROM UNDERGROUND DETENTION TO PROPOSED DI ST'D ST-I REQ'D IST'D SL-IREQ'D

CONNECT UD-4 TO STRUCTURE (DI-15-7) 226'-24" CONC.SSP CLASS III REQ'D (12' COVER) SILT TIGHT JOINT TYPE INV(IN)=77.75 INV(OUT)=72.19

DI-16-5) I ST'D DI-1 REQ'D H=8.7' INV=85.67 ST'D ST-I REQ'D

(DI-16-4) 44'-24" CONC.SSP CLASS III REQ'D (7' COVER) SILT TIGHT JOINT TYPE INV(IN)=85.67 INV(OUT)=84.71

(DI-16-7) I ST'D DI-3B REQ'D H=4.6' L=10' INV=89.90 ST'D ST-I REQ'D

CONNECT UD-4 & UD-2 TO STRUCTURE (DI-15-21) 249'-15" CONC.SSP CLASS III REQ'D (3' COVER)

SILT TIGHT JOINT TYPE INV(IN)=89.90 INV(OUT)=75.70 (DI-16-8) I ST'D DI-3B REQ'D

H=4.5' L=10' INV=102.00 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE — DI-16-7) 178'-15" CONC.SSP CLASS III REQ'D (3' COVER) DI-16-8 SILT TIGHT JOINT TYPE

INV(IN)=102.00 INV(OUT)=90.00

(MH-16-10) 16.3 L.F OF ST'D MH-1 OR 2 REQ'D IST'D MH-IFRAME & COVER REQ'D CONNECT TO EXISTING 15" RCP 4 L.F OF 15" RCP REQ'D FOR CONNECTION INV=89**.**55 ST'D ST-I REQ'D

IST'D SL-IREQ'D (MH-16-10)—(DI-16-5) 185'-18" CONC.SSP CLASS III REQ'D (16' COVER) SILT TIGHT JOINT TYPE INV(IN)=89.55 INV(OUT)=85.77

(MH-16-11) 6.7 L.F OF ST'D MH-1 OR 2 REQ'D IST'D MH-IFRAME & COVER REQ'D INV=105.03 ST'D ST-I REQ'D ST'D IS-I REQ'D

(MH-16-10) 198'-18" CONC.SSP CLASS III REQ'D (6' COVER) SILT TIGHT JOINT TYPE INV(IN)=105.03 INV(OUT)=101.00

> IST'D DI-3BB REQ'D H=8.0' L=6' INV=105.54 CONNECT TO EXISTING 24" RCP 4 L.F OF 24" RCP REQ'D FOR CONNECTION ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

(MH-16-11) 17'-18" CONC.SSP CLASS III REQ'D (6' COVER) SILT TIGHT JOINT TYPE INV(IN)=105.54 INV(OUT)=105.13

IST'D DI-2A REQ'D CONNECT TO EXISTING 15" HDPE H=4.0' INV=107.17 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

DI-Ex249) MODIFY EX.STRUCTURE TO ACCEPT PROPOSED 15" PIPE

DI-16-5) II'-15" CONC.SSP CLASS III REQ'D (5' COVER) SILT TIGHT JOINT TYPE INV(IN)=88.35 INV(OUT)=88.00

SHEET 17

DI-17-3 MODIFY EX. STRUCTURE TOP TO ST'D PRECAST T-DI-4 L=8' INV=80.65

DI-17-5) I ST'D DI-3BB REQ'D H=10.0' L=8' INV=97.54 ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

MH-17-12) 83'-18" CONC.SSP CLASS III REQ'D (10' COVER) SILT TIGHT JOINT TYPE INV(IN)=97.54 INV(OUT)=95.00

DI-17-5) 88'-18" CONC.SSP CLASS III REQ'D (9' COVER) SILT TIGHT JOINT TYPE INV(IN)=99.69 INV(OUT)=97.64

> (DI-17-6) I ST'D DI-3B REQ'D H=5.1' L=8' INV=106.23 IST'D ST-IREQ'D CONNECT UD-4 TO STRUCTURE

(6)-EX288 38'-15" CONC.SSP CLASS III REQ'D (5' COVER) SILT TIGHT JOINT TYPE INV(IN)=106.23 INV(OUT)=106.02

IST'D DI-3B REQ'D H=5.0' L=8' INV=103.45 IST'D ST-IREQ'D CONNECT UD-4 TO STRUCTURE

(DI-17-3) 163'-18" CONC.SSP CLASS III REQ'D (3' COVER) SILT TIGHT JOINT TYPE INV(IN)=103.45 INV(OUT)=101.00

DI-17-8 I ST'D DI-3B REQ'D H=4.7' L=8' INV=104.50 IST'D ST-IREQ'D CONNECT UD-4 TO STRUCTURE

(DI-17-7) 32'-15" CONC.SSP CLASS III REQ'D (3' COVER) SILT TIGHT JOINT TYPE INV(IN)=104.50 INV(OUT)=103.60

DI-17-9 I ST'D DI-3B REQ'D H=6.3' L=10' INV=105.40 IST'D ST-IREQ'D CONNECT UD-4 TO STRUCTURE

DI-17-8 66'-15" CONC.SSP CLASS III REQ'D (4' COVER) SILT TIGHT JOINT TYPE INV(IN)=105.40 INV(OUT)=104.60

H=4.1' L=10' INV=105.94 IST'D ST-IREQ'D CONNECT UD-4 TO STRUCTURE (DI-17-9) 16'-15" CONC.SSP CLASS III REQ'D (4' COVER) SILT TIGHT JOINT TYPE

INV(IN)=105.94 INV(OUT)=105.50

IST'D DI-3B REQ'D H=5.0' L=6' INV=100.44 IST'D ST-IREQ'D

(DI-17-10) I ST'D DI-3B REQ'D

CONNECT UD-4 TO STRUCTURE (MH-17-12) 75'-18" CONC.SSP CLASS III REQ'D (6' COVER) SILT TIGHT JOINT TYPE INV(IN)=100.44 INV(OUT)=98.88

(MH-17-12) 18.4 L.F OF ST'D MH-2 REQ'D IST'D MH-IFRAME & COVER REQ'D CONNECT TO EXISTING 42" RCP DOUBLE 4 L.F 42" RCP REQ'D FOR CONNECTION INV=87**.**39 IST'D SL-IREQ'D ST'D ST-I REQ'D CONNECT UD-4 TO STRUCTURE

(MH-448) ADJUST EXISTING STRUCTURE TOP ELEVATION TO 100.00 IST'D MH-IFRAME & COVER REQ'D

ADJUST EXISTING STRUCTURE TOP ELEVATION TO 95.00 IST'D MH-IFRAME & COVER REQ'D INV=75**.**04

DI-EX293 MODIFY EXISTING DITO ACCEPT PROPOSED 18" RCP

DI-FX330 ADJUST EXISTING STRUCTURE TOP ELEVATION TO 100.00

SHEET IK(03)

Silt-Tight Joint Type INV(IN)=135.90 INV(OUT)=132.80

(IK(03)-2) 13.5 L.F OF ST'D MH-2 REQ'D IST'D FRAME & COVER REQ'D CONNECT TO EXISTING 15" RCP 4 L.F 15" RCP REQ'D FOR CONNECTION INV=122**.**50

(IK(03)-3) 170'-18" CONC.SSP CLASS III REQ'D (12' COVER) Silt-Tight Joint Type INV(IN)=122.50 INV(OUT)=116.00

I ST'D 18" ES-I REQ'D 3.38 TONS ST'D EC-ICLASS AIREQ'D TYPE A INSTALLATION INV=116.00 TEMPORARY END STRUCTURE FOR MOT

SHEET IK(04)

Silt-Tight Joint Type INV(IN)=78.87 INV(OUT)=78.58 CONNECT TO EXISTING RCP

SHEET IK(05)

47'-42" CONC.SSP CLASS III REQ'D (4' COVER) 10-3 Silt-Tight Joint Type INV(IN)=14.00 INV(OUT)=13.00 TEMPORARY PIPE FOR MOT

30'-18" CONC.SSP CLASS III REQ'D (3' COVER) Silt-Tight Joint Type INV(IN)=12.00 INV(OUT)=10.50

3.38 TONS ST'D EC-ICLASS AIREQ'D TYPE A INSTALLATION 1NV=10**.**50 TEMPORARY END STRUCTURE FOR MOT

INV=75**.**71

IST'D ST-IREQ'D

MODIFY EXISTING DITO ACCEPT PROPOSED 15" RCP

IST'D MH-IFRAME & COVER REQ'D

304'-18" CONC.SSP CLASS III REQ'D (3' COVER)

TEMPORARY PIPE FOR MOT TEMPORARY STRUCTURE FOR MOT

TEMPORARY PIPE FOR MOT

12'-15" CONC.PIPE CLASS III REQ'D (2' COVER) TEMPORARY PIPE FOR MOT

SHEET IK(06)

7.2 L.F OF ST'D MH-2 REQ'D IST'D FRAME & COVER REQ'D CONNECT TO EXISTING 3'X3' BOX CULVERT INV=14.00 TEMPORARY STRUCTURE FOR MOT

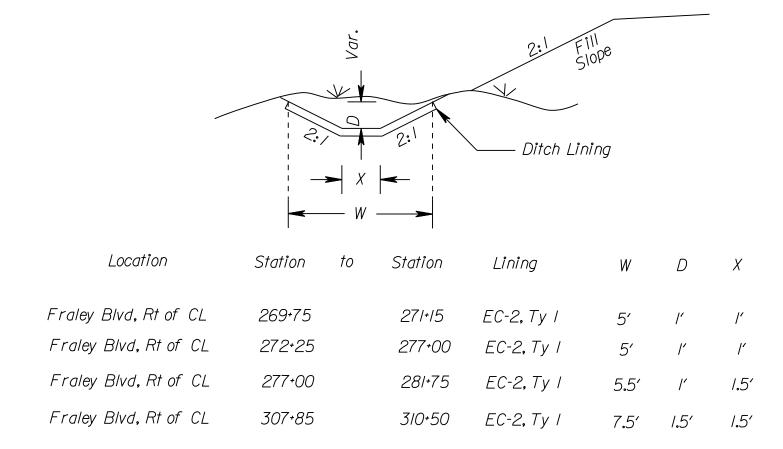
TEMPORARY PIPE FOR MOT

(IK(06)-2) I ST'D 18" ES-I REQ'D

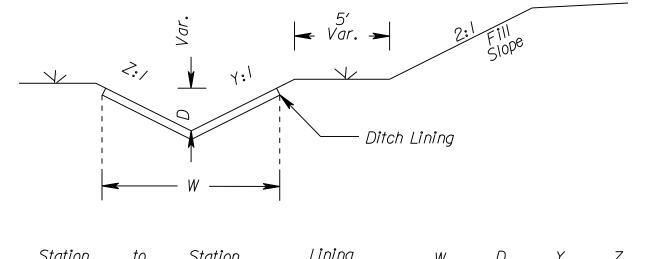
DESIGN FEATURES RELATING TO CONSTRUCTION OR TO REGULATION AND CONTROL OF TRAFFIC MAY BE SUBJECT TO CHANGE AS DEEMED NECESSARY BY THE DEPARTMENT Note: In addition to the visual inspection performed VDOT Location & Design by the Department during the initial installation of Richmond, Virginia storm sewer pipes and pipe culverts, a post installation HYDRAULIC ENGINEER visual/video camera inspection shall be conducted by the Contractor in accordance with the requirements of this specification and VTM 123 on all storm sewer pipe and a selected number of pipe culverts. TABLE AI - ALLOWABLE TYPE OF STORM SEWER PIPE

| LOCATION | CONCRETE | CORRUGATED STEEL ALUMINUM COATED TYPE 2 FULLY CONCRETE LINED | ALUMINUM COATED TYPE 2 STEEL SPIRAL RIB | POLYMER COATED (10/10) CORRUGATED STEEL SPIRAL RIB | POLYMER COATED (10/10) CORRUGATED STEEL DOUBLE WALL (SMOOTH INTERIOR) | ALUMINUM SPIRAL RIB | POLYVINYLCHLORIDE (PVC) RIBBED PIPE (SMOOTH INTERIOR) | POLYETHYLENE (PE) CORRUGATED TYPE S |
|-------------------------|----------|--|--|--|---|---------------------|---|--|
| Fraley Blvd ,Route I | Х | | | | Х | | Х | Х |
| Side Street Connections | Х | | | | Х | | Х | Х |
| Entrances | Х | Х | Х | Х | Х | Х | Х | Х |

Trapezoidal Ditches



Vee Ditches



| Location | Station | to | Station | Lining | W | D | Υ | Ζ | |
|-------------------------|----------------|----|----------------|------------|----|------------|---|---|--|
| Fraley Blvd, Lt of CL | <i>254+50</i> | | <i>255</i> +75 | EC-2, Ty 1 | 4′ | /' | 2 | 2 | |
| Fraley Blvd, Rt of CL | 265+50 | | 271+00 | EC-2,Ty I | 4′ | <i>ا</i> ′ | 2 | 2 | |
| Fraley Blvd, Rt of CL | <i>283+</i> 75 | | 284+50 | EC-2, Ty 1 | 4′ | <i>ا</i> ' | 2 | 2 | |
| Graham Park Rd,Lt of CL | II+65 | | II+25 | EC-2, Ty 1 | 4′ | <i>ا</i> ′ | 2 | 2 | |
| Fraley Blvd,Rt of CL | 297+00 | | <i>302+50</i> | EC-2,Ty I | 4′ | <i>l</i> ′ | 2 | 2 | |
| Fraley Blvd, Lt of CL | <i>303+90</i> | | <i>306+35</i> | EC-2,Ty I | 4' | <i>l</i> ′ | 2 | 2 | |
| Fraley Blvd, Lt of CL | <i>325+90</i> | | <i>323+60</i> | EC-2,Ty I | 4' | /' | 2 | 2 | |
| | | | | | | | | | |

SSP:STORM SEWER PIPE ST'D IS-I REQ'D FOR ALL STRS. PIPE: SPEC. SECTION 232 (AASHTO MI70. GASKET: SPEC. SECTION 212 (ASTM C443

SHEET NO.

2E:(6)

PROJECT 0001-212-249

PLANS

| PROJECT MANAGER |
|-----------------------------|
| SURVEYED BY, DATE |
| DESIGN BY |
| SUBSURFACE UTILITY BY, DATE |

STORMWATER POLLUTION PREVENTION PLAN (SWPPP) GENERAL INFORMATION SHEET

The information contained in the SWPPP General Information sheets is intended to comply with the requirements of the VPDES General Permit For Discharges Of Stormwater From Construction Activities (the VPDES Construction Permit) issued July 1, 2019 and VDOT's approved Annual ESC and SWM Standards and Specifications.

The SWPPP General Information sheets are to be completed and included in the construction plan set (or other such documents) for land disturbance activities that disturb an area equal to or greater than 10,000 square feet outside the Chesapeake Bay Preservation Area, or equal to or greater than 2,500 square feet in the area defined as Tidewater, Virginia in the Virginia Chesapeake Bay Preservation Act.

The VDOT RLD (as defined in the latest IIM 242) will ensure that the information shown on the SWPPP General Information sheets is updated/revised as necessary in order to reflect changes that may occur during the construction phase of the land disturbing (construction) activity. The updated/revised sheets shall be maintained with the designated record set of plans (or other such documents) for the land disturbance (construction) activity.

Icertify under penalty of law that I have read and understand this document and that this document and all attachments were prepared in accordance with a system designed to assure that qualified personnel properly gathered and evaluated the information submitted. Based on my inquiry of the person or persons who manage the system, or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate, and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations.

Ifurther certify that this document and all other documents related to the SWPPP, as identified on the SWPPP General Information Sheets, are maintained at the activity site, or at a location convenient to the activity site where no on-site facilities are available, and such documents will be made available for review upon request in accordance with the provisions of the <u>General VPDES Permit for Discharges of Stormwater from Construction Activities (VAR10)</u> when applicable. Where the SWPPP documents are not stored on-site, a copy of such documents shall be in the possession of those with day to day operational control over the implementation of the SWPPP whenever they are on site.

* or ** Delegated Authority Signature"

Signature: ______ Printed Name: ______ Date:_____

(1) See Section 1, Item 11 relating to delegation of authority, and form LD-445H (Delegation of Authority).

ACRONYMS

CBPA - Chesapeake Bay Preservation Act BMP - Best Management Practice

DEQ - Department of Environmental Quality EPA - U.S. Environmental Protection Agency

ESC - Erosion and Sediment Control

IIM - Instructional and Informational Memorandum

R&B - Road and Bridge RLD - Responsible Land Disturber SWPPP - Stormwater Pollution Prevention Plan
TMDL - Total Maximum Daily Load

VDOT - Virginia Department of Transportation
VPDES - Virginia Pollutant Discharge Elimination System

VSMP - Virginia Stormwater Management Program
VESCP - Virginia Erosion and Sediment Control Program

WLA - Waste Load Allocation SWM - Stormwater Management

SECTION I GENERAL INFORMATION

1. Activity Description - This project consists of the widening of the U.S. Route 1, Fraley Boulevard, roadway. The existing roadway will be widened from two lanes to six lanes. Existing side roadwas will be realigned to access the widened roadway. The project will also include the construction of a sidewalk along the east side of the roadway and a mixed use path on the west side. Eight stormwater BMPs will be constructed to manage water quality and quantity.

2. This land disturbance (construction) activity site is located in Town of Dumfries, Prince William County and approximately 36.30 acres will be disturbed by excavation, grading or other construction activities.

3. (Include one of the following notes as appropriate)

A. This proposed activity disturbs one acre or greater and requires coverage under the VPDES General Permit for Discharges Of Stormwater from Construction Activities (the VPDES Construction Permit) as issued by the DEQ. A copy of the VPDES Construction Permit (VAR10), the registration information (LD-445 & LD-445C forms) and the permit coverage letter received from DEQ shall be maintained with other SWPPP documents for this land disturbing activity.

B. This proposed activity disturbs less than one acre and is exempt from coverage under the VPDES General Permit for Discharges of Stormwater from Construction Activities (the VPDES Construction Permit) as issued by the DEQ.

C.This proposed activity is exempt from coverage under the VPDES General Permit For Discharges Of Stormwater From Construction Activities (the VPDES ConstructionPermit) as issued by the DEQ because it is considered a routine maintenance activity (i.e., the proposed activity is intended to maintain the original line and grade, hydraulic capacity or original construction of the project or involves the paving of an existing roadway with a compacted or impervious surface and the reestablishment of associated ditches and shoulders).

XX 4. The location of on-site support facilities that will be covered under the VPDES Construction Permit coverage for this land disturbance (construction) activity shall be provided by the contractor and identified on the record set of plans or in other appropriate contract documents. Support facilities shall include, but not be limited to, borrow and disposal areas, construction and waste material storage areas, equipment and vehicle washing, maintenance, storage and fueling areas, storage areas for fertilizers, fuels or chemicals, concrete wash out areas, sanitary waste facilities and any other areas that may generate a stormwater or non-stormwater discharge directly related to the construction site.

XX 5. Written Evidence of permit coverage shall be provided by the contractor for all support activities located outside of VDOT right of way or easement in the form of the Construction General Permit coverage letter: (List VPDES Permit * or Letter from VSMP Authority stating coverage not needed)

6. List the surface waters that have been identified as impaired in the DEQ 2012 305(b)/303(d) Water Quality Assessment Integrated Report for sediment, total suspended solids, turbidity, Nitrogen or Phosphorus. These pollutants are considered benthic impairments: Qunatico Creek (sediment)

7. Identify the TMDL's where stormwater from construction activities discharges into a watershed with a TMDL waste load allocation established and approved by the State Water Control Board prior to July 1, 2016 for sediment, total suspended solids, turbidity, nitrogen or phosphorus:

Chesapeake Bay TMDL (nitrogen, phsphorus and total suspended solids)

8. This land disturbance activity discharges stormwater to the following surface waters that have been identified as exceptional in Section 9VAC25-260-30 A 3 c of the Virginia Administrative Code: N/A

9. Locations of surface waters and locations where concentrated stormwater is discharged from this land disturbance (construction) activity are identified in the construction plan set (or other such documents) for this land disturbance (construction) activity. (List name of surface waters and locations here if not shown in construction plan or other such documents).

10. The ESC and SWM plans (where applicable) for this land disturbance (construction) activity have been developed in accordance with VDOT's Approved Annual Erosion and Sediment Control and Stormwater Management Standards and Specifications as approved by the DEQ.

11. List the RLD and other responsible parties for the land disturbance activity: (required for erosion and sediment control). The following individual(s) have "delegated authority" to sign all reports required by the construction permit including the SWPPP General Information Sheets and Inspection Reports (C-107). Reference form LD-445H for delegation of authority (form 445H for the project is hereby incorporated by reference into this SWPPP). These individual(s) has/have overall responsibility or the environmental matters for the project: (required only for permitted projects):

| Name | Position | Responsibility | | |
|------|---------------------|--------------------------------------|--|--|
| | RLD | Certify the SWPPP (with date & sig.) | | |
| | Certified Inspector | Sign (C-107) Inspection Form Part 1 | | |
| | Certified Inspector | Sign (C-107) Inspection Form Part 2 | | |
| | | | | |
| | | | | |
| | | | | |

X 12. The name of the VDOT individual(s) responsible for the oversight inspection in accordance with IIM-LD-256 on these land disturbance construction activities as identified on these SWPPP General Information Sheets. The names will be updated and maintained with the other SWPPP documents for this land disturbance activity.

| VDOT Individuals | Position | Responsibility |
|------------------|----------|--|
| Marian Carroll | NPDES | NPDES coordinator responsible for the oversight inspection in accordance with IIM-LD-256 |
| | | |
| | | |
| | | |

REVISED
STATE
ROUTE
PROJECT

VA.

0001-212-249, PE-101
2H(1)

DESIGN FEATURES RELATING TO CONSTRUCTION
OR TO REGULATION AND CONTROL OF TRAFFIC
MAY BE SUBJECT TO CHANGE AS DEEMED
NECESSARY BY THE DEPARTMENT

Johnson, Mirmiran & Thompson Richmond, Virginia HYDRAULIC ENGINEER

X 13. The ESC and P2 inspections for this land disturbing (construction) activity shall follow (Select Schedule 1 or 2, if schedule #2 is used, void note #14) as defined in 2016 R&B Specifications except for Section 107.16(e) 4.an Inspection Requirements Rain gauge notes apply only to Inspection Schedule 1.

XX 14. The location of the on-site rain gage that will be used to determine the occurrence of a measurable storm event for the purposes of ESC and Pollution Prevention inspections will be provided by the contractor and identified on the record set of plans or in other appropriate SWPPP documents for this land disturbance activity:

(List location of rain gage).

The rain gage shall be observed daily at "_______" to determine the occurrence of a measurable storm event (i.e., 0.25 inches of rainfall or greater in a 24 hour period). A log book shall be maintained to record observation information which shall include (1) the date, (2) the time, (3) whether or not rainfall is occurring at the time of the observation, (4) the amount of accumulated rainfall in the gage, if any, and (5) whether or not an inspection is required based on the amount of accumulated rainfall in the gage. If there is no rainfall occurring at the time of the observation, the observation information shall be noted in the log book and the rain gage emptied and replaced. An inspection is required if there is rainfall occurring at the time of the observation, the observation information is to be noted in the log book. The rain gage is not to be emptied but left to accumulate additional rainfall until the conclusion of the rainfall event. At the conclusion of the rainfall event, an observation of the rain gage shall be made and the observation information shall be noted in the log book and the rain gage emptied and replaced. An inspection is required if there is 0.25 inches or more accumulation noted in the rain gage.

15. The following VDOT documents are applicable to a) permitted projects b) non-permitted projects in Chesapeake Bay Preservation Areas (CBPA) with 2,500 S.F. to 1.0 acre of land disturbance c) non-permitted projects requiring a SWPPP and d) Non-permitted, Non-CBPA with BMP projects that have a water quantity BMP:

VDOT LD-445: Permitted projects, CBPA projects and Non-permitted, Non-CBPA with BMP projects that have a water quantity BMP and ESC projects > 10,000 s.f. but <1 acre.

VDOT LD-445A: Permitted projects only.

VDOT LD-445C: Projects that require a permit, ESC Plan, or SWPPP.

VDOT LD-445D: Permitted projects, CBPA projects and Non-permitted,

Non-CBPA with BMP projects that have a water quantity BMP.

VDOT LD-445F: Emergency work projects (when applicable).
Water Quality Requirement (when applicable)

VĎOT LD-445H: Permitted projects only.

VDOT C-107 Part Land Part II. All projects that require a permit or SWPPP.

VDOT LD-445I: AS&S Approval Form (when applicable)

16. If there is an excessive loading of sediment from the project (i.e. more than to be expected from the project with an implemented ESC plan) that is discovered within a local watershed with a sediment TMDL that allocates a WLA to VDOT's MS4, (see note #7) the contractor shall investigate the area of concern at the site within 24 hours of discovery and ensure all erosion and sediment control best management practices are being implemented in accordance with the permits approved standards and specifications required by Part I.B of the current Construction General Permit. If corrective action is necessary, the contractor shall initiate corrective actions no later than 5 business days after the initial investigation.

17. If excessive loading of sediment from a land disturbing activity that is not the responsibility of the contractor is discovered discharging into a MS-4, the contractor shall notify the municipality with jurisdiction over erosion and sediment control activities.

X Denotes information that is to be provided/completed by the RLD.

XX Denotes information that is to be provided/completed by the contractor.

PROJECT SHEET NO. 0001-212-249 2H(1)

SECTION II EROSION AND SEDIMENT CONTROL

- XX 1. The intended sequence and timing of activities that disturb soils at the site (e.g., grubbing, excavation, grading, utilities and infrastructure installation, etc.) shall be provided by the contractor in accordance with the current edition of Section 108.03 of the VDOT R&B Specifications and shall be included with the other SWPPP documents for this land disturbance (construction) activity.
- 2. Directions of stormwater flow and approximate slopes anticipated after major grading activities are identified in the construction plan set (or other such documents) for this land disturbance (construction) activity.
- 3. Areas of soil disturbance and areas of the site which will not be disturbed are identified in the construction plan set (or other such documents) for this land disturbance (construction) activity.
- 4. Locations of major structural and nonstructural ESC measures intended to filter, settle or similarly remove sediment are identified in the construction plan set (or other such documents) for this land disturbance (construction) activity.
- 5. Locations where stabilization practices are expected to occur are identified in the construction plan set (or other such documents) for this land disturbance (construction) activity.
- 6. A description of interim and permanent stabilization practices for the site are identified in the applicable sections of the documents identified in the Note 1 of Section IV.
- XX 7. A record of the dates when major grading activities occur, when construction activities temporarily or permanently cease on a portion of the site, and when stabilization measures are initiated will be provided by the contractor and maintained with the record set of plans or other SWPPP documents for this land disturbance (construction) activity:

 (List how this will be tracked and the location)
 - 8. A description and schedule of procedures to maintain vegetation, erosion and sediment control measures and other protective measures in good and effective operating conditions are identified in the current edition of Sections 107.16 and 303.03 of the VDOT R&B Specifications.
 - 9. Nutrients shall be applied in accordance with the current edition of Sections 603 and 604 of the VDOT Road and Bridge Specifications. Nutrients shall not be applied during rainfall events. Top soil shall be applied in accordance with the current edition of section 602 of the latest Road and Bridge Specifications.
 - 10. All engineering calculations supporting the design of erosion and sediment control measures proposed for this land disturbance (construction) activity are contained in the project drainage file located in the (insert appropriate location, i.e., VDOT Central Office Hydraulics Section or the VDOT (specify) District Hydraulics Section or the VDOT (specify) Residency Office) and will be made available for review upon request during normal business hours.
 - 11. The temporary erosion and siltation controlitems shown on the ESC Plan for this land disturbing (construction) activity are intended to provide a general plan for controlling erosion and sediment within the project limits. The ESC Plan is based on field conditions at the time of plan development and an assumed sequence of construction for the project. The contractor, in conjunction with the VDOT Project Engineer and/or ESC Inspector, shall adjust the location, quantity and type of erosion and sediment controlitems required based on the actual field conditions encountered at the time of construction and the actual scheduling and sequencing of the construction activities. Significant changes to the proposed ESC Plan (e.g., those that require an engineering analysis, elimination of a perimeter control, change to ESC concept that would affect the quantity or direction of flow of water) shall be submitted to the applicable District Hydraulics Engineer for review and approval. Any changes to the proposed ESC Plan must be noted on the designated record set of plans which shall be retained on the project site and made available upon request during normal business hours.
 - 12. The areas beyond the project's construction limits are to be protected from siltation. Perimeter controls such as silt fence, diversion dikes, turbidity curtains, etc. shall be installed prior to any grubbing operations or other earth moving activities.
 - 13. Temporary earthen structures such as dikes and berms are to be stabilized immediately upon installation. Stabilization may include temporary or permanent seeding, riprap, aggregate, sod, mulching, and/or soil stabilization blankets and matting in conjunction with seeding.
 - 14. All channel relocations are to be constructed during the earliest stage of construction and shall be constructed in accordance with all applicable permit requirements and shall be constructed in the dry wherever possible. Stabilization or vegetation shall be established before flow is redirected through the constructed area as directed by the Engineer.
 - 15. The contractor shall plan and implement his land disturbance operations in order to:
 - a. Control the volume and velocity of stormwater runoff within the site to minimize erosion.
 - b. Control the peak flow rates, volume and velocity of stormwater discharges to minimize erosion at outlets and in downstream channels.
 - c. Minimize the amount of soil exposed.
 - d. Minimize the disturbance of steep slopes.
 - e. Minimize sediment discharge from the site.
 - f. Provide and maintain natural buffers around surface waters, direct stormwater runoff to vegetated areas and maximize stormwater infiltration, unless infeasible.
 - g. Minimize soil compaction (except in those areas where compaction is required by the contract documents) and preserve topsoil where feasible.

- XX 16. The name of the individual(s) or contractor(s) responsible for the installation and maintenance of the erosion and sediment control measures shall be supplied by the contractor and maintained with the other SWPPP documents for this land disturbance (construction) activity.
 - 17. Soil stockpiles temporarily placed within the project area or on VDOT right of way or easement shall be identified, stabilized, and protected with sediment trapping measures.
 - 18. A construction entrance or other approved measure shall be installed at all locations where construction vehicular traffic access routes intersect a paved or a public road in order to minimize the transport of sediment by vehicular tracking onto the paved surface. Where sediment is transported onto a paved or a public road surface, the road shall be cleaned thoroughly at the end of each work day by shoveling or sweeping. Removed sediment shall be disposed of in accordance with Section 106.04 of the R&B Specifications.
 - 19. Any variance, exception or deviation approved by DEQ must be listed below and supporting documentation (exception/variance/deviation request and DEQ approval) must be maintained with the SWPPP.
 - The following exceptions to the Water Quantity criteria of the VSMP Regulation have been approved by the DEQ for this land disturbance (construction) activity: (list all approved exceptions and include a brief description of the exception, the date approved and the approving DEQ Office)

| - | Type(1) | Regulation Modified(2) | Approval Date(3) | Description of Variance |
|---|---------|------------------------|------------------|-------------------------|
| | | | | |
| | | | | |
| | | | | |

- (1) Type of modification (Variance from ESC regulations, or Deviation from published guidance)
- (2) Section of Regulation or Guidance Document Modified (e.g. ESC Min. Std. 15) (3) Date that variance/exception/deviation was approved by DEQ.

SECTION III POST CONSTRUCTION STORMWATER MANAGEMENT

Choose the appropriate note 1A or 1B that is applicable to the proposed post construction SWM Plan for this land disturbance (construction) activity. (Delete, strikethrough or mark as NA those notes not applicable.)

- 1. (Include one of the following notes as appropriate)
- X A. This land disturbance activity is grandfathered under Section 9VAC25-870-48 of the VSMP Regulations and utilizes the Part IIC technical criteria (i.e., Performance or Technology Based, MS 19, etc.) in Section 9VAC25-870-93 et seq. of the VSMP Regulations.
- **B. This land disturbance activity utilizes the Part IIB technical criteria (i.e., Runoff Reduction Method, Energy Balance Equation, etc.) in Section 9VAC25-870-62 et seq. of the VSMP Regulations.

2. An exception for (number) pounds of phosphorus removal has been granted for this land disturbance activity by the DEQ in its letter dated (date).

3. Any variance, exception or deviation approved by DEQ must be listed below and supporting documentation (exception/variance/deviation request and DEQ approval) must be maintained with the SWPPP.

The following exceptions to the Water Quantity criteria of the VSMP Regulation have been approved by the DEQ for this land disturbance activity: (list all approved exceptions and include a brief description of the exception, the date approved and the approving DEQ Office)

| Type(1) | Regulation Modified(2) | Approval Date(3) | Description of Waiver |
|---------|------------------------|------------------|-----------------------|
| | | | |
| | | | |
| | | | |

- (1) Type of modification (Variance, or Exception from SWM Regulations or Deviation from published guidance)
- (2) Section of Regulation or Guidance Document Modified (e.g. ESC Min. Std. 15) (3) Date that variance/exception/deviation was approved by DEQ.
- 4. The permanent onsite SWM facilities or offsite strategies proposed to meet the water quality/quantity requirements for this land disturbance (construction) activity are listed in Section VI.

| REVISED | STATE | | STATE | SHEET NO |
|---------|-------|-------|----------------------|----------|
| | SIHIE | ROUTE | PROJECT | SHEET NO |
| | VA. | / | 0001-212-249, PE-101 | 2H(2) |

SWPPP - Stormwater Pollution Prevention Plan

VDOT - Virginia Department of Transportation

VSMP - Virginia Stormwater Management Program

VPDES - Virginia Pollutant Discharge Elimination System

VESCP - Virginia Erosion and Sediment Control Program

TMDL - Total Maximum Daily Load

WLA - Waste Load Allocation

SWM - Stormwater Management

DESIGN FEATURES RELATING TO CONSTRUCTION
OR TO REGULATION AND CONTROL OF TRAFFIC
MAY BE SUBJECT TO CHANGE AS DEEMED
NECESSARY BY THE DEPARTMENT

Johnson, Mirmiran & Thompson Richmond, Virginia HYDRAULIC ENGINEER

- 5. A description of all post-construction stormwater management measures that will be installed during the construction process to control pollutants in stormwater discharges after construction operations have been completed is included in the construction plan set (or other such documents) for this land disturbance (construction) activity.
- 6. All engineering calculations supporting the design of the post-construction stormwater management measures for this land disturbance (construction) activity, including an explanation of the technical basis used to select the practices, are contained in the project drainage file located in the (insert appropriate location, i.e., VDOT Central Office Hydraulics Section or the VDOT (specify) District Hydraulics Section or the VDOT (specify) Residency Office) and will be made available for review upon request during normal working business hours.

ACRONYMS

CBPA - Chesapeake Bay Preservation Act

BMP - Best Management Practice

DEQ - Department of Environmental Quality
EPA - U.S. Environmental Protection Agency

EPA - U.S. Environmental Protection Agency
ESC - Erosion and Sediment Control

IIM - Instructional and Informational Memorandum R&B - Road and Bridge

PLANS

RLD - Responsible Land Disturber

X Denotes information that is to be provided/ completed by the RLD.

XX Denotes information that is to be provided/completed by the contractor.

PROJECT SHEET NO. 2H(2)

STORMWATER POLLUTION PREVENTION PLAN (SWPPP) GENERAL INFORMATION SHEET

The information contained in the SWPPP GeneralInformation sheets is intended to comply with the requirements of the VPDES GeneralPermit For Discharges Of Stormwater From Construction Activities (the VPDES Construction Permit) issued July 1, 2019 and VDOT's approved AnnualESC and SWM Standards and Specifications.

The SWPPP General Information sheets are to be completed and included in the construction plan set (or other such documents) for land disturbance (construction) activities that disturb an area equal to or greater than 10,000 square feet outside the Chesapeake Bay Preservation Area, or equal to or greater than 2,500 square feet in the area defined as Tidewater, Virginia in the Virginia Chesapeake Bay Preservation Act.

The VDOT RLD will ensure that the information shown on the SWPPP General Information sheets is updated/revised as necessary in order to reflect changes that may occur during the construction phase of the land disturbing (construction) activity. The updated/revised sheets shall be maintained with the designated record set of plans (or other such documents) for the land disturbance (construction) activity.

SECTION IV SWPPP

1. All documents related to the SWPPP for this land disturbance (construction) activity shall be maintained at the activity site and shall be readily available for review upon request during normal business hours. Such documents include, but are not limited to, the construction plans (or other such documents), the ESC Plan, the Pollution Prevention Plan, the post construction SWM Plan (if applicable), the VDOT R&B Standards and Specifications, Supplemental Specifications, Special Provisions and Special Provision Copied Notes. Documents related to stormwater pollution prevention which are not a part of those documents referenced above, such as copies of the VPDES Construction Permit coverage letter (when applicable) and the VPDES General Permit For Discharges Of Stormwater From Construction Activities (when applicable) and those required to be developed by the contractor for pollution prevention associated with any on-site support facilities being included in the VPDES Construction Permit coverage for this land disturbance (construction) activity are to be maintained at the activity site with the other SWPPP documents for this land disturbance (construction) activity. Where no facilities are available at the activity site to maintain the SWPPP documents, they are to be kept by or with the designated RLD at a location convenient to the activity site where they would be made available for review upon request during normal business hours.

- 2. The SWPPP and any subsequent amendments, modifications and updates shall be implemented from commencement of land disturbance until termination of VPDES Construction Permit coverage or completion of land disturbance (construction) activities where no VPDES Construction Permit coverage is required.
- XX 3. For all on-site support facilities that will be included in the VPDES Construction Permit coverage for this land disturbance (construction) activity, the contractor shall develop a SWPPP in accordance with, but not limited to, Section 106.08, 107.02 and 107.16 of the VDOT Road and Bridge Specifications. The SWPPP for the on-site support facilities shall be maintained with and become a component of the SWPPP for this land disturbance (construction) activity. Support facilities shall include, but not be limited to, borrow and disposal areas, construction and waste material storage areas, equipment and vehicle washing, maintenance, storage and fueling areas, storage areas for fertilizers, fuels or chemicals, concrete wash out areas, sanitary waste facilities and any other areas that may generate a stormwater or non-stormwater discharge directly related to the construction site.
 - 4. For those land disturbing (construction) activities requiring coverage under the VPDES Construction Permit, the SWPPP shall be made available for review upon the request of the DEQ, the EPA, the VSMP Authority, the VESCP Authority, local government officials or the operator of a municipal separate storm sewer system (MS4) receiving discharge from the construction site.
- X 5. For those land disturbing (construction) activities requiring coverage under the VPDES Construction Permit, the VDOT RLD shall post, or have posted, a copy of the General Permit coverage letter and a copy of a completed LD-445A form, noting the name and contact information for the VDOT person responsible for the land disturbing (construction) activity and its SWPPP, outside the project's construction office along with other Federal and State mandated information. Where there is no construction office (e.g., a maintenance activity), the permit coverage letter and the LD-445A form are to be maintained with the other SWPPP documents for the land disturbing (construction) activity.

6. The SWPPP shall be made available for review by the public upon request. Such reviews shall be at a time and publicly accessible location convenient to the VDOT and shall be scheduled during normal business hours and no less than once per month.

SECTION V - POLLUTION PREVENTION PLAN

- 1. The following non-stormwater discharges from this land disturbing (construction) activity and any on-site support facilities are prohibited:
 - a. Wastewater from concrete washouts.
 - b. Wastewater from the washout and cleanout of stucco, paint, from release oils, curing compounds and other construction materials.
 - c. Fuels, oils or other pollutants used in vehicle and equipment operation and maintenance.
 - d. Oils, toxic substances or hazardous substances from spills or other releases.
 - e. Soaps, solvents or detergents used in equipment and vehicle washing.
 - f. There shall be no discharge of floating solids or visible foam in other than trace amounts
- 2. The following non-stormwater discharges from this land disturbing (construction) activity and any on-site support facilities are allowed when discharged in compliance with the VPDES Construction Permit:
 - a. Discharges from firefighting activities.
 - b. Fire hydrant flushings.

I. Landscape irrigation.

- c. Water's used to wash vehicles or equipment where soaps, solvents or detergents have not been used and the wash water has been filtered, settled or similarly treated prior to discharge.
- d. Water used to control dust that has been filtered, settled or similarly treated prior to discharge.
- e. Potable water sources including uncontaminated waterline flushings managed in a manner to avoid stream impacts.
- f. Routine external building wash down where soaps, solvents or detergents have not been used and the wash water has been filtered, settled or similarly treated prior to discharge.
- g. Pavement wash waters where spills or leaks of toxic or hazardous materials have not occurred (or where all spilled or leaked material has been removed prior to washing), where soaps, solvents or detergents have not been used and where the wash water has been filtered, settled or similarly treated prior to discharge.
- h. Uncontaminated air conditioning or compressor condensate.
- i. Uncontaminated ground water or spring water.
- j. Foundation or footing drains where flows are not contaminated with process materials such as solvents.
- k. Uncontaminated excavation dewatering, including dewatering trenches and excavations that have been filtered, settled or similarly treated prior to discharge.

$\mathbb{X}\mathbb{X}$

3. The contractor shall develop a Pollution Prevention Plan to address any of his onsite operations that have a potential to generate a pollutant that may reasonably be expected to affect the quality of stormwater discharges from this land disturbance (construction) activity. The Pollution Prevention Plan shall be developed in accordance with, but not limited to, Sections 106.08, 107.02 and 107.16 of the VDOT Road and Bridge Specifications and shall include a narrative with appropriate plan detail and shall be provided on standard 8.5 x 11 inch paper or larger and shall:

- a. Identify the potential pollutant-generating activities and the pollutant that is expected to be exposed to stormwater.
- b. Describe the location where the potential pollutant-generating activities will occur, or if identified on the record set of plans, reference the record set of plans.
- c. Identify all non-stormwater discharges, as described in note two of this section, that are or will be commingled with stormwater discharges from the construction activity, including any on-site support activities.
- d. Identify the person(s) or contractor(s) responsible for implementing and maintaining the pollution prevention practice or practices for each pollutant-generating activity.
- e. Describe the pollution prevention practices and procedures that will be implemented to:
 - 1) Prevent and respond to leaks, spills, and other releases, including procedures for expeditiously stopping, containing, and cleaning up spills, leaks, and other releases, and procedures for reporting leaks, spills, and other releases in accordance with Section 107.16 of the VDOT Road and Bridge Specifications and the requirements within the VPDES Construction Permit.

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DESIGN FEATURES RELATING TO CONSTRUCTION

OR TO REGULATION AND CONTROL OF TRAFFIC

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NECESSARY BY THE DEPARTMENT

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- 2) Prevent the discharge of spilled and leaked fuels and chemicals from vehicle fueling and maintenance activities.
- 3) Prevent the discharge of soaps, solvents, detergents, and wash water from construction materials, including procedures for the clean-up of stucco, paint, form release oils, and curing compounds.
- 4) Minimize the discharge of pollutants from vehicle and equipment washing, wheel wash water, and other types of washing.
- 5) Direct concrete wash water into a leak-proof container or leak-proof settling basin. The container or basin shall be designed so that no overflows can occur due to inadequate sizing or precipitation.

 Hardened concrete wastes shall be removed and disposed of in a manner consistent with the handling of other construction wastes.

 Liquid concrete wastes shall be removed and disposed of in a manner consistent with the handling of other construction wash waters and shall not be discharged to surface waters.
- 6) Minimize the discharge of pollutants from storage, handling, and disposal of construction products, materials, and wastes including building products (such as asphalt sealants, copper flashing, roofing materials, adhesives, and concrete admixtures), pesticides, herbicides, insecticides, fertilizers, landscape materials, construction and domestic wastes (such as packaging materials), scrap construction materials, masonry products, timber, pipe and electrical cuttings, plastics, styrofoam, concrete, and other trash or building materials.
- 7) Prevent the discharge of fuels, oils, and other petroleum products, hazardous or toxic wastes, waste concrete and sanitary wastes.
- 8) Address any other discharge from any potential pollutant-generating activity not listed herein.
- 9) Minimize the exposure of waste materials to precipitation by closing or covering waste containers during precipitation events and at the end of the business day, or implementing other similarly effective practices. Minimization of exposure is not required in case where the exposure to precipitation will not result in a discharge of pollutants.
- 10) Describe and implement procedures for providing pollution prevention awareness (including but not limited to prevention practices, disposal practices and appropriate disposal locations) for all applicable wastes (including any wash water), to appropriate personnel.
- X Denotes information that is to be provided/completed by the RLD.
- XX Denotes information that is to be provided/completed by the contractor.

PROJECT SHEET NO. 0001-212-249 2H(3)

PROJECT MANAGER_____ SURVEYED BY, DATE DESIGN BY ______ SUBSURFACE UTILITY BY, DATE ______

> The information contained in the SWPPP GeneralInformation sheets is intended to comply with the requirements of the VPDES General Permit For Discharges Of Stormwater From Construction Activities (the VPDES Construction Permit) issued July 1, 2019 and VDOT's approved Annual ESC and SWM Standards and Specifications.

The SWPPP GeneralInformation sheets are to be completed and included in the construction plan set (or other such documents) for land disturbance (construction) activities that disturb an area equal to or greater than 10,000 square feet, or equal to or greater than 2,500 square feet in the area defined as Tidewater, Virginia in the Virginia Chesapeake Bay Preservation Act.

STORMWATER POLLUTION PREVENTION PLAN (SWPPP) GENERAL INFORMATION SHEET

The VDOT RLD will ensure that the information shown on the SWPPP General Information sheets is updated/revised as necessary in order to reflect changes that may occur during the construction phase of the land disturbing (construction) activity. The updated/revised sheets shall be maintained with the designated record set of plans (or other such documents) for the land disturbance (construction) activity.

SECTION VI - PERMANENT BMP INFORMATION \triangle

X Denotes information that is to be completed by the RLD. () See note referenced by number in parentheses.

REVISED SHEET NO ROUTE PROJECT 0001-212-249, PE-101 2H(4) VA.

DESIGN FEATURES RELATING TO CONSTRUCTION OR TO REGULATION AND CONTROL OF TRAFFIC MAY BE SUBJECT TO CHANGE AS DEEMED NECESSARY BY THE DEPARTMENT

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INSTALLED BMP INFORMATION (VDOT Owned/Operated)

| Plan Sheet(s) | Date BMP Made Functional | Type of BMP Installed (See Table A and C) | Geographic Location (County or City) | Latitude/Longitude (1) LAT LONG | VA 6th Order HUC (7) | Receiving Water (2) | Name of Impaired Water (9) | Acres Treated Per BMP (3) Impervious Pervious TOTAL | X BMP Maintenance ID Number (10) | BMP Maintenance Manual (11) SECTION | BMP Inspection Manual (11) SECTION |
|---------------|-----------------------------|--|---|---------------------------------------|-------------------------------|------------------------|-------------------------------|--|--|--|---|
| | | | | | | | | | | | |
| 5 | | Bio-Retention Basin | Dumfries | 38.5566 -77.3317 | PL52 | Qunatico Creek | n/a | 2.66 1.15 3.81 | | 4.1.1 | 4.1.1 |
| 10 | | Bio-Retention Basin | Dumfries | 38.5647 -77.3266 | PL52 | Qunatico Creek | n/a | 3.18 0.92 4.10 | | 4.1.1 | 4.1.1 |
| 12 | | Bio-Retention Basin | Dumfries | 38.5668 -77.3232 | PL52 | Qunatico Creek | n/a | 2.57 0.94 3.51 | | 4.1.1 | 4.1.1 |
| 13 | | Bio-Retention Basin | Dumfries | 38.5685 -77.3213 | PL52 | Qunatico Creek | n/a | 1.96 0.89 2.85 | | 4.1.1 | 4.1.1 |
| 15/15C | | Bio-Retention Basin | Dumfries | 38.5717 -77.3175 | PL52 | Qunatico Creek | n/a | 1.82 0.99 2.81 | | 4.1.1 | 4.1.1 |
| | | | | | | | | | | | |
| | | | | | | | | | | | |
| | | | | | | | | | | | |

ALTERNATIVE BMP INFORMATION

| Plan Sheet(s) | Date | Type of BMP Installed (See Table B) | (County or City) (5) | Latitude/Longitude (1) (5) | Order HUC (5) (7) | Receiving Water (2) | Name of Impaired Water (9) |
|---------------|------|--|-------------------------|-------------------------------|-------------------------|------------------------|-------------------------------|
| | | | | LAT LONG | | | |
| | | | | | | | |
| | | | | | | | |
| | | | | | | | |
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| | | | | | | | |

Perpetual Nutrient Credits Acquired for Project

| | · |
|-------------------|------------------|
| Name of | Nutrient Credits |
| Nutrient Credit | (lbs./TP./year) |
| Generating Entity | Acquired |
| (6) | (6) (12) |
| | |

| SWM #1 | 3.83 |
|--------|------|
| SWM #4 | 4.54 |
| SWM #6 | 3.69 |
| SWM #5 | 2.83 |
| SWM #8 | 2.64 |
| | |
| | |
| | |
| | |
| | |

necessitated during the construction phase of the project that affects the proposed construction details or potentially affects the informationshown in the BMP Tables A and/or B shall be coordinated by the VDOT RLD with the appropriate VDOT District Hydraulics Engineer. The construction plans and the BMP Tables A and/or B are to be formally revised to reflect any authorized/ approved changes to the proposed SWM Plan and/or the proposed BMP construction details. All plan revisions shall be completed in accordance with the Road Design Manual and the Construction Division IIM-CD-2013-12.01, signed and sealed in accordance with Department's sealing and signing policy IIM-LD-243 and filed with the construction record drawings maintained in the VDOT Central Office Plan File Room (ProjectWise). Prior to submitting for termination of coverage under the VPDES General Permit For The Discharge Of Stormwater From Construction Activities, the RLD shall have the District Maintenance Division review the BMPs installed with the project (BMP Table A) for acceptance of maintenance responsibility and to obtain a Maintenance ID number for each BMP listed in BMP Table A. The RLD shall use the information in BMP Tables A and B along with the assigned Maintenance ID number and the date that the BMP became functional as a permanent control measure (for BMPs in Table A only) to complete the LD-445D form when certifying the construction of the BMPs and submitting for termination of coverage under the VPDES General Permit For The Discharge Of Stormwater From Construction Activities.

/\ Any changes to the proposed SWM Plan or BMPs

Table A: Permanent BMP Types (1999 Va. SWM Handbook)

Bio-retention Basin Bio-retention Filter Constructed Stormwater Wetlands Extended Detention Basin Extended Detention Basin Enhanced Grassed Swale Infiltration Basin Infiltration Trench Manufactured Treatment Device (MTD) (8) Retention Basin I Retention Basin II Retention Basin III Sand Filter Vegetated Filter Strip Other Approved Types (List Type)

Table B: Alternative BMP Types Comprehensive SWM Plan (Regional) Facility Pollutant Loading Pro Rata Share Program Other Approved Options (List Type) (4)

Detention Basin

Table C: Permanent BMP Types (BMP Clearing House)

Sheet Flow to Vegetated Filter Strip Grass Channel Soil Compost Amendment Permeable Pavement (Level 1) Permeable Pavement (Level 2) Infiltration Practice (Level 1) Infiltration Practice (Level 2) Bioretention (Level 1) Bioretention (Level 2) Dry Swale (Level 1)

Geographic Location

Dry Swale (Level 2) Wet Swale (Level 1) Wet Swale (Level 2) Filtering Practice (Level 1)

Filtering Practice (Level 2) Constructed Wetlands (Level 1) Constructed Wetlands (Level 2) Extended Detention Pond (Level 1) Extended Detention Pond (Level 2) Wet Pond (Level1) Wet Pond (Level2)

Manufactured Treatment Device (MTD)(8) Other Approved Types (List Type)

VA 6th

(1) In decimal degrees to the nearest one ten-thousandth of a degree.

(2) For streams with no names, list "(Unnamed Tributary to downstream name)".

(3) Show acres treated to the nearest one hundreths acre.

(4) Include agreements with off-site BMP owners.

(5) Information pertains to the alternative BMP option location, where applicable. Exception - Not required for nutrient credit purchase option.

(6) Applies to the purchase of nutrient credits only.

(7) Virginia 6th Order HUC (VAHU6) Example - YO30.

(8) Final approved shop drawings of Manufactured Treatment Devices (MTDs) are to be included with the BMP information submitted with the LD-445D form.

(9) List the name of any impaired water to which the BMP discharges. The determination of impaired water shall be based on those streams listed as impaired in the DEQ 2012 305(b)/303(d) Water Quality Assessment Integrated Report and shall be the first named waterbody to which the BMP discharges. The impaired waters are those impaired by sediment, total suspended solids, turbidity, nitrogen or phosphorus.

(10) BMP Maintenance ID Number is to be assigned by the District Maintenance Division at permit termination or project completion. This ID number shall be assigned prior to the permit close out process and entered by the area construction engineer under this column, per IIM-LD-95

(11) Provide the section of each Maintenance manual that pertains to the type of BMP. Both manuals can be found at www.vdot.virginia.gov/ business/manuals in the Maintenance selections. Example: Section 4 would be noted for both the maintenance and inspection manuals for a Bioretention Linfiltration BMP.

(12) Nutrient credits purchased to the nearest one hundredth pound.

Revised 5/1/19

PROJECT SHEET NO. 0001-212-249 2H(4)

PLANS

jinia 23236) 323–9900

LANDSCAPING & PLANTING PLAN

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VA. I OOOI-212-249, PE-IOI 2i(I)

DESIGN FEATURES RELATING TO CONSTRUCTION
OR TO REGULATION AND CONTROL OF TRAFFIC
MAY BE SUBJECT TO CHANGE AS DEEMED
NECESSARY BY THE DEPARTMENT

VDOT Location & Design Richmond, Virginia HYDRAULIC ENGINEER

Landscape Planting Notes

I. Plants shall be in accordance with the current edition of the 'American Standard for Nursery Stock' published by the American Association of Nurserymen and conform in general to the representative species.

2. Substitutions shall not be made without prior written approval from the owner.any substitutions made without this approval may be subject to rejection and removal at the contractor's expense.

3. Plant material shall be subject to inspection and approval by owner or owner's representative for conformity to specification requirements as to quality, size, and variety. plants damaged in handling or transportation may be rejected by the owner.

4. All plants shall be nursery grown in accordance with good horticultural practice and be free of plant disease, insects, eggs and larvae and shall have healthy root systems. plants shall be obtained from sites which are similar in soil and climatic conditions as those of the project site.

5. All plant sizes shall be at least as specified in the plant schedule, but in no case shall any plant be less than specified without written approval from the owner.

6. Container grown stock shall have been grown in the container long enough for the root system to have developed sufficiently to hold its soil together.

7. All plant material in transit shall be covered with burlap or similar cover to keep it from wind damage and drying out.

8. Do not remove container grown stock from containers until planting time.

9. Shredded hardwood bark mulch (shbm) 3 inches deep shall be placed to the limits of the bioretention soil mix (bsm) where trees, shrubs, flowering perennials or ornamental grasses are proposed on the landscape plans. Ddo not place the mulch up against a tree's trunk or the stems of shrubs, perennials or ornamental grasses. reduce the mulch depth immediately adjacent to the trunk or stem.

10. Offsets indicated in these notes are to the center of the plant.

II. Ornamental grass and perennial plants near the outside edge of the bsm shall be offset a minimum of 12 inches inside the edge of the bsm.

I2. Ornamental grasses and perennial plants shall be offset a minimum of I8 inches from the center of cleanouts or observation wells.

13. Landscape plans are only for landscape planting purposes. Refer to other plan sheets for grading, drainage, etc.

14. Contractor shall be responsible during the contract and up to the time of final acceptance for keeping the planting and work incidental thereto in good condition by replanting, plant replacement, watering, weeding, pruning, spraying, and cleaning up and by performing all other necessary operations of care for promotion of good plant growth so that all work is in satisfactory condition at time of final acceptance, at no additional cost to the owner.

I5. The contractor shall remove all dead plant material from the job site on a weekly basis. Contractor shall also be required to retain a log of all plant material removed due to death or injury so as to properly identify those plants for replacement.

I6. All plant material shall be unconditionally guaranteed for one year from the date of initial acceptance. The contractor is not responsible for losses or damage caused by other trades, mechanical injury, or vandalism

17. Any plant material that is 25% or more dead shall be considered dead and shall be replaced at no cost to the owner.

VA Northern Coastal Plain Detention Basin Mix - ERNMX-874

| | Botanical Name | Common Name |
|--------|--|---|
| 3.00 % | Panicum anceps, Eastern Shore MD Ecotype | Beaked Panicgrass, Eastern Shore MD Ecotype |
| 9.00 % | Elymus virginicus, PA Ecotype | Virginia Wildrye, PA Ecotype |
| 7.00 % | Panicum virgatum, NJ Ecotype | Switchgrass, NJ Ecotype |
| 5.00 % | Juncus effusus, Coastal Plain NC Ecotype | Soft Rush, Coastal Plain NC Ecotype |
| 2.00 % | Agrostis hyemalis, Piedmont NC Ecotype | Winter Bentgrass, Piedmont NC Ecotype |
| 2.00 % | Helenium autumnale, PA Ecotype | Common Sneezeweed, PA Ecotype |
| 2.00 % | Helenium flexuosum, VA Ecotype | Purplehead Sneezeweed, VA Ecotype |
| | | |

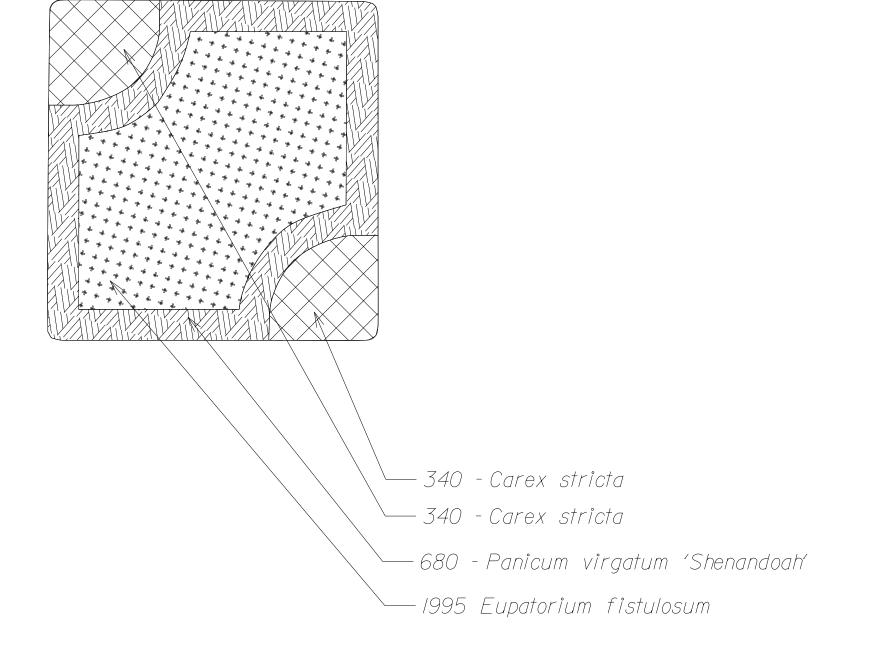
100.00 %

Seeding Rate: Seed at 20 lbs/acre with a cover crop. For a

cover crop use Japanese Millet (10 lbs/acre; 1 May to 31 Aug), Barnyard Grass (10 lbs/acre; 1 May to 31 Aug), or Grain Rye (30 lbs/acre; 1 Sept to 30 Apr).

Stormwater Management

Mix formulations are subject to change without notice depending on the availability of existing and new products. While the formula may change, the guiding philosophy and function of the mix will not.



PROJECT SHEET NO. **2i(1)**

PROJECT MANAGER*Susie_Lue (70.3)259-2918_NOVA_District*SURVEYED BY, DATE *Leon_E_Treutle_LS_(70.3)259-3224_7/17/13______*DESIGN BY *JMT_Engineering_(804)_323-9900_____*SUBSURFACE UTILITY BY, DATE *Leon_E_Treutle_LS_(70.3)259-3224_7/17/13*

BIORETENTION CONSTRUCTION SEQUENCE

REVISED
STATE
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PROJECT

VA. / OOOI-212-249,PE-IOI 21(2)

DESIGN FEATURES RELATING TO CONSTRUCTION

DESIGN FEATURES RELATING TO CONSTRUCTION
OR TO REGULATION AND CONTROL OF TRAFFIC
MAY BE SUBJECT TO CHANGE AS DEEMED
NECESSARY BY THE DEPARTMENT

VDOT Location & Design Richmond, Virginia HYDRAULIC ENGINEER

Construction Sequence

- I. Bioretentions to be used as Sediment Basins at proposed grading during construction activities.
- 2. Construction of the bioretention may only begin after the entire contributing drainage area has been stabilized with vegetation. It may be necessary to block certain curb or other inlets while the bioretention is being constructed. The proposed site should be checked for existing utilities prior to any excavation.
- 3. The designer and the installer should have a preconstruction meeting, checking the boundaries of the contributing drainage area and the actual inlet elevations to ensure they conform to original design. Since other contractors may be responsible for constructing portions of the site, it is quite common to find subtle differences in site grading, drainage and paving elevations that can produce hydraulically important differences for the proposed bioretention area. The designer should clearly communicate, in writing, any project changes determined during the preconstruction meeting to the installer and the plan review/inspection authority.
- 4. Temporary E&S controls are needed during construction of the bioretention area to divert stormwater away from the bioretention area until it is completed. Special protection measures such as erosion control fabrics may be needed to protect vulnerable side slopes from erosion during the construction process.
- 5. Any pre-treatment cells should be excavated first and then sealed to trap sediments.
- 6. Excavators or backhoes should work from the sides to excavate the bioretention area to its appropriate design depth and dimensions. Excavating equipment should have scoops with adequate reach so they do not have to sit inside the footprint of the bioretention area. Contractors should use a cell construction approach in larger bioretention basins, whereby the basin is split into 500 to 1,000 sq.ft. temporary cells with a 10-15 foot earth bridge in between, so that cells can be excavated from the side.
- 7. It may be necessary to rip the bottom soils to a depth of 6 to 12 inches to promote greater infiltration.
- 8. Place geotextile fabric on the sides of the bioretention area with a 6-inch overlap on the sides. If a stone storage layer will be used, place the appropriate depth of #57 stone on the bottom, install the perforated underdrain pipe, pack #57 stone to 3 inches above the underdrain pipe, and add approximately 3 inches of choker stone/pea gravel as a filter between the underdrain and the soil media layer. If no stone storage layer is used, start with 6 inches of #57 stone on the bottom, and proceed with the layering as described above.
- 9. Deliver the soil media from an approved vendor, and store it on an adjacent impervious area or plastic sheeting. Apply the media in 12-inch lifts until the desired top elevation of the bioretention area is achieved. Wait a few days to check for settlement, and add additional media, as needed, to
- 10. Prepare planting holes for any trees and shrubs, install the vegetation, and water accordingly. Install any temporary irrigation.
- II. Place the surface cover in both cells (mulch, river stone or turf), depending on the design. If coir or jute matting will be used in lieu of mulch, the matting will need to be installed prior to planting (Step 10), and holes or slits will have to be cut in the matting to install the plants.
- 12. Install the plant materials as shown in the landscaping plan, and water them during weeks of no rain for the first two months.
- I3. Conduct the final construction inspection (see Section 9.2 of the VA DEQ Stormwater BMP Clearinghouse). Then log the GPS coordinates for each bioretention facility and submit them for entry into the local maintenance tracking database.

PROJECT SHEET NO. 0001-212-249 2i(2)

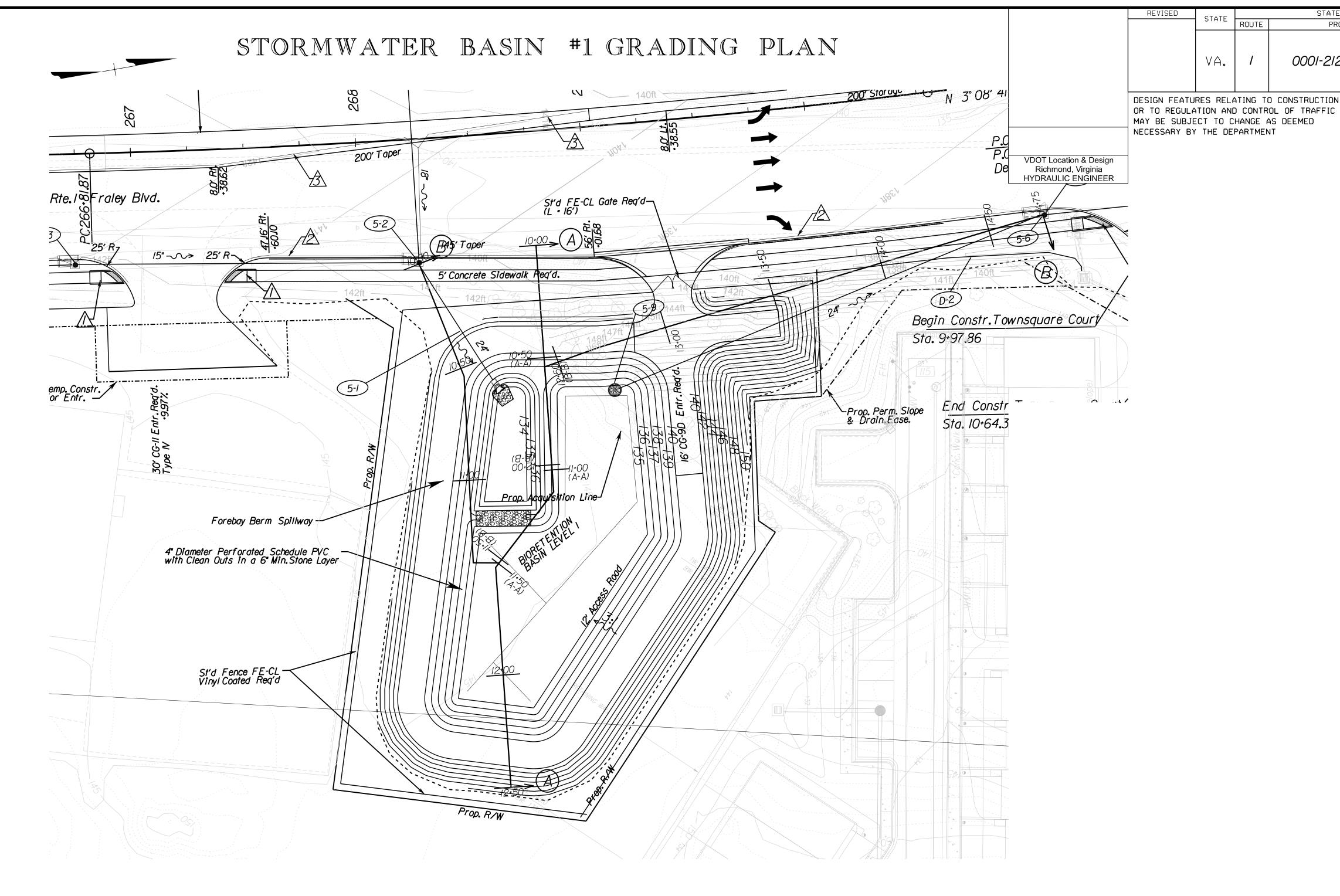
ROUTE

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PROJECT

0001-212-249,PE-101 2i(3)

PROJECT MANAGER*Susie_Lue (703)259-2918_NOVA_District______* SURVEYED BY, DATE Leon E.Treutle LS_(703)259-3224 7/17/13_____ DESIGN BY JMT_ Engineering_(804)_323-9900______ SUBSURFACE UTILITY BY, DATE Lean F. Treutle LS (703)259-3224 7/17/13



"The foundation material under the dam and the material to be used for the embankment of the dam shall be an AASHTO Type A-4 or finer and meet the approval of the materials engineer.

- The contractor shall provide 'As-Built' drawings of all stormwater management facilities. The 'As-Built' drawings shall show the actual finished ground contours, outlet structure dimensions and elevations, etc. as they exist at the completion of the project. These drawings shall be signed and sealed by the Licensed Professional Engineer or Land Surveyor registered in the State of Virginia. All costs shall be included under Construction Surveying.
- The as-built drawings shall be signed and sealed by a Professional Engineer or Land Surveyor.
- SWM I shall be maintained by the Virginia Department of Transportation in accordance with VDOT BMP Maintenance Manual.
- For Profile SWM I (B-B), Cross Section A-A and details see sheet 2i(3A).
- For Drainage Descriptions see sheet 2E(01)-2E(06). For Landscaping and Planting Plan see sheet 2i(1) For Bioretention Construction Sequence see sheet 2i(2)
- Construction Inspections must be conducted in accordance with Non-Proprietary BMPs 9 Bioretention Inspection Checklist, dated April 2018, including completion of the Prince William County Construction Inspection Checklist found at https://www.pwcva.gov/department/environmental-services/stormwater-management. Additionally, the Inspection Checklist shall be kept in the Route I Fraley Boulevard Project SWPPP.

NOTE: FOR TEMPORARY SWM-ISTR.T-5-9 RISER OF 60" AT ELEVATION 137.00' IS REQUIRED. TEMPORARY SWM-ISTRUCTURE SHALL BE CONVERTED TO PERMANENT STRUCTURE 5-9 WITH RISER OF 60" AT ELEVATION 136.00'.

> SHEET NO. PROJECT SCALE 0001-212-249

ROUTE

STATE

PROJECT

0001-212-249,PE-101 |21(3A)

BIORETENTION SOIL MIXTURE, 2.5' DEPTH

12.50

- PEA GRAVEL.0.25' DEPTH

12.00

PROJECT

0001-212-249

SHEET NO.

2i(3A)

REVISED PROJECT MANAGER*Susie_Lue_(7.0.3)259-2918_NOV_A_District_______* SURVEYED BY, DATE Leon E.Treutle LS_(703)259-3224 7/17/13_____ DESIGN BY JMT_ Engineering_(804)_323-9900______ SUBSURFACE UTILITY BY, DATE Lean E. Treutle LS (703)259-3224_7/17/13 BASIN #1 DETAILS, PROFILE, AND CROSS-SECTION DESIGN FEATURES RELATING TO CONSTRUCTION OR TO REGULATION AND CONTROL OF TRAFFIC MAY BE SUBJECT TO CHANGE AS DEEMED NECESSARY BY THE DEPARTMENT VDOT Location & Design Richmond, Virginia HYDRAULIC ENGINEER EXISTING GRADE _ EXISTING GRADE - EXISTING GRADE BERM ELEV 140.00 -12' WIDTH - PROPOSED GRADE - PROPOSED GRADE 5-2 RISER CREST ELEV 136.00 -FOREBAY SPILLWAY VIOO YR.WSE - 137.23 IYR.WSE - 136.46 135 5-9 5-6 FOREBAY BOTTOM ELEV 134,00 205'-24" CONC. SSP CLASS III @ 1.56% SLOPE - 24" CONC. PIPE └ INV.OUT 134.31 4" UNDERDRAIN AT 1% -LONGITUDINAL SLOPE WITH FILTER FABRIC OVER UNDERDRAIN 10.00 10+50 GEOTEXTILE + └_ INV.OUT | 131.00 CROSS SECTION A-A 4" ORIFICE, ELEV 13110 - CONNECT TO UNDERDRAIN CONC.CRADLE -VERTICAL SCALE: I" = 2.5' INV.OUT | 127.91 -HORIZONTAL SCALE: I" = 25' *12*5 //**•5**0 10+50 12.00 12+50 13:00 13•50 14.00 14•50 14.75 //·00 10.00 I PROFILE B-B VERTICAL SCALE: I" = 2.5' HORIZONTAL SCALE: I" = 25' ST'D. STORMWATER MANAGEMENT TRASH RACK SWM-DR - 4" P.V.C.SCREWCAP ON GRADE 4" NONPERFORATED P.V.C.UNDERDRAIN USE 45 DEG.WYE AND FITTING OR EQUIVALENT DIRECTIONAL CONNECTION <u>4" PERFORATED</u> _ _ 4" RIGID SCHEDULE 40
P.V.C.PIPE (OR EQUIV.
CORRUGATED HDPE
FOR BIORETENTION).
3/8" PERFORATIONS AT TO CONNECT UNDERDRAIN 24" RCP OUTFALL TO STANDPIPE 6" ON CENTER. -3.0' DEPTH CONC. FLOW POSITION UNDERDRAIN AT 1% OR 2% SLOPE LOCATED NO MORE THAN 20' FROM THE L = 24' WATERTIGHT CAP EL.128.00 ON TERMINAL END

NEXT PIPE.

UNDERDRAIN CLEANOUT DETAIL

NOT TO SCALE

OF PIPE

SWM RISER DETAIL

NOT TO SCALE

PLANS

SEDIMENT FOREBAY SPILLWAY DETAIL

NOT TO SCALE

ROUTE

DESIGN FEATURES RELATING TO CONSTRUCTION OR TO REGULATION AND CONTROL OF TRAFFIC MAY BE SUBJECT TO CHANGE AS DEEMED

NECESSARY BY THE DEPARTMENT

REVISED

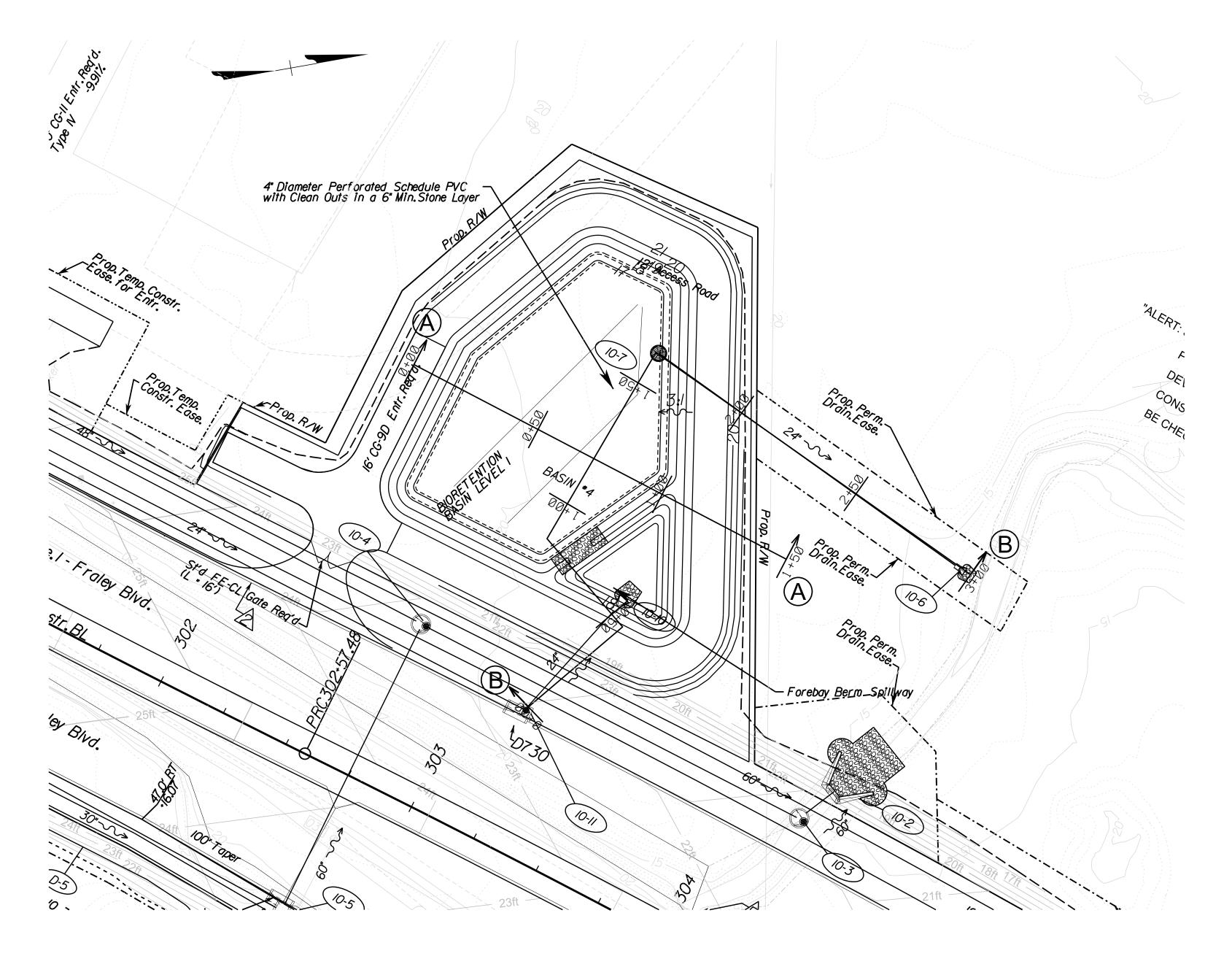
VDOT Location & Design Richmond, Virginia HYDRAULIC ENGINEER STATE

PROJECT

0001-212-249,PE-101 | 2i(4)

PROJECT MANAGERSusie_Lue (7.0.3)259-2918_NOVA_District______ SURVEYED BY, DATE Leon_E_Treutle_LS_(7.0.3)259-3224_7/17/13______ DESIGN_BY_JMT_Engineering_(8.0.4)_3.2.3-9900______ SUBSURFACE_UTILITY_BY, DATE_Leon_E_Treutle_LS_(7.0.3)259-3.2.24_7/17/13

STORMWATER BASIN #4 GRADING PLAN



Notes:

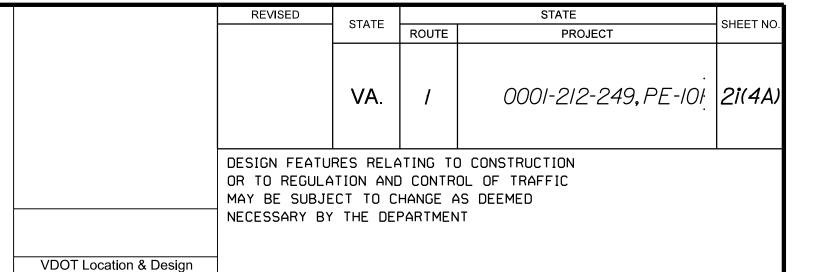
- The foundation material under the dam and the material to be used for the embankment of the dam shall be an AASHTO Type A-4 or finer and meet the approval of the materials engineer.
- 2. The contractor shall provide 'As-Built' drawings of all stormwater management facilities. The 'As-Built' drawings shall show the actual finished ground contours, outlet structure dimensions and elevations, etc. as they exist at the completion of the project. These drawings shall be signed and sealed by the Licensed Professional Engineer or Land Surveyor registered in the State of Virginia. All costs shall be included under Construction Surveying.
- 3. The as-built drawings shall be signed and sealed by a Professional Engineer or Land Surveyor.
- 4. SWM 4 shall be maintained by the Virginia Department of Transportation in accordance with VDOT BMP Maintenance Manual.
- 5. For Profile SWM 4 (B-B), Cross Section A-A and details see sheet 2i(5A).
- 6. For Drainage Descriptions see sheet 2E(I)-2E(6).
 For Landscaping and Planting Plan see sheet 2i(I)
 For Bioretention Construction Sequence see sheet 2i(2)
- 7. Construction Inspections must be conducted in accordance with Non-Proprietary BMPs 9 Bioretention Inspection Checklist, dated April 2018, including completion of the Prince William County Construction Inspection Checklist found at https://www.pwcva.gov/department/environmental-services/stormwater-management. Additionally, the Inspection Checklist shall be kept in the Route I Fraley Boulevard Project SWPPP.

NOTE: FOR TEMPORARY SWM-ISTR.T-10-7 RISER OF 60" AT ELEVATION 18.25' IS REQUIRED. TEMPORARY SWM-ISTRUCTURE SHALL BE CONVERTED TO PERMANENT STRUCTURE 10-7 WITH RISER OF 60" AT ELEVATION 17.50'.

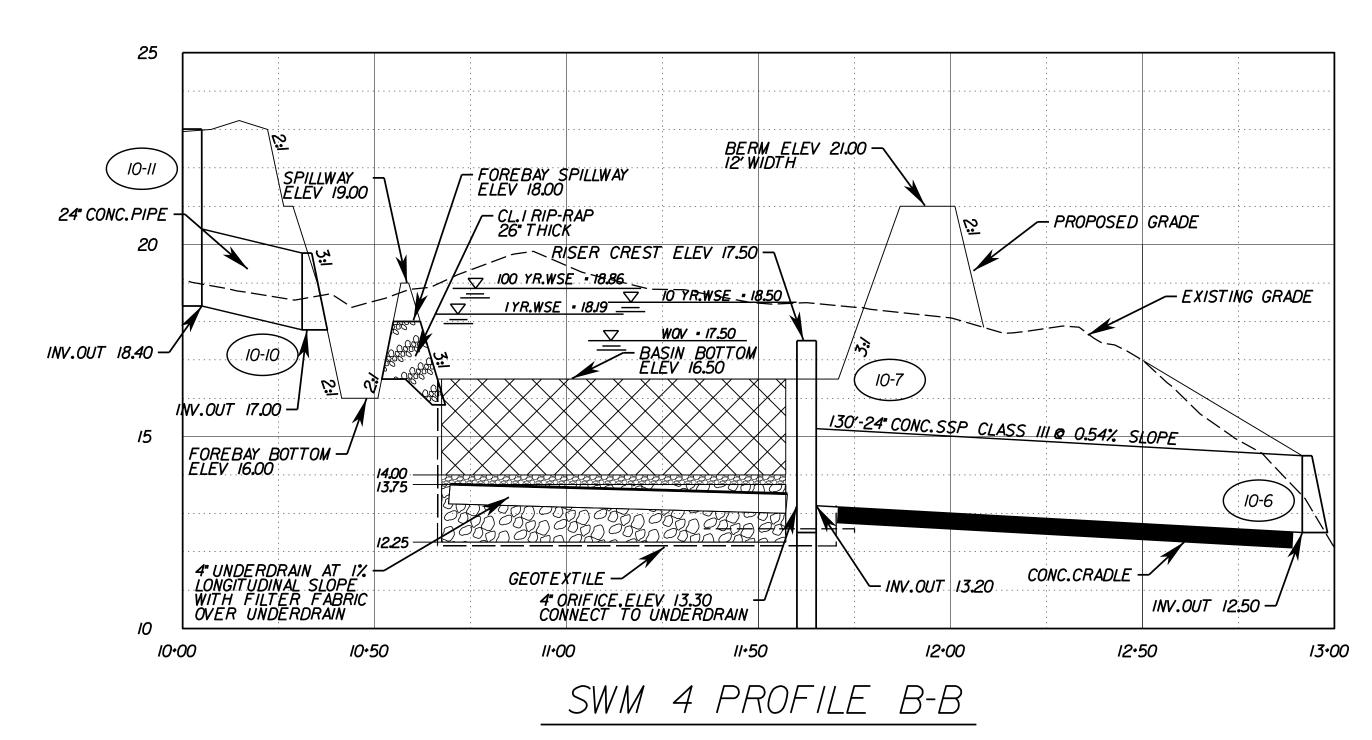
SCALE PROJECT SHEET NO.

0 25' 50' 0001-212-249 2i(4)

BASIN #4 DETAILS, PROFILE, AND CROSS-SECTION

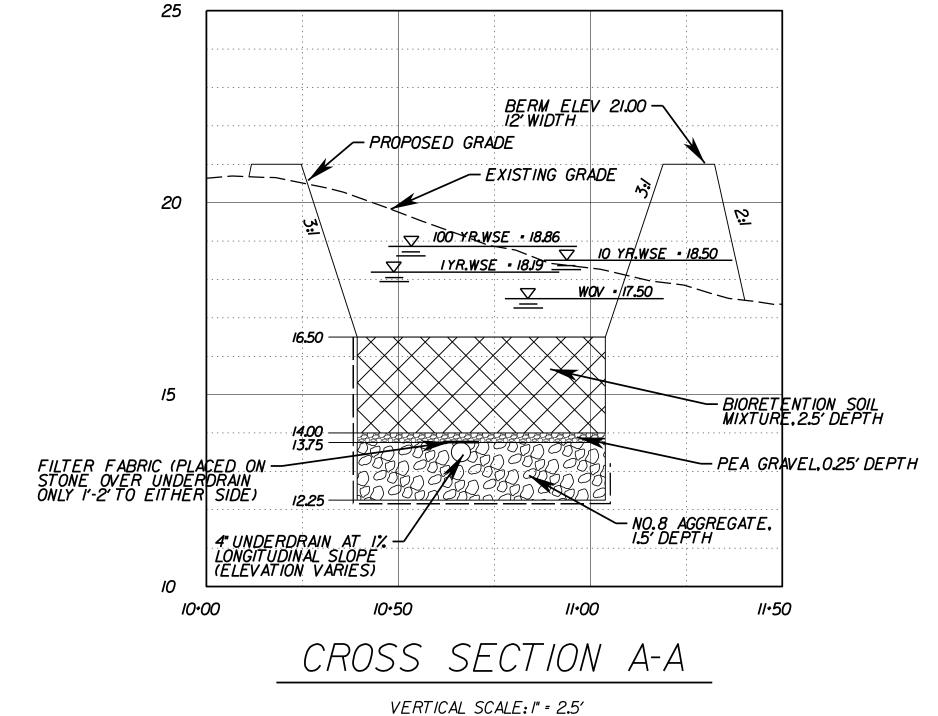


Richmond, Virginia
HYDRAULIC ENGINEER

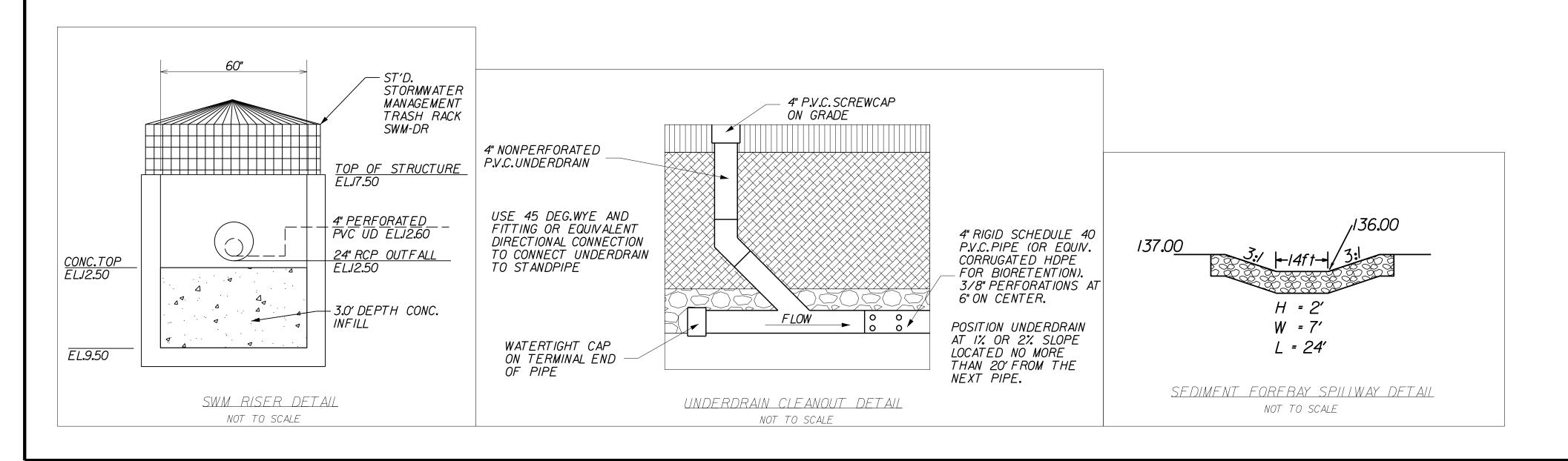


VERTICAL SCALE: I" = 2.5'

HORIZONTAL SCALE: I" = 25'



HORIZONTAL SCALE: I" = 25'



PROJECT SHEET NO. 21(4A)

R/W PLANS

ROUTE

DESIGN FEATURES RELATING TO CONSTRUCTION OR TO REGULATION AND CONTROL OF TRAFFIC MAY BE SUBJECT TO CHANGE AS DEEMED

NECESSARY BY THE DEPARTMENT

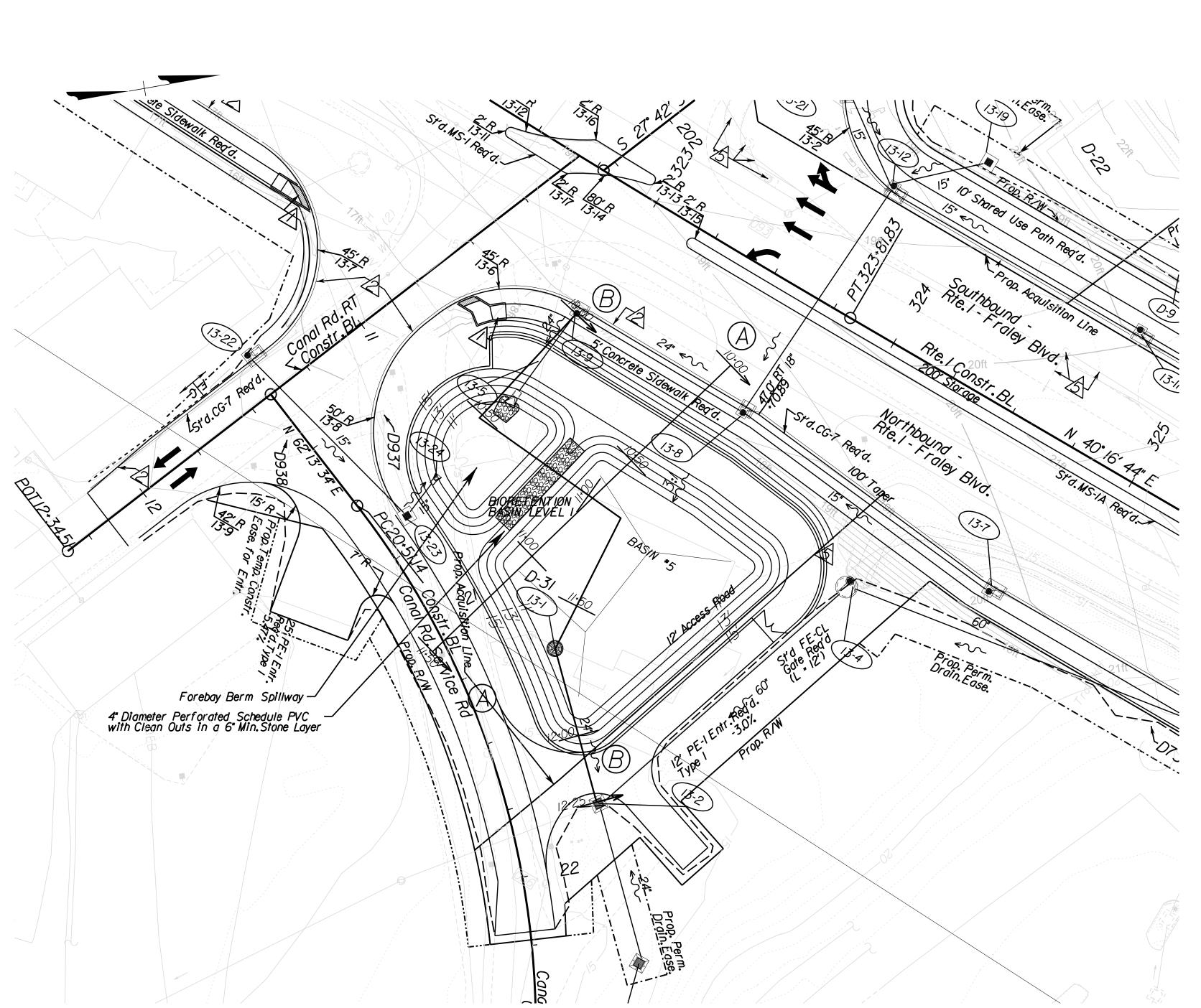
REVISED

VDOT Location & Design Richmond, Virginia HYDRAULIC ENGINEER STATE

PROJECT

0001-212-249,PE-101 | **21(5)**

STORMWATER BASIN #5 GRADING PLAN



Votes:

The foundation material under the dam and the material to be used for the embankment of the dam shall be an AASHTO Type A-4 or finer and meet the approval of the materials engineer.

2. The contractor shall provide 'As-Built' drawings of all stormwater management facilities. The 'As-Built' drawings shall show the actual finished ground contours, outlet structure dimensions and elevations, etc. as they exist at the completion of the project. These drawings shall be signed and sealed by the Licensed Professional Engineer or Land Surveyor registered in the State of Virginia. All costs shall be included under Construction Surveying.

- 3. The as-built drawings shall be signed and sealed by a Professional Engineer or Land Surveyor.
- 4. SWM 5 shall be maintained by the Virginia Department of Transportation in accordance with VDOT BMP Maintenance Manual.
- 5. For Profile SWM 5 (B-B), Cross Section A-A and details see sheet 2i(6A).
- 6. For Drainage Descriptions see sheet 2E(I)-2E(6).
 For Landscaping and Planting Plan see sheet 2i(I)
 For Bioretention Construction Sequence see sheet 2i(2)
- Construction Inspections must be conducted in accordance with Non-Proprietary BMPs 9 Bioretention Inspection Checklist, dated April 2018, including completion of the Prince William County Construction Inspection Checklist found at https://www.pwcva.gov/department/environmental-services/stormwater-management Additionally, the Inspection Checklist shall be kept in the Route I Fraley Boulevard Project SWPPP.

NOTE: FOR TEMPORARY SWM-I STR.T-I3-5 RISER OF 60" AT ELEVATION I2.25' IS REQUIRED. TEMPORARY SWM-I STRUCTURE SHALL BE CONVERTED TO PERMANENT STRUCTURE I3-5 WITH RISER OF 60" AT ELEVATION I2.00'.

SCALE PROJECT SHEET NO. 0001-212-249 21(5)

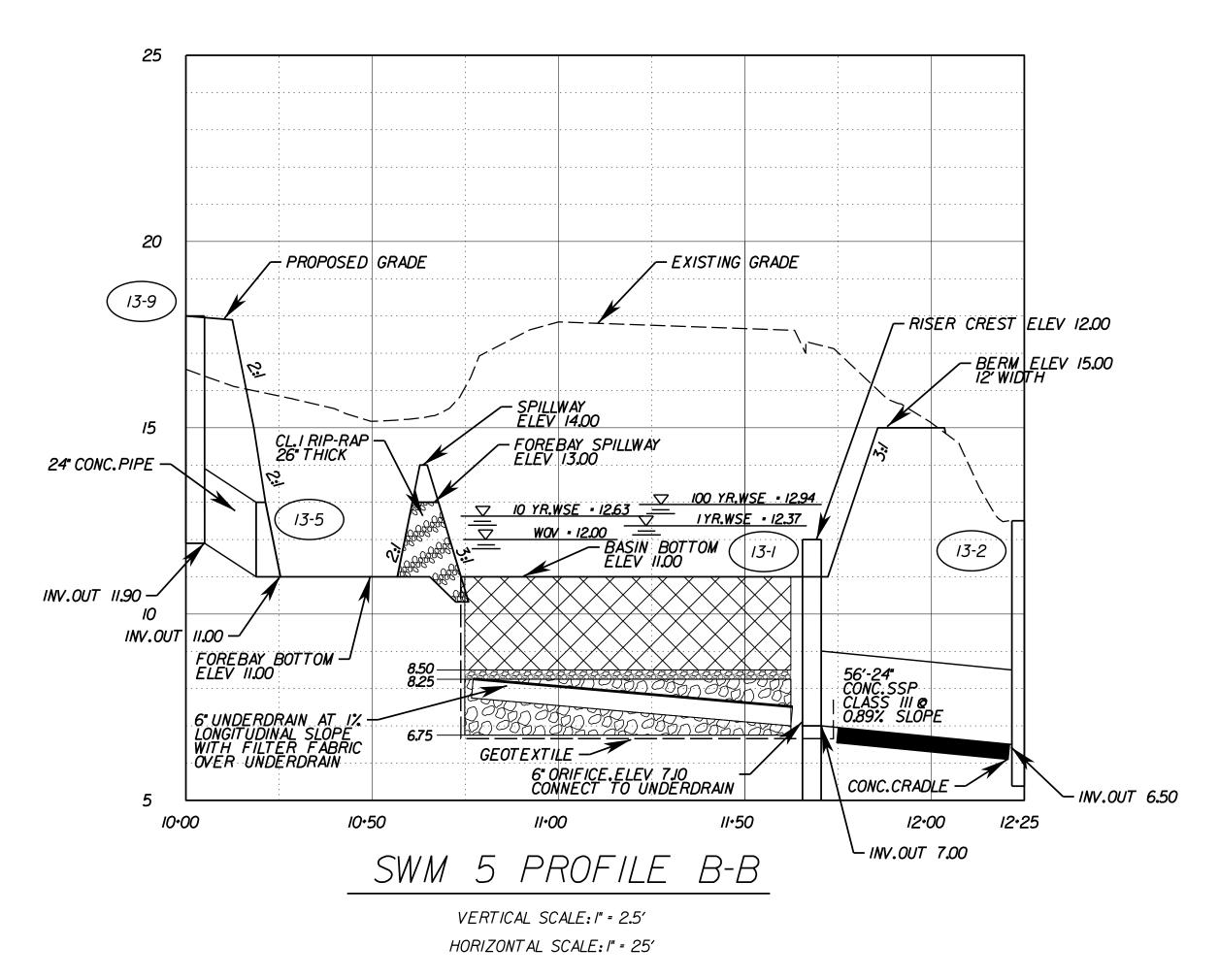
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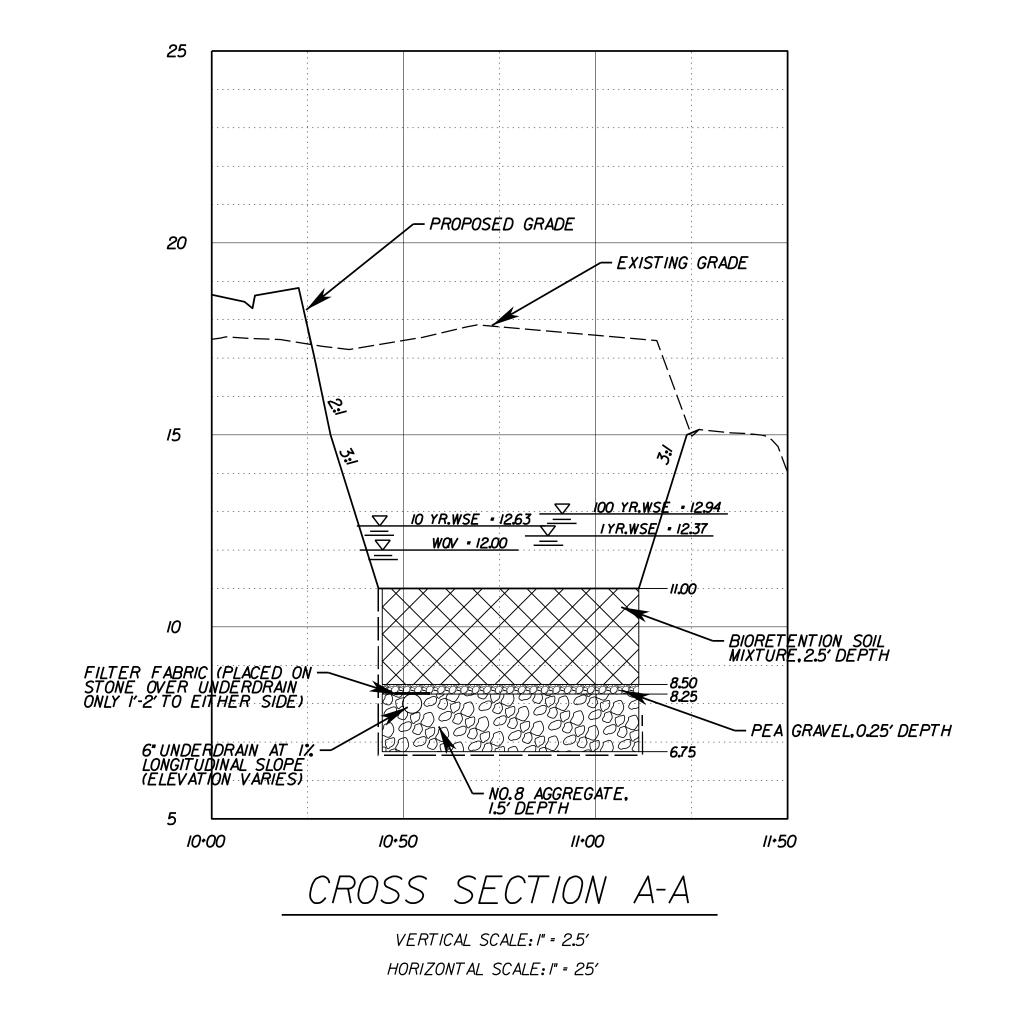
PROJECT MANAGER*Susie_Lue_(7.03)259-2918_NOVA_District______*SURVEYED BY, DATE *Leon_E_Treutle_LS_(7.03)259-3224_7/17/13______*DESIGN BY *JMT_Engineering_(8.04) 323-9900______*SUBSURFACE_UTILITY_BY, DATE *Leon_E.Treutle_LS_(7.03)259-3224_7/17/13*

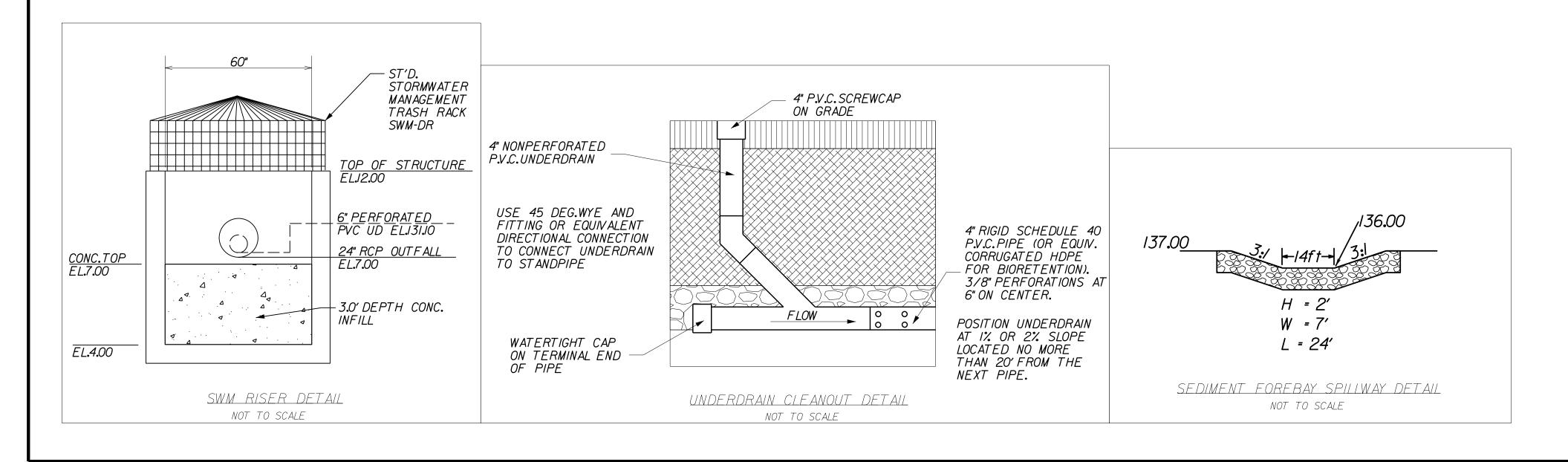
BASIN #5 DETAILS, PROFILE, AND CROSS-SECTION

DESIGN FEATURES RELATING TO CONSTRUCTION
OR TO REGULATION AND CONTROL OF TRAFFIC
MAY BE SUBJECT TO CHANGE AS DEEMED
NECESSARY BY THE DEPARTMENT

VDOT Location & Design Richmond, Virginia HYDRAULIC ENGINEER



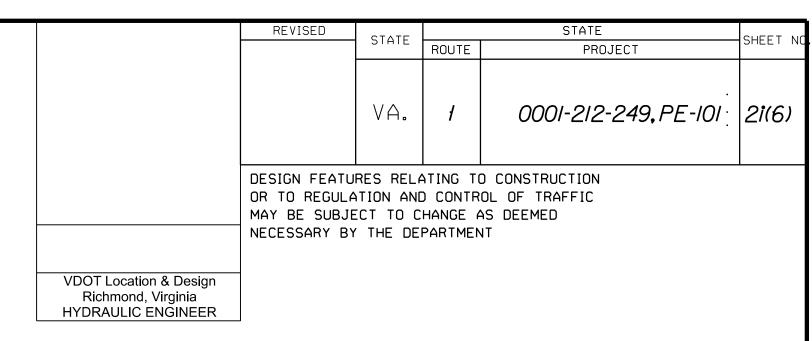


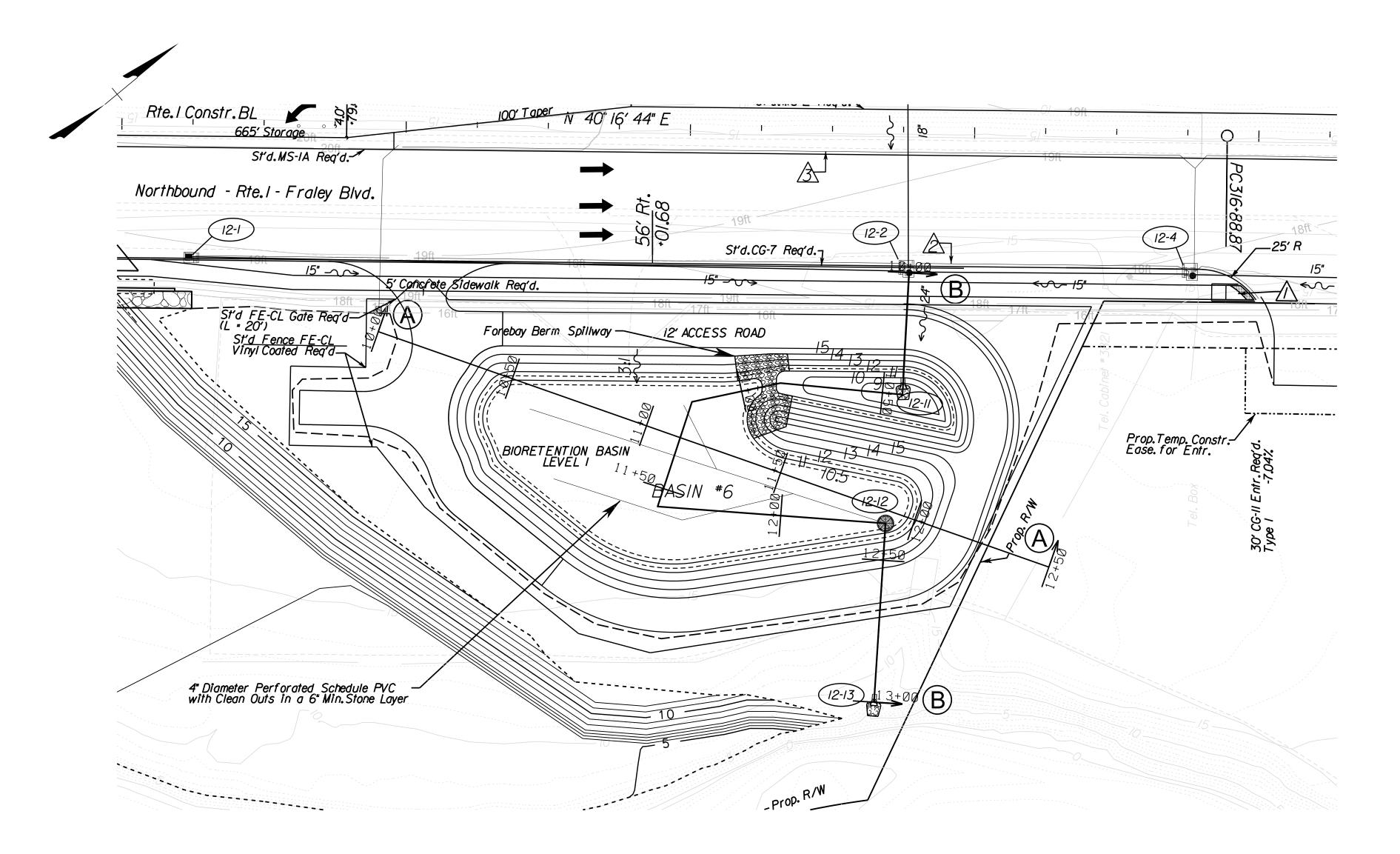


PROJECT SHEET NO. 21(5A)

R/W PLANS

STORMWATER BASIN #6 GRADING PLAN





Notoc

The foundation material under the dam and the material to be used for the embankment of the dam shall be an AASHTO Type A-4 or finer and meet the approval of the materials engineer.

- 2. The contractor shall provide 'As-Built' drawings of all stormwater management facilities. The 'As-Built' drawings shall show the actual finished ground contours, outlet structure dimensions and elevations, etc. as they exist at the completion of the project. These drawings shall be signed and sealed by the Licensed Professional Engineer or Land Surveyor registered in the State of Virginia. All costs shall be included under Construction Surveying.
- 3. The as-built drawings shall be signed and sealed by a Professional Engineer or Land Surveyor.
- 4. SWM 6 shall be maintained by the Virginia Department of Transportation in accordance with VDOT BMP Maintenance Manual.
- 5. For Profile SWM 3 (B-B), Cross Section A-A and details see sheet 2i(4A).
- 6. For Drainage Descriptions see sheet 2E(I)-2E(6).
 For Landscaping and Planting Plan see sheet 2i(I)
 For Bioretention Construction Sequence see sheet 2i(2)
- 7. Construction Inspections must be conducted in accordance with Non-Proprietary BMPs 9 Bioretention Inspection Checklist, dated April 2018, including completion of the Prince William County Construction Inspection Checklist found at https://www.pwcva.gov/department/environmental-services/stormwater-management Additionally, the Inspection Checklist shall be kept in the Route I Fraley Boulevard Project SWPPP.

NOTE: FOR TEMPORARY SWM-I STR.T-12-12 RISER OF 60" AT ELEVATION 10.00' IS REQUIRED. TEMPORARY SWM-I STRUCTURE SHALL BE CONVERTED TO PERMANENT STRUCTURE 12-12 WITH RISER OF 60" AT ELEVATION 11.50'.

SCALE PROJECT SHEET NO. 0001-212-249 21(6)